

- SURFACE WATER DRAINAGE-**
- Surface water disposal should be managed in view of the SuDS hierarchy;
 - 1. Infiltration
 - 2. Watercourse
 - 3. Public Surface Water Sewer(s)
- Following the undertaking of groundwater monitoring and infiltration testing, there are limited opportunities to propose infiltration-based methods and techniques as a means of dealing with run-off generated by the development.
- It is proposed that surface water runoff is captured and attenuated within permeable block pavement (min 400-550mm subbase thickness, see modelling calculations for site variance), geocellular underground storage, and open ponds across the site, adjacent to the existing boundary watercourses.
- Permeable paving will have a total plan area of 6,749m², providing approximately 1,000m³ storage (based on a sub-base depth of between 400-550mm and 32% voids).
- Geocellular storage will provide a total attenuation volume of 427.5m³, based on tank depths of 0.8m and 95% voids.
- The open storage ponds will provide a total attenuation volume of 1,597.5m³, with 1:3 sloped sides.
- The approved drainage strategy determined the Greenfield runoff rate (QBar) for 'Site A' - South Side to be 34.0l/s.
- Further to the above, run-off from the development will be discharged at a rate of 34.0l/s.
- It is proposed that all captured run-off from the proposed areas of impermeable surfacing will be captured up to the in 100 year storm event, including an allowance of 40% for Climate Change (without exceedance).

- FOUL WATER STRATEGY-**
- Foul water disposal should adhere to the following hierarchy;
 - 1. Public Sewer
 - 2. Private Treatment Plant
 - 3. Septic Tank and Drainage Field
 - 4. Cesspool
- The proposed development will establish a new connection to the public foul water sewer located in Summer Lane, via a proposed foul pumping station to serve both the northern and southern parcels of the proposed development.
- It is proposed that flows from the site will be pumped, with a Type 3 adoptable pumping station required.
- In relation to the proposed Type 3 adoptable pumping station, the wet well must be positioned at least 15.00m from habitable buildings.
- Post development, there will be an estimated peak foul water flow generation of 9.70l/s (based on Water UK Sewer Sector Guidance, Design and Construction Guidance).



- DRAINAGE KEY-**
- Watercourse Building/Structure Buffer Zone (5.0m) (Measured from Top of Bank)
 - SuDS Maintenance Buffer Zone (3.0m) (Measured from Top of Bank)
 - Existing Watercourse (Top of Bank to Top of Bank)
 - Proposed Surface Water Sewer
 - Proposed Surface Water Manhole
 - Proposed Surface Water Outfall (Headwall)
 - Proposed Surface Water Flow Control
 - Proposed Permeable Paving (450mm Sub-Base)
 - Proposed Basin (Pond)
 - Proposed Watercourse
 - Existing Foul Water Drain/Sewer
 - Existing Foul Water Rising Main
 - Public Sewer Protected Strip
 - Foul Water (Private Drain/Public Sewer)
 - Proposed Private Pumping Station
 - Proposed Foul Water Rising Main
 - Existing Road Gully
 - Existing Highway Drainage

NOTES

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C	Final Issue	05/11/2021
B	Annotations updated	03/11/2021
A	Revised Foul Rising Main Line & Level	25/10/2021
-	Original Issue	18/10/2021

Rev.	Amendments	Date
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Drawing Status **PLANNING**

Client **Foreman Homes**

Project **Land at Summer Lane
Pagham, West Sussex**

Drawing Title **Proposed FW & SW Drainage Strategy
(Southern) Sheet 1 of 2**

Scale	Date	Drawn By	Checked By
1:500	Nov 21	DJL	MJA

Drawing No.	Rev.
D1915-300	C

Proposed Drainage @ 1:500
1:500 @ A1
1:1000 @ A3
0 5 10 15 20 25 30 35 40 45 50 m