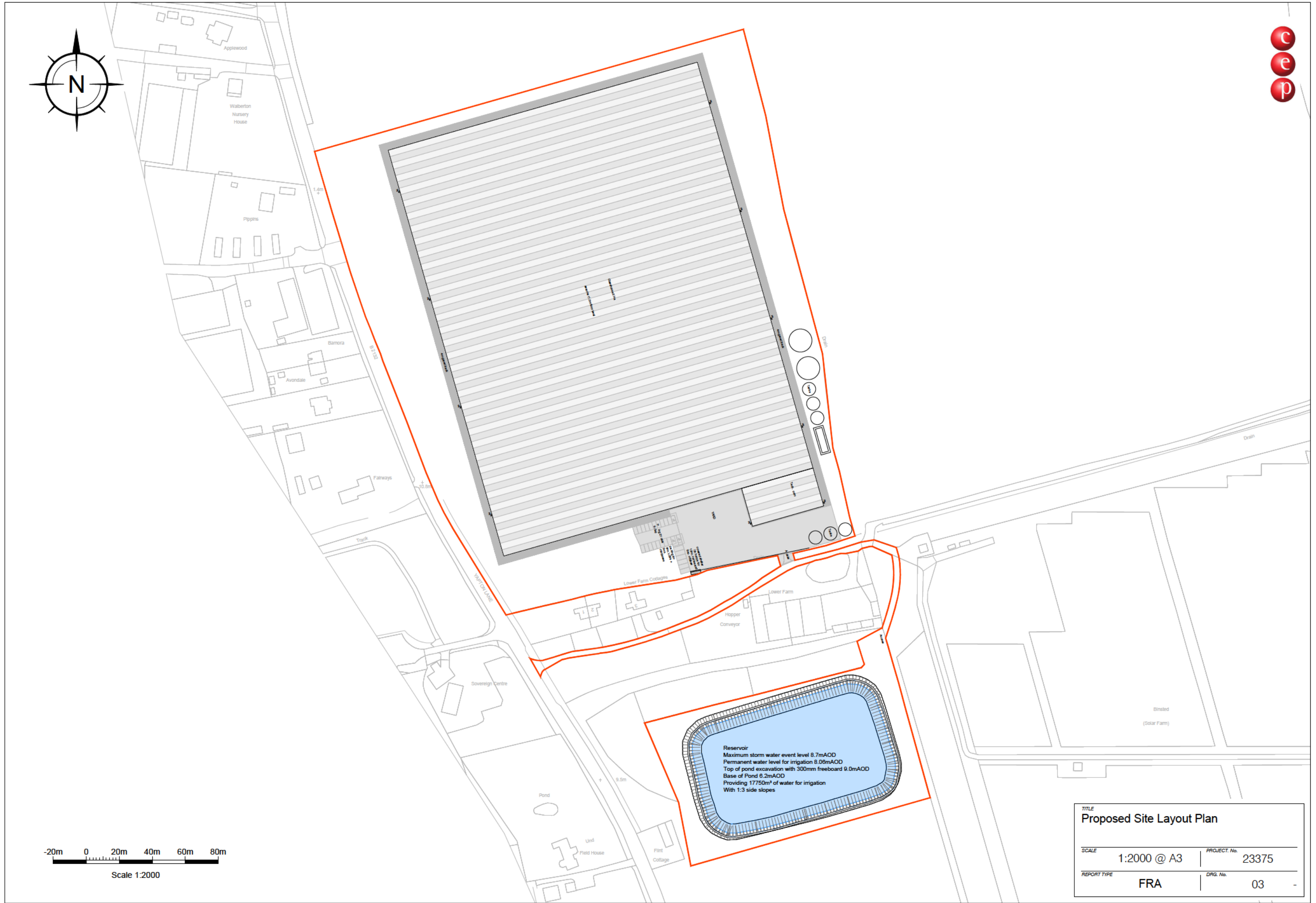
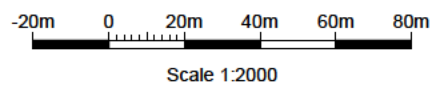


Appendix 6

Proposed Site Layout Plan and Impermeable Areas Plan



Reservoir
Maximum storm water event level 8.7mAOD
Permanent water level for irrigation 8.08mAOD
Top of pond excavation with 300mm freeboard 9.0mAOD
Base of Pond 8.2mAOD
Providing 17750m³ of water for irrigation
With 1:3 side slopes

TITLE Proposed Site Layout Plan	
SCALE 1:2000 @ A3	PROJECT No. 23375
REPORT TYPE FRA	DRG. No. 03

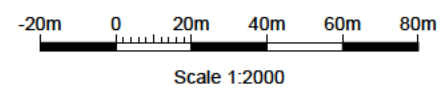


Gravel access track to
perimeter of greenhouse

Impermeable Areas

Roof 51375m²
Yard 3450m²

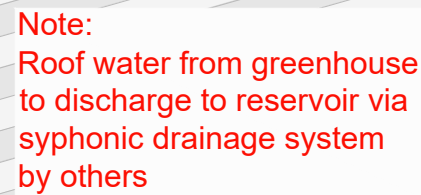
Red Line 92733m² (9.3ha)



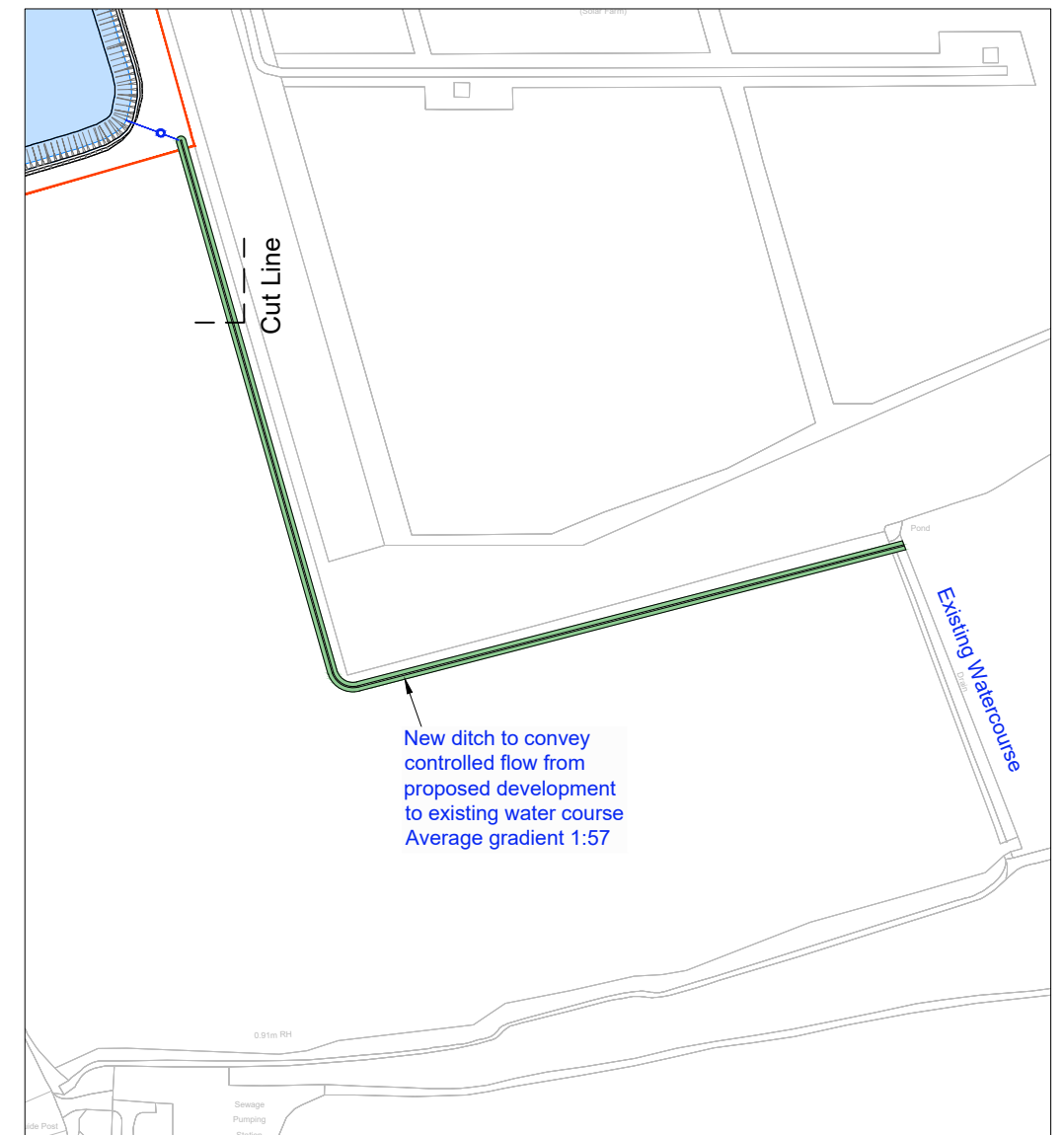
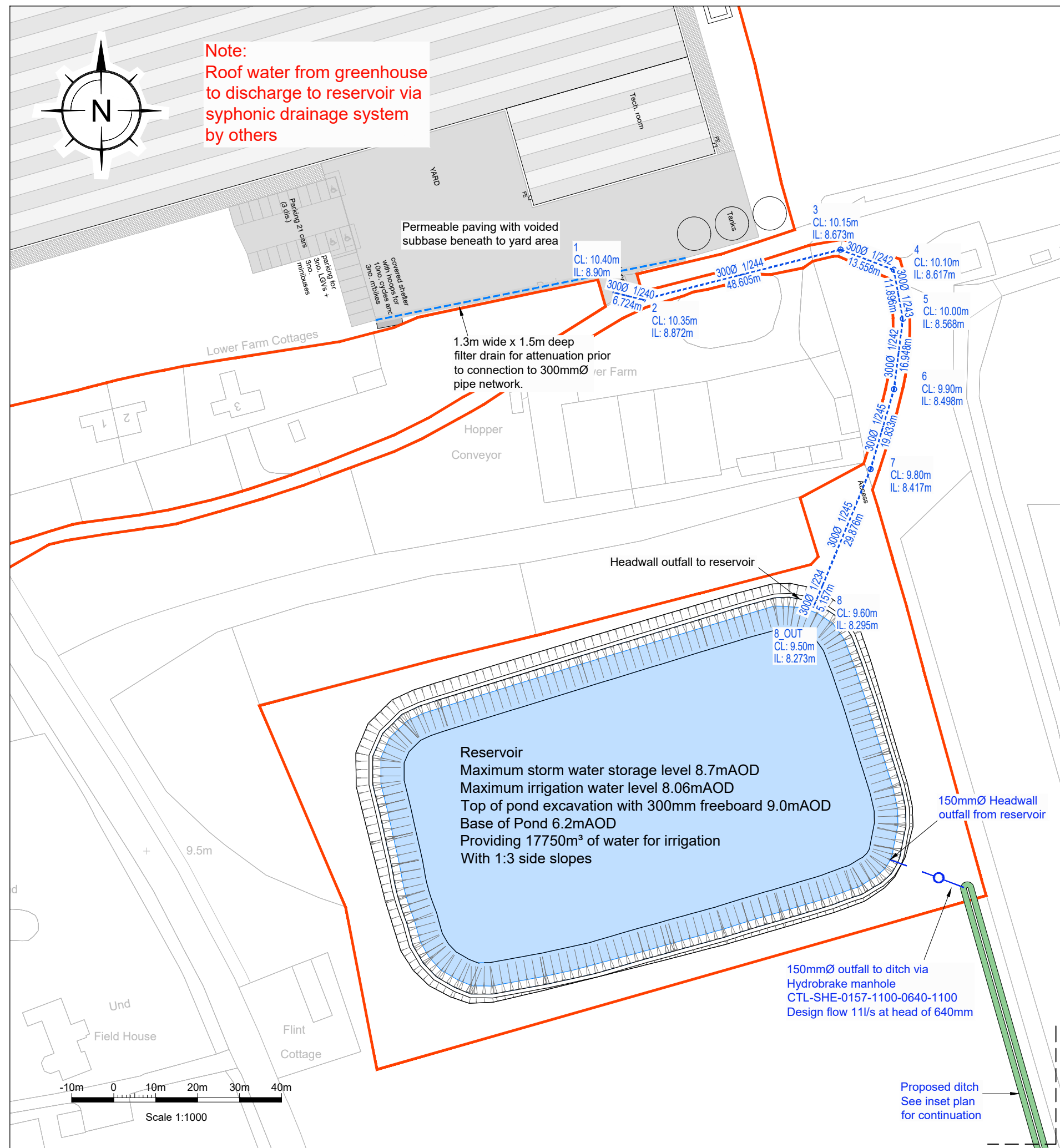
TITLE			
Proposed Impermeable Areas Plan			
SCALE		PROJECT No.	23375
1:2000 @ A3			
REPORT TYPE		DRG. No.	04
FRA			

Appendix 7

**Drainage Strategy Plans and Calculations and
Exceedance Event Flow Route Plan**

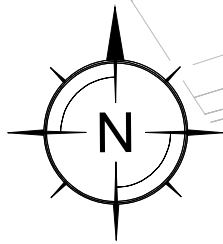


PHASE: 0							
MH No.	MANHOLE DIAMETER (mm)	MANHOLE TYPE	COVER LEVEL (m)	INVERT LEVEL (m)	DEPTH TO SOFFIT (m)	EASTING (m)	NORTHING (m)
1	1200	Type E	10.40	8.90	1.20	498052.947	104878.337
2	1200	Type E	10.35	8.872	1.178	498059.471	104876.711
3	1200	Type E	10.15	8.673	1.177	498106.59	104888.638
4	1200	Type E	10.10	8.617	1.183	498119.275	104883.852
5	1200	Type E	10.00	8.568	1.132	498121.447	104872.156
6	1200	Type E	9.90	8.498	1.102	498119.336	104855.34
7	1200	Type E	9.80	8.417	1.083	498113.757	104836.308
8	1200	Type E	9.60	8.295	1.005	498102.354	104808.694
8 OUT	1200	Type B	9.50	8.273	0.927	498100.355	104803.94



INSET PLAN
(Scale 1:2500)

TITLE	
Proposed Drainage Strategy Plan	
SCALE	PROJECT. No.
1:1000 @ A3	23375
REPORT TYPE	DRG. No.
FRA	05 -



Scale 1:2000

150mmØ @1:70 average gradient
private foul drainage serving proposed
development

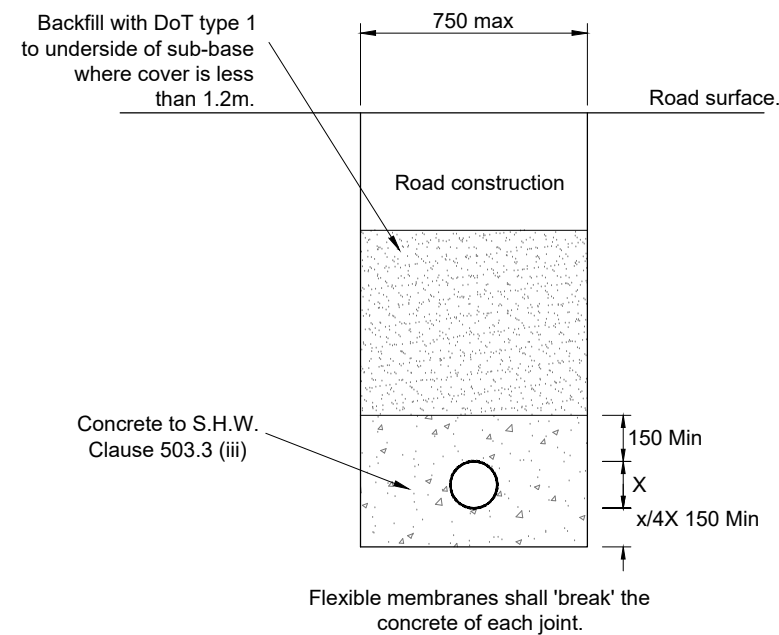
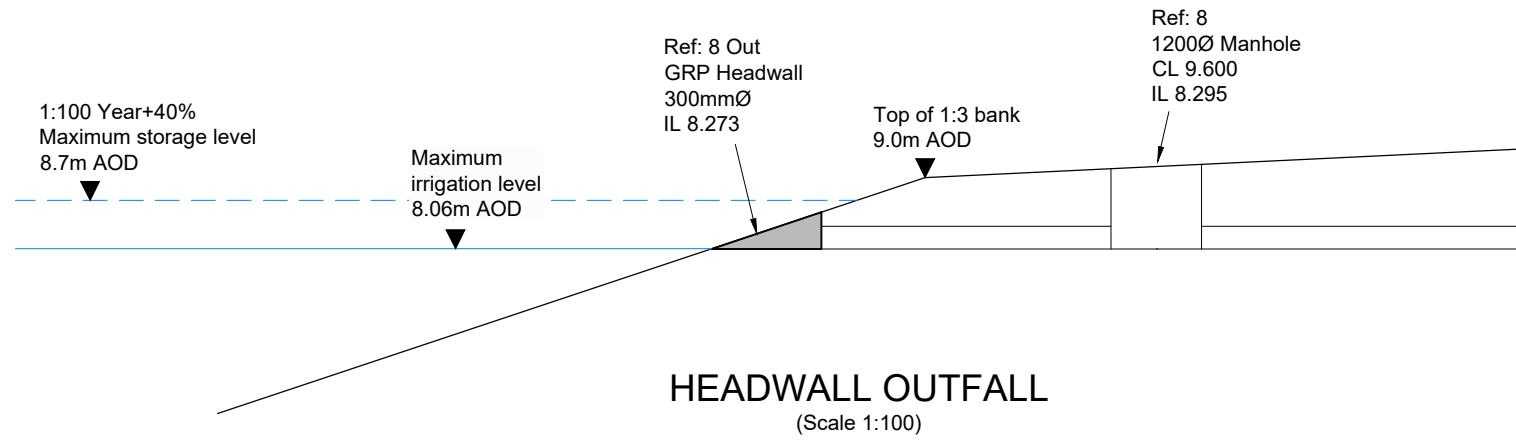
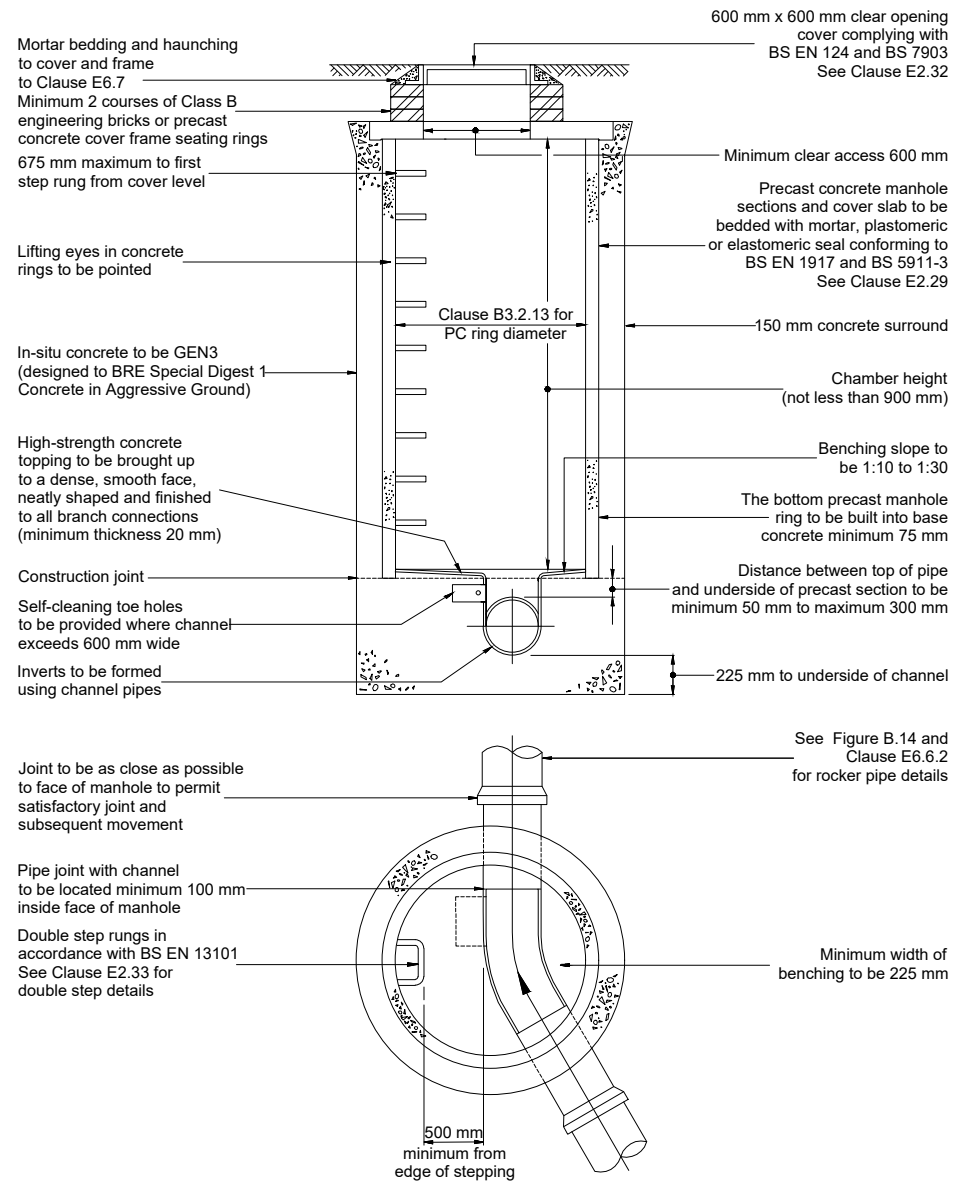
Foul water from development to
discharge to existing Southern Water
manhole via gravity subject to
Section 106 application

1404
CL 3.97
IL 2.30

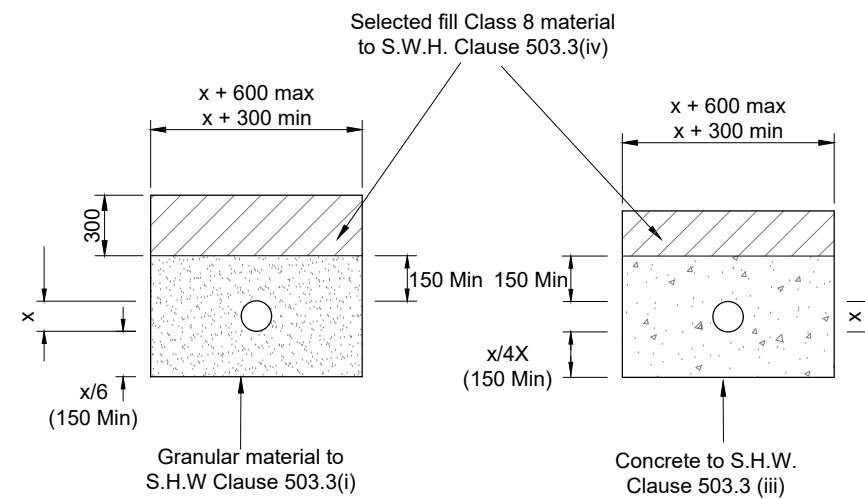
Sewage
Pumping
Station

TITLE Proposed Foul Drainage Strategy Plan	
SCALE 1:2000 @ A3	PROJECT No. 23375
REPORT TYPE FRA	DRG. No. 06

FIGURE B.12
TYPICAL MANHOLE DETAIL - TYPE 2
Maximum depth from cover level to soffit of pipe 3.0 m



PIPE BEDDING FOR ADOPTABLE DRAINS UNDER HIGHWAY CONSTRUCTION



CLASS S PIPE BEDDING

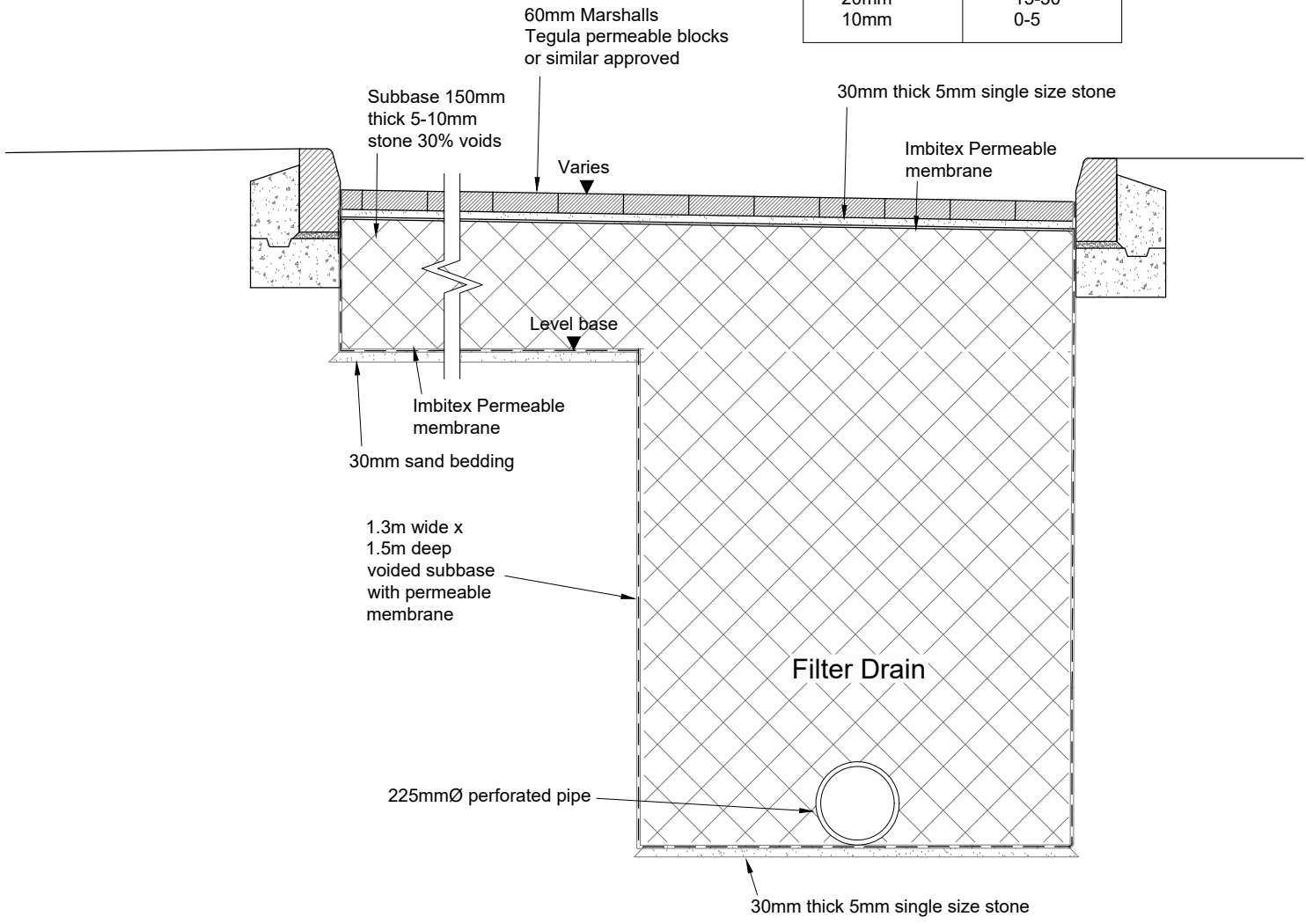
CLASS Z PIPE BEDDING

PIPE BEDDING DETAILS
(ADOPTABLE DRAINS)
(Not To Scale)

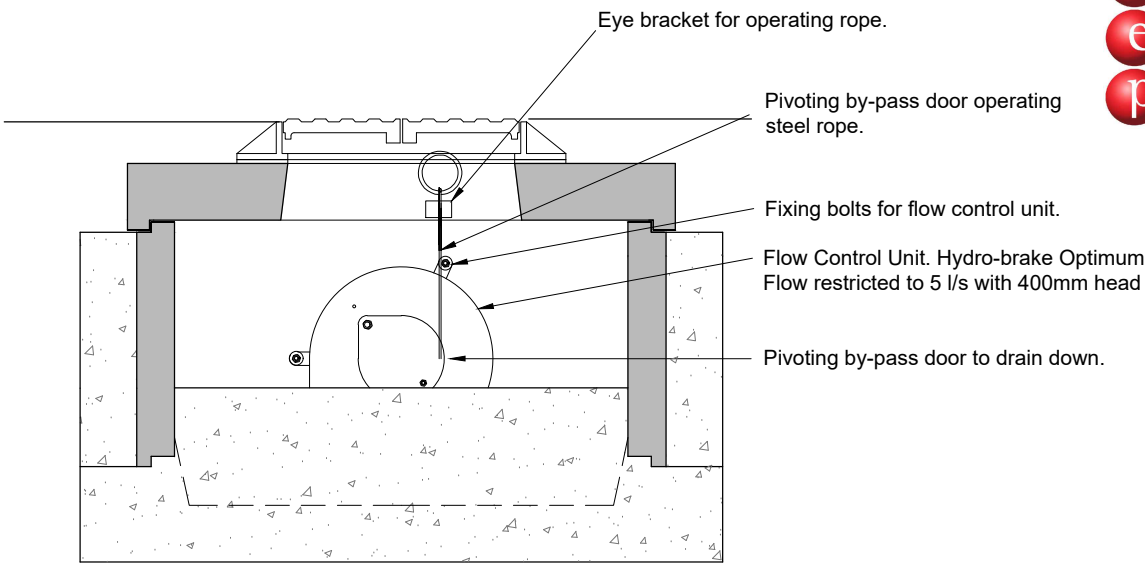
TITLE Construction Details Sheet 1	
SCALE As Shown	PROJECT No. 23375
REPORT TYPE DS	DRG. No. 07

* Grading of Sub-base stone

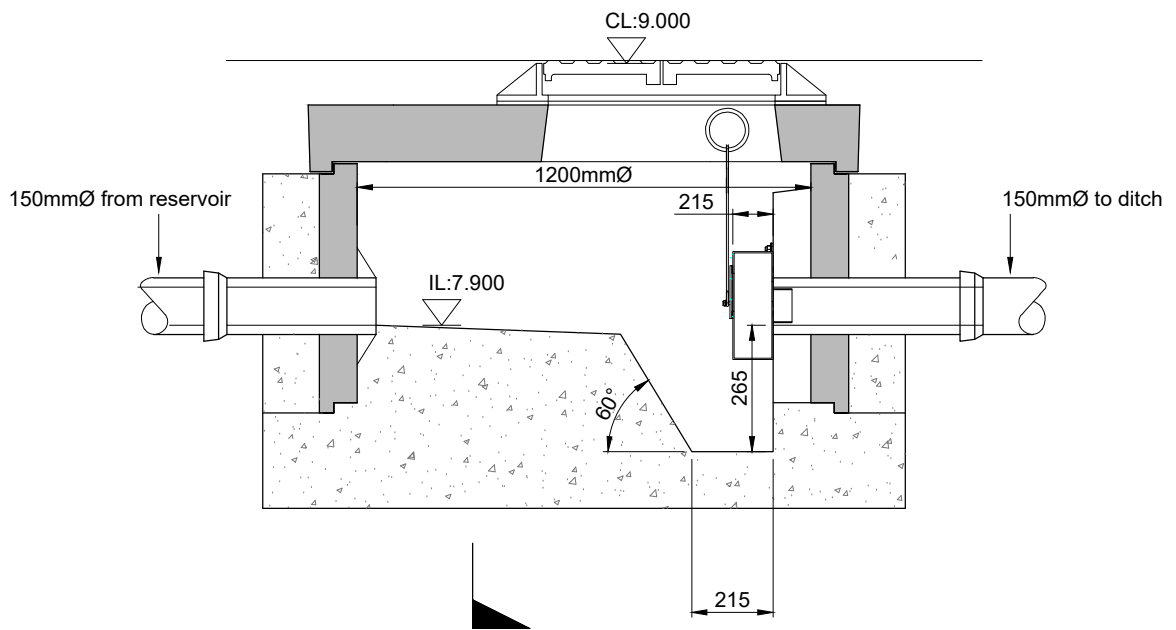
SIEVE SIZE	% PASSING
100mm	100
63mm	90-100
37.5mm	60-80
20mm	15-30
10mm	0-5



**PERMEABLE BLOCK PAVING YARD CONSTRUCTION
WITH VOIDED SUBBASE AND FILTER DRAIN**
(Scale 1:20)



SECTION
(Scale 1:20)



HYDROBRAKE MANHOLE DETAIL
(Scale 1:20)

TITLE Construction Details Sheet 2			
SCALE	As Shown	PROJECT No.	23375
REPORT TYPE	DS	DRG. No.	08

Design Settings

Rainfall Methodology	FEH-13	Minimum Velocity (m/s)	1.00
Return Period (years)	2	Connection Type	Level Soffits
Additional Flow (%)	0	Minimum Backdrop Height (m)	1.000
CV	0.850	Preferred Cover Depth (m)	1.200
Time of Entry (mins)	5.00	Include Intermediate Ground	✓
Maximum Time of Concentration (mins)	30.00	Enforce best practice design rules	✓
Maximum Rainfall (mm/hr)	50.0		

Nodes

Name	Area (ha)	T of E (mins)	Cover Level (m)	Diameter (mm)	Easting (m)	Northing (m)	Depth (m)
1	0.345	5.00	10.400	1200	498052.947	104878.337	1.500
2			10.350	1200	498059.471	104876.711	1.478
3			10.150	1200	498106.590	104888.638	1.477
4			10.100	1200	498119.275	104883.852	1.483
5			10.000	1200	498121.447	104872.156	1.432
6			9.900	1200	498119.336	104855.340	1.402
7			9.800	1200	498113.757	104836.308	1.383
8			9.600	1200	498102.354	104808.694	1.305
8_OUT			9.500	1200	498100.355	104803.940	1.227

Links

Name	US Node	DS Node	Length (m)	ks (mm) / n	US IL (m)	DS IL (m)	Fall (m)	Slope (1:X)	Dia (mm)	T of C (mins)	Rain (mm/hr)
1.000	1	2	6.724	0.600	8.900	8.872	0.028	240.1	300	5.11	50.0
1.001	2	3	48.605	0.600	8.872	8.673	0.199	244.2	300	5.92	50.0
1.002	3	4	13.559	0.600	8.673	8.617	0.056	242.1	300	6.14	50.0
1.003	4	5	11.896	0.600	8.617	8.568	0.049	242.8	300	6.34	50.0
1.004	5	6	16.948	0.600	8.568	8.498	0.070	242.1	300	6.62	50.0
1.005	6	7	19.833	0.600	8.498	8.417	0.081	244.9	300	6.95	50.0
1.006	7	8	29.876	0.600	8.417	8.295	0.122	244.9	300	7.45	48.6
1.007	8	8_OUT	5.157	0.600	8.295	8.273	0.022	234.4	300	7.54	48.4

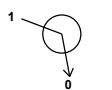
Name	Vel (m/s)	Cap (l/s)	Flow (l/s)	US Depth (m)	DS Depth (m)	Σ Area (ha)	Σ Add Inflow (l/s)	Pro Depth (mm)	Pro Velocity (m/s)
1.000	1.010	71.4	53.0	1.200	1.178	0.345	0.0	193	1.103
1.001	1.001	70.8	53.0	1.178	1.177	0.345	0.0	194	1.095
1.002	1.006	71.1	53.0	1.177	1.183	0.345	0.0	194	1.099
1.003	1.004	71.0	53.0	1.183	1.132	0.345	0.0	194	1.098
1.004	1.006	71.1	53.0	1.132	1.102	0.345	0.0	194	1.099
1.005	1.000	70.7	53.0	1.102	1.083	0.345	0.0	194	1.094
1.006	1.000	70.7	51.5	1.083	1.005	0.345	0.0	191	1.088
1.007	1.022	72.3	51.2	1.005	0.927	0.345	0.0	187	1.107

Pipeline Schedule

Link	Length (m)	Slope (1:X)	Dia (mm)	Link Type	US CL (m)	US IL (m)	US Depth (m)	DS CL (m)	DS IL (m)	DS Depth (m)
1.000	6.724	240.1	300	Circular	10.400	8.900	1.200	10.350	8.872	1.178
1.001	48.605	244.2	300	Circular	10.350	8.872	1.178	10.150	8.673	1.177
1.002	13.559	242.1	300	Circular	10.150	8.673	1.177	10.100	8.617	1.183
1.003	11.896	242.8	300	Circular	10.100	8.617	1.183	10.000	8.568	1.132
1.004	16.948	242.1	300	Circular	10.000	8.568	1.132	9.900	8.498	1.102
1.005	19.833	244.9	300	Circular	9.900	8.498	1.102	9.800	8.417	1.083
1.006	29.876	244.9	300	Circular	9.800	8.417	1.083	9.600	8.295	1.005
1.007	5.157	234.4	300	Circular	9.600	8.295	1.005	9.500	8.273	0.927

Link	US Node	Dia (mm)	Node Type	MH Type	DS Node	Dia (mm)	Node Type	MH Type
1.000	1	1200	Manhole	Adoptable	2	1200	Manhole	Adoptable
1.001	2	1200	Manhole	Adoptable	3	1200	Manhole	Adoptable
1.002	3	1200	Manhole	Adoptable	4	1200	Manhole	Adoptable
1.003	4	1200	Manhole	Adoptable	5	1200	Manhole	Adoptable
1.004	5	1200	Manhole	Adoptable	6	1200	Manhole	Adoptable
1.005	6	1200	Manhole	Adoptable	7	1200	Manhole	Adoptable
1.006	7	1200	Manhole	Adoptable	8	1200	Manhole	Adoptable
1.007	8	1200	Manhole	Adoptable	8_OUT	1200	Manhole	Adoptable

Manhole Schedule

Node	Easting (m)	Northing (m)	CL (m)	Depth (m)	Dia (mm)	Connections	Link	IL (m)	Dia (mm)	
1	498052.947	104878.337	10.400	1.500	1200		0	1.000	8.900	300
2	498059.471	104876.711	10.350	1.478	1200		1	1.000	8.872	300
						0	1.001	8.872	300	
3	498106.590	104888.638	10.150	1.477	1200		1	1.001	8.673	300
						0	1.002	8.673	300	
4	498119.275	104883.852	10.100	1.483	1200		1	1.002	8.617	300
						0	1.003	8.617	300	
5	498121.447	104872.156	10.000	1.432	1200		1	1.003	8.568	300
						0	1.004	8.568	300	
6	498119.336	104855.340	9.900	1.402	1200		1	1.004	8.498	300
						0	1.005	8.498	300	
7	498113.757	104836.308	9.800	1.383	1200		1	1.005	8.417	300
						0	1.006	8.417	300	

Manhole Schedule

Node	Easting (m)	Northing (m)	CL (m)	Depth (m)	Dia (mm)	Connections	Link	IL (m)	Dia (mm)	
8	498102.354	104808.694	9.600	1.305	1200		1	1.006	8.295	300
							0	1.007	8.295	300
8_OUT	498100.355	104803.940	9.500	1.227	1200		1	1.007	8.273	300

Simulation Settings

Rainfall Methodology	FEH-13	Analysis Speed	Normal	Additional Storage (m³/ha)	20.0
Summer CV	0.850	Skip Steady State	x	Check Discharge Rate(s)	x
Winter CV	0.850	Drain Down Time (mins)	240	Check Discharge Volume	x

Storm Durations

15	30	60	120	180	240	360	480	600	720	960	1440
----	----	----	-----	-----	-----	-----	-----	-----	-----	-----	------

Return Period (years)	Climate Change (CC %)	Additional Area (A %)	Additional Flow (Q %)
100	40	0	0

Node 1 Depth/Area Storage Structure

Base Inf Coefficient (m/hr)	0.00000	Safety Factor	2.0	Invert Level (m)	8.900
Side Inf Coefficient (m/hr)	0.00000	Porosity	0.30	Time to half empty (mins)	11

Depth (m)	Area (m²)	Inf Area (m²)	Depth (m)	Area (m²)	Inf Area (m²)	Depth (m)	Area (m²)	Inf Area (m²)
0.000	97.5	0.0	1.400	97.5	0.0	1.401	0.0	0.0



Results for 100 year +40% CC Critical Storm Duration. Lowest mass balance: 100.00%

Node Event	US Node	Peak (mins)	Level (m)	Depth (m)	Inflow (l/s)	Node Vol (m³)	Flood (m³)	Status
30 minute summer	1	21	10.345	1.445	223.1	49.2444	0.0000	FLOOD RISK
30 minute summer	2	21	10.222	1.350	107.4	1.5268	0.0000	FLOOD RISK
30 minute summer	3	22	9.741	1.068	104.9	1.2074	0.0000	SURCHARGED
30 minute summer	4	22	9.565	0.948	103.2	1.0727	0.0000	SURCHARGED
30 minute summer	5	22	9.404	0.836	103.4	0.9455	0.0000	SURCHARGED
30 minute summer	6	22	9.196	0.698	103.3	0.7900	0.0000	SURCHARGED
30 minute summer	7	22	8.963	0.546	103.2	0.6176	0.0000	SURCHARGED
30 minute summer	8	22	8.640	0.345	103.0	0.3897	0.0000	SURCHARGED
30 minute summer	8_OUT	22	8.521	0.248	102.9	0.0000	0.0000	OK

Link Event (Upstream Depth)	US Node	Link	DS Node	Outflow (l/s)	Velocity (m/s)	Flow/Cap	Link Vol (m³)	Discharge Vol (m³)
30 minute summer	1	1.000	2	107.4	1.525	1.504	0.4735	
30 minute summer	2	1.001	3	104.9	1.489	1.481	3.4227	
30 minute summer	3	1.002	4	103.2	1.466	1.452	0.9548	
30 minute summer	4	1.003	5	103.4	1.468	1.456	0.8377	
30 minute summer	5	1.004	6	103.3	1.468	1.454	1.1935	
30 minute summer	6	1.005	7	103.2	1.465	1.459	1.3966	
30 minute summer	7	1.006	8	103.0	1.463	1.457	2.1038	
30 minute summer	8	1.007	8_OUT	102.9	1.478	1.424	0.3424	140.7

Design Settings

Rainfall Methodology	FEH-13	Minimum Velocity (m/s)	1.00
Return Period (years)	2	Connection Type	Level Soffits
Additional Flow (%)	0	Minimum Backdrop Height (m)	0.200
CV	0.850	Preferred Cover Depth (m)	1.200
Time of Entry (mins)	5.00	Include Intermediate Ground	✓
Maximum Time of Concentration (mins)	30.00	Enforce best practice design rules	✓
Maximum Rainfall (mm/hr)	50.0		

Nodes

Name	Area (ha)	Cover Level (m)	Depth (m)
1	5.480	9.000	0.940

Simulation Settings

Rainfall Methodology	FEH-13	Skip Steady State	✓	2 year (l/s)	0.0
Summer CV	0.850	Drain Down Time (mins)	240	Check Discharge Volume	✓
Winter CV	0.850	Additional Storage (m³/ha)	0.0	100 year 360 minute (m³)	
Analysis Speed	Normal	Check Discharge Rate(s)	✓		

Storm Durations

360	600	960	2160	4320	7200	10080
480	720	1440	2880	5760	8640	

Return Period (years)	Climate Change (CC %)	Additional Area (A %)	Additional Flow (Q %)
100	40	0	0

Pre-development Discharge Rate

Site Makeup	Greenfield	Soil Index	1	Betterment (%)	0
Greenfield Method	IH124	SPR	0.10	QBar	
Positively Drained Area (ha)		Region	1	Q 2 year (l/s)	
SAAR (mm)		Growth Factor 2 year	0.90		

Pre-development Discharge Volume

Site Makeup	Greenfield	SPR	0.10	Storm Duration (mins)	360
Greenfield Method	FSR/FEH	CWI		Betterment (%)	0
Positively Drained Area (ha)		Return Period (years)	100	PR	
Soil Index	1	Climate Change (%)	0	Runoff Volume (m³)	

Node 1 Online Hydro-Brake® Control

Flap Valve	x	Objective	(HE) Minimise upstream storage
Replaces Downstream Link	✓	Sump Available	✓
Invert Level (m)	8.060	Product Number	CTL-SHE-0157-1100-0640-1100
Design Depth (m)	0.640	Min Outlet Diameter (m)	0.225
Design Flow (l/s)	11.0	Min Node Diameter (mm)	1200

Node 1 Depth/Area Storage Structure

Base Inf Coefficient (m/hr)	0.00000	Safety Factor	2.0	Invert Level (m)	8.060
Side Inf Coefficient (m/hr)	0.00000	Porosity	1.00	Time to half empty (mins)	



Depth (m)	Area (m ²)	Inf Area (m ²)	Depth (m)	Area (m ²)	Inf Area (m ²)
0.000	7971.0	0.0	0.940	8888.5	0.0



Results for 100 year +40% CC Critical Storm Duration. Lowest mass balance: 99.99%

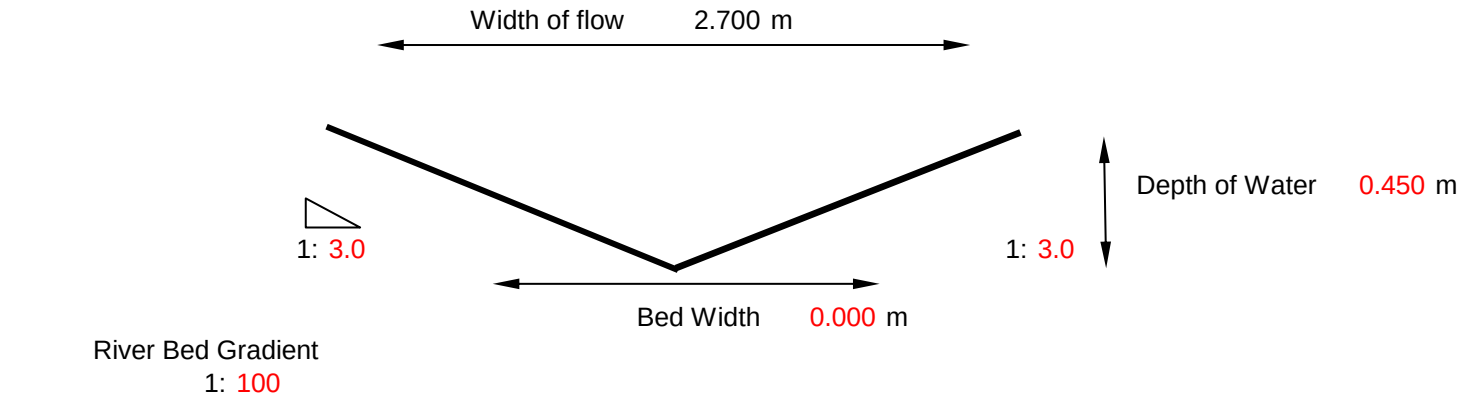
Node Event	US Node	Peak (mins)	Level (m)	Depth (m)	Inflow (l/s)	Node Vol (m ³)	Flood (m ³)	Status
10080 minute summer	1	6900	8.692	0.632	59.6	5229.8260	0.0000	OK

Link Event (Upstream Depth)	US Node	Link	Outflow (l/s)	Discharge Vol (m ³)
10080 minute summer	1	Hydro-Brake®	11.0	4832.3

23375 Lower Farm - Outfall Drainage Ditch

Open Channel Flow - Manning Equation

Kinematic Viscosity of Water at 15°C= 1.139E-06 m²/s



Area	A =	0.608 m ²	Discharge Capacity $Q = \frac{1}{n} \frac{A^{\frac{5}{3}}}{p^{\frac{2}{3}}} S_o^{\frac{1}{2}} =$	0.620 m³/s 619.937 l/s
Wetted Perimeter	P =	2.846 m		
Bed Slope	So =	0.0100		
Hydraulic Radius	R =	0.213 m		
Manning's n	n =	0.035		
Reynolds Number	Re =	1.913E+05	Average Velocity =	1.020 m/s

Appendix 8

Arun District Council's Surface Water Drainage Proposal Checklist

Surface Water Drainage Proposal Checklist

Ground Investigation Results				
Results Required	Results Provided?			
Winter groundwater monitoring results (Please refer to guidance note 1 below)	Yes	☐	No	☒
Period of winter groundwater monitoring ^{Note 1}	From	PENDING		
	To	PENDING		
Maximum recorded groundwater level	BGS INDICATES MAOB 3M BGL mBGL			
Winter infiltration test results ^{Note 2}	Yes	☐	No	☒
Date of winter infiltration testing	100% RECYCLING			
Details of the location and depth of the infiltration testing ^{Note 3}	Yes	☐	No	☒
Minimum infiltration rate ^{Note 4}	NA			m/s

Guidance Notes

1. A full winters groundwater monitoring should be undertaken, (October to March inclusive) to enable the peak annual groundwater levels to be established. The extent of winter monitoring for developments of less than 5 properties should be discussed with the council's Engineers.
2. Infiltration testing should be undertaken during the wet winter months (typically January) and in line with BRE365, or a similar approved method. Tests should be repeated 3x, to replicate saturated ground conditions. Please note that infiltration testing is to be carried out at a time and depth agreed with the Councils engineers.
3. Infiltration testing should be undertaken at the location and depth of any proposed infiltration structures. This must be above the maximum recorded groundwater level in that vicinity.
4. The minimum infiltration rate obtained at the location and depth of the proposed infiltration structures should be used to inform the design of the structures.

Proposed Method of Surface Water Disposal				
Infiltration	Yes	☐	No	☒
On-site, via soak-away structures	Yes	☒	No	☐
If on-site infiltration is not to be used, has justification for this been provided? ^{Note 5}	Yes	☒	No	☐
Attenuation (on-site) with a restricted discharge to a watercourse on, or adjacent to, the site	Yes	☒	No	☐
Attenuation (on-site) with a restricted discharge to a surface water sewer on, or adjacent to, the site	Yes	☐	No	☐
Attenuation (on-site) with a restricted discharge to a foul water sewer on, or adjacent to, the site	Yes	☐	No	☐

Guidance Notes

5. If on-site infiltration is not being used to drain the site you will need to provide evidence that demonstrates on-site infiltration is unviable.

Drainage Design: Supporting Calculations					
Calculations Required			Calculations Provided?		
Infiltration Structures	Half-drain time for 10 year event (plus climate change allowance)	Yes		No	
					Hours
	Half-drain time for 100 year event (plus climate change allowance)	Yes		No	
					Hours
	Calculations demonstrating that the 1 in 100 year event, plus climate change allowance can be accommodated on-site. ^{Note 7}	Yes		No	
	Lowest base level of infiltration structure				mAOD mBGL
	Has the design sought to provide treatment of potential contaminants? E.g. hydrocarbons.	Yes		No	
Attenuation and Subsequent Restricted Discharge	Pre-development run-off rate calculations	Yes	✓	No	
		QBAR			l/s
	Proposed discharge rate ^{Note 6}				l/s
	Calculations demonstrating that the 1 in 100 year event, plus climate change allowance can be accommodated on-site. ^{Note 7}	Yes	✓	No	
	Has the design sought to provide treatment of potential contaminants? E.g. hydrocarbons.	Yes	✓	No	
	If your proposed scheme incorporates any impermeable lined attenuation features we require calculations to demonstrate that appropriate resistance to floatation (due to groundwater levels) is catered for in the design	Yes	NA	No	
	If you propose discharging into or altering an ordinary watercourse; have you sought Ordinary Watercourse/Land Drainage Consent? ^{Note 8}	Yes	NA	No	
	If discharging to a watercourse, piped system or the sea, has the proposed drainage network been modelled against predicted top water levels for the 1 in 100 year storm event plus climate change allowance, within the existing system? ^{Note 9}	Yes	NA	No	

Guidance Notes

NON SURCHARGES OUTFALL

- Any proposed discharge rate must be less than, or equal to, the pre-development rate. Greenfield sites must use the mean flow rate (Qbar) and be based upon the area occupied by the development, ignoring significant open spaces. Please note that the minimum acceptable discharge rate is 2l/s when using formal flow control devices, unless otherwise agreed. Additionally, if you are developing a brownfield site, we expect you to provide fully detailed plans of the existing surface water drainage arrangements on site, including impermeable areas, gullies, outfalls, pipes & diameters, manholes, etc. We generally seek 50% betterment in terms of run off from brownfield sites.
- The appropriate factor of safety from the table below must be applied to your calculations to ensure your design is fully in accordance with CIRIA R156/ CIRIA C753.

Factor of Safety Table						
Size of Area to be Drained	Consequences of failure					
	No damage or inconvenience		Minor inconvenience, e.g. surface water on car park		Damage to buildings or structures or roads.	
	Cv=1.0	Cv=0.75	Cv=1.0	Cv=0.75	Cv=1.0	Cv=0.75
<100 m ²	1.5	2	2	3	10	13
100m ² to 1000m ²	1.5	2	3	4	10	13
>1000m ²	1.5	2	5	6.5	10	13

8. [Ordinary Watercourse/Land Drainage Consent](#) will need to be sought, ideally in parallel with planning permission, if you propose discharging into, culverting or altering a watercourse.
9. Modelling is required to ensure that problems, such as surcharged outfalls, are addressed.

Plans/Drawings/Diagrams				
Required	Provided?			
Plan detailing the location of groundwater monitoring and infiltration testing	Yes	☐	No	☒
Detailed drainage layout plan ^{Note 10}	Yes	☒	No	☐
Construction detail plans ^{Note 11}	Yes	☒	No	☐
Exceedance flow route plans ^{Note 12}	Yes	☒	No	☐
Impermeable area plan	Yes	☒	No	☐
If ground levels are being raised $\geq 300\text{mm}$ above existing levels and is unavoidable, have fully detailed plans been provided, together with drainage proposals, to address any potential drainage related issues.	Yes	NA	No	☐

Guidance Notes

10. Drainage layout plans should include:
 - All surface water drainage pipes labelled with; diameters, pipe materials, falls and invert levels.
 - All infiltrating and attenuating structures (including permeable paving) labelled with; dimensions, invert and cover levels.
 - All manholes labelled with; cover levels, invert levels, cover types detailed.
 - All silt traps clearly labelled with sump depths.
 - Control structures with outflow rates and invert levels.
 - Proposed/existing levels of any areas subject to ground raising, together with suitable measures/detailed drawings for the associated management of surface water runoff. Please note that ground raising should be avoided unless there are exceptional circumstances.
11. Site specific construction detail plans should be supplied if your proposed drainage scheme includes any of the following elements: pipe bed and surround; infiltrating structures; attenuating structures; manholes; catch-pits/silt traps; control devices; permeable paving; headwalls.
12. Unless specifically requested by the council's drainage engineers 'Exceedance Flow Route Plans' are only required for developments of more than 5 residential properties or more than 0.5ha of commercial development .

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Appendix 9

Outline Drainage Maintenance Schedule

Drainage Maintenance Schedule



Project	Lower Farm, Yapton Lane, Walberton
Project Number	23375

By Steve Doughty

Date 25 November 2020

1 Schedule of Maintenance

- 1.1 Once appointed the Contractor will prepare a site specific method statement for the control of silt and other pollutants during construction. CIRIA Report C532, Control of water pollution from construction sites, provides further guidance on this.
- 1.2 The Contractor will maintain the proposed drainage system during construction and until the handing over of the site.
- 1.3 Upon completion management of the onsite drainage infrastructure will be passed on to the Site Operator.
- 1.4 The following maintenance schedule details the typical tasks to be undertaken at different intervals.

Maintenance Schedule	Required Action	Frequency
Regular Maintenance	Manage vegetation and remove nuisance plants – aesthetics	As required
	Litter and debris removal – catch pits and infiltration basin	Monthly or as required
	Remove sediment and debris from silt trap chambers, channel drains and inlet chambers	6 monthly
	Visual inspection of permeable paving for defects and settlement	Annually
	Sweeping / brushing of permeable paving	Every 2 years
	Surface and foul water pipework – jetting / rodding	Every 2 years or as required
	Remove silt and debris from oil interceptors where provided	When alarm indicates
Corrective Maintenance	Remove debris / blockages to silt traps / channel drains	As required
	Repairs to access chambers / manhole covers	As required
	Replace any broken permeable blocks / surface, remedial works to any depressions or rutting	As required
	Inspect inlet, outlet from downpipe and gullies for blockages, standing water and clear	As required

Indicative Schedule of Maintenance for the Proposed Drainage System



Inspection / Action Required	Gullies, Channels and Gutters	Catchpits	Permeable Surface	Surface Water Pipework	Oil Interceptors
6 Months	✓	✓	✓		
1 Year			✓		
After leaf fall in Autumn	✓	✓			
2 Years				✓	
When alarm indicates					✓

Drainage System Maintenance Summary