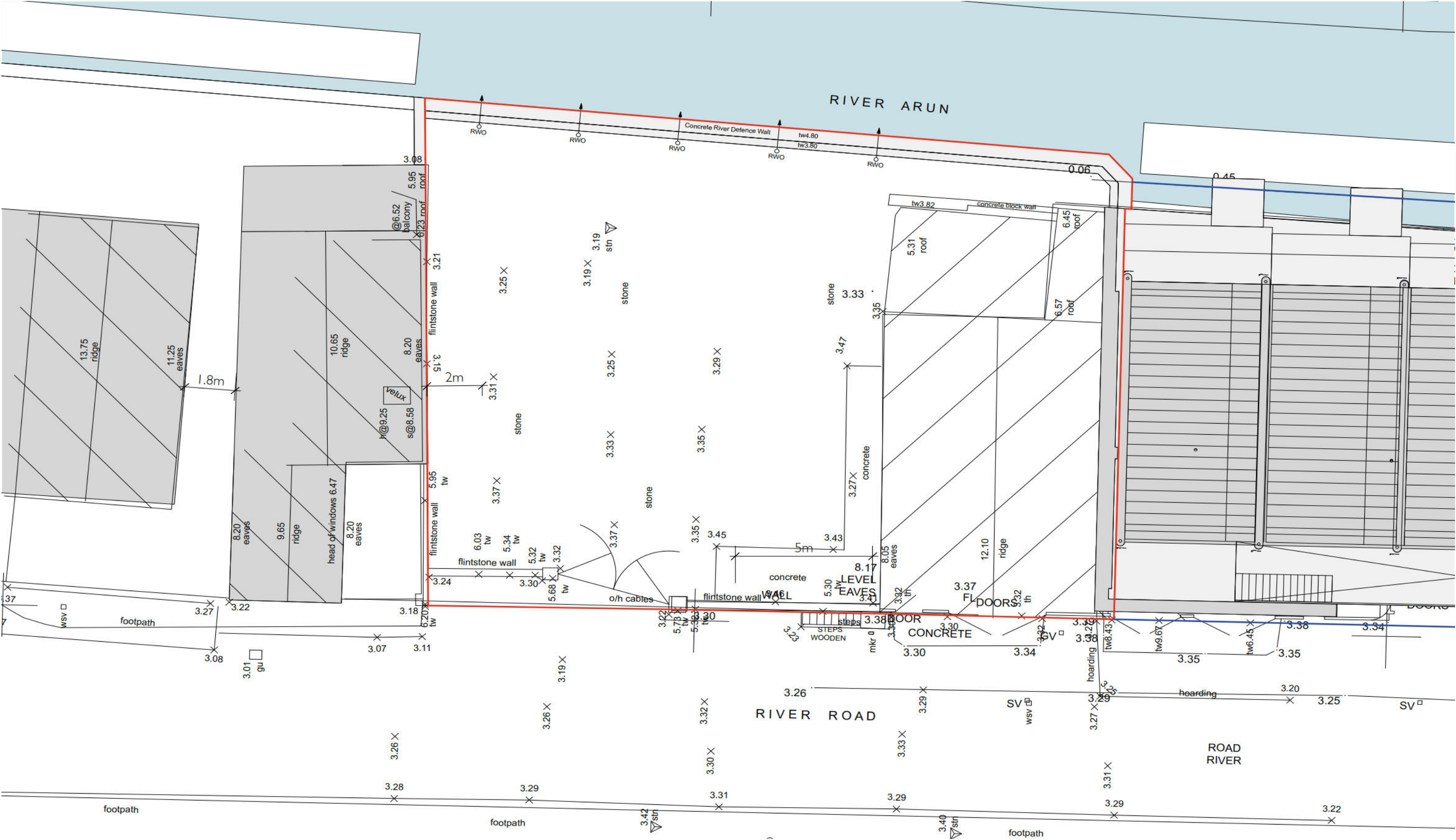


Appendix A

Topographic survey





Appendix B

Calculations



B.1 Climate Change Allowance

In the following calculations, a climate change allowance of 40% has been made which corresponds with the "Upper Limit" of the DEFRA guidance (refer to the table below):

Range	Total potential change anticipated for 2010-2039	Total potential change anticipated for 2040-2059	Total potential change anticipated for 2060-2115
Upper end	10%	20%	40%
Central	5%	10%	20%

B.2 Existing site surface water runo rates

The development site occupies a total area of 475 m² which is currently entirely hardstanding.

In accordance with the Modified Rational Method, the peak existing run-o from the site is calculated from the formula:

$$Q = 3.61 \times C_v \times A \times i$$

where C_v is the volumetric runo coefficient, A is the catchment area in hectares and i is the peak rainfall intensity in mm / hr.

For this site, the following site characteristics have been used in the calculation:

- M5-60min = 19 mm
- Ratio r = 0.35
- Peak run o rates have been based on a 15 minute rainfall event
- Impermeable Area Factor = 1.00
- Permeable Area Factor = 0.00
- $C_v = 0.95$

For the peak return period storm events this gives existing discharge rates from the site of:

$$Q_1 = 3.61 \times 0.95 \times 0.0475 \times 33.3 = \mathbf{5.4 \text{ litres / sec}}$$

$$Q_{30} = 3.61 \times 0.95 \times 0.0475 \times 68.8 = \mathbf{11.2 \text{ litres / sec}}$$

$$Q_{100} = 3.61 \times 0.95 \times 0.0475 \times 89.0 = \mathbf{14.5 \text{ litres / sec}}$$

$$Q_{100+cc} = 3.61 \times 0.95 \times 0.0475 \times 124.6 = \mathbf{20.3 \text{ litres / sec}}$$

B.3 Existing Discharge Volumes

The existing discharge volumes have been calculated in accordance with BS 8582 which requires the volume for the 6 hour storm event to be considered. The volume discharged during the storm event is calculated from the formula:

$$Vol_x = A \times i \times 6 \text{ hrs}$$

where A is the existing hardstanding area in m² and i is the X-year, 6 hour event rainfall intensity in mm / hr.

$$Vol_1 = 0.475 \times 3.90 \times 6 = \mathbf{11.1 \text{ m}^3}$$

$$Vol_{30} = 0.475 \times 8.11 \times 6 = \mathbf{23.1 \text{ m}^3}$$

$$Vol_{100} = 0.475 \times 10.51 \times 6 = \mathbf{30.0 \text{ m}^3}$$

$$Vol_{100+cc} = 0.475 \times 14.71 \times 6 = \mathbf{41.9 \text{ m}^3}$$

B.4 Existing Runo Summary

The existing runo values are summarised below:

Return Period	Existing peak runo rate	Existing Discharge Volume
1 year	5.4 litres/sec	11.1 m³
30 year	11.2 litres/sec	23.1 m³
100 year	14.5 litres/sec	30.0 m³
100 year (+40%)	20.3 litres/sec	41.9 m³

B.5 Greenfield Runo Rates

In accordance with the method outlined in the Institute of Hydrology Report 124, the Greenfield runo for the site is calculated from the formula:

$$Q_{BAR} = 0.00108 \times AREA^{0.89} \times SAAR^{1.17} \times SOIL^{2.17}$$

where AREA is the site area in km² (pro rata of 50 ha if the site is less than 50 ha), SAAR is the Standard Average Annual Rainfall in mm and SOIL is the Soil Index. For this site, the following site characteristics have been used in the calculation:

- IH 124 Soil Type = 2, therefore, SOIL = 0.3
- Standard Annual Average Rainfall (SAAR) = 700 mm
- Hydrological Growth Curve Region = 6/7 (South East England)

For a 50 ha site this gives a greenfield runo rate of:

$$Q_{BAR} = 0.00108 \times 0.5^{0.89} \times 700^{1.17} \times 0.30^{2.17} = \mathbf{91.1 \text{ litres / sec}}$$

Scaling this for the actual site area gives:

$$Q_{BAR} = (91.1 \times 0.0475) \div 50 = \mathbf{0.09 \text{ litres / sec}}$$

Using the Hydrological Growth Curve for south east England, gives:

$$Q_1 = 0.850 \times 0.09 = \mathbf{0.08 \text{ litres / sec}}$$

$$Q_{30} = 2.400 \times 0.09 = \mathbf{0.22 \text{ litres / sec}}$$

$$Q_{100} = 3.190 \times 0.09 = \mathbf{0.29 \text{ litres / sec}}$$

$$Q_{100+cc} = 3.61 \times 0.09 = \mathbf{0.32 \text{ litres / sec}}$$

B.6 Proposed Unattenuated Surface Water Runo Rates

The proposed development will incorporate approximately 35 m² of soft landscaping which leaves 440 m² of hardstanding. Again using the Modified Rational Method, the proposed (unattenuated) peak run-o from the site for the 1-in-1-year return period storm would be:

$$Q_1 = 3.61 \times 0.95 \times 0.0440 \times 33.3 = \mathbf{5.0 \text{ litres / sec}}$$

$$Q_{30} = 3.61 \times 0.95 \times 0.0440 \times 68.8 = \mathbf{10.4 \text{ litres / sec}}$$

$$Q_{100} = 3.61 \times 0.95 \times 0.0440 \times 89.0 = \mathbf{13.4 \text{ litres / sec}}$$

$$Q_{100+cc} = 3.61 \times 0.95 \times 0.0440 \times 124.6 = \mathbf{18.8 \text{ litres / sec}}$$

B.7 Proposed Attenuated Surface Water Runo Rates

In accordance with the Environment Agency's guidelines, the Building Regulations and the Water Authority's advice, the preferred means of surface water drainage for any new development is into a suitable soakaway or infiltration drainage system. Sustainable Urban Drainage Systems (SuDS) can reduce the impact of urbanisation on watercourse flows, ensure the protection and enhancement of water quality and encourage recharging of groundwater in a manner which mimics nature.

In addition to this, the National Planning Policy Framework requires that surface water arising from a developed site should, as far as is practicable, be managed in a sustainable manner to mimic surface water flows arising from the site prior to the proposed development, whilst reducing flood risk to the site itself and elsewhere, taking climate change into account.

Therefore, as an absolute minimum, the proposed site discharge under the 1-in-100-year storm plus climate change should be no greater than the existing 1-in-100-year storm discharge (i.e. mitigate the impact of climate change and any increase in the area of hardstanding). In this case, this would mean that, rather than discharging **20.3 litres / sec**, the maximum permissible discharge from the site would be **14.5 litres / sec**.

Further to the above, the London Plan's Policy SI 13 states that "Development proposals should aim to achieve greenfield run-o rates and ensure that surface water run-o is managed as close to its source as possible". The Environment Agency (EA) suggests that Developers should aim to achieve greenfield run o from their site.

However, Clause 17 of the DEFRA / EA publication 'Rainfall runo management for developments' states that "A practicable minimum limit on the discharge rate from a flow attenuation device is often

a compromise between attenuating to a satisfactorily low flow rate while keeping the risk of blockage to an acceptable level.

This limit is set at 5 litres per second, using an appropriate vortex or other flow control device. Where sedimentation could be an issue, the minimum size of orifice for controlling flow from an attenuation device should normally be 150 mm laid at a gradient not flatter than 1 in 150, which meets the requirements of Sewers for Adoption 7th Edition".

Based on the above considerations, it is proposed to limit the post-development runo rate from the roof area to a maximum of 1.6 litres/sec (approximately 30% of the existing 1 in 1 year rate) under the 1 in 100 year plus 40% climate change event as any less than this is considered impractical to implement. The corresponding rates for shorter return period storms are summarised below:

$$Q_1 = \mathbf{0.8 \text{ litres / sec}}$$

$$Q_{30} = \mathbf{1.4 \text{ litres / sec}}$$

$$Q_{100} = \mathbf{1.6 \text{ litres / sec}}$$

$$Q_{100+cc} = \mathbf{1.6 \text{ litres / sec}}$$

B.8 Proposed Discharge Volumes

The existing discharge volumes have been calculated in accordance with BS 8582 which requires the volume for the 6 hour storm event to be considered. The volume discharged during the storm event is calculated from the formula:

$$Vol_x = A \times i \times 6 \text{ hrs}$$

where A is the existing hardstanding area in hectares and i is the X-year, 6 hour event rainfall intensity in mm / hr.

$$Vol_1 = 0.440 \times 3.90 \times 6 = \mathbf{10.3 \text{ m}^3}$$

$$Vol_{30} = 0.440 \times 8.11 \times 6 = \mathbf{21.4 \text{ m}^3}$$

$$Vol_{100} = 0.440 \times 10.51 \times 6 = \mathbf{27.7 \text{ m}^3}$$

$$Vol_{100+cc} = 0.440 \times 14.71 \times 6 = \mathbf{38.8 \text{ m}^3}$$

B.9 Network Design

The InfoDrainage results contained on the following pages demonstrate that all water is contained within the proposed drainage system for every event up to and including the 1 in 100 year plus 40% climate change allowance event.

B.10 Summary

The proposed and existing runo values are summarised below:

Return Period	Existing Peak run o rate² (litres/sec)	Proposed scheme unattenuated peak run o rate (litres/sec)	Greenfield run o rate (litres/sec)	Proposed Design Peak run o rate (litres/sec)	Existing Discharge Volume (m³)	Proposed Discharge Volume (m³)
1 in 1 year	5.4	5.0	0.08	0.8	11.1	10.3
1 in 30 years	11.2	10.4	0.22	1.4	23.1	21.4
1 in 100 years	14.5	13.4	0.29	1.6	30.0	27.7
1 in 100 years (+40%)	20.3	18.8	0.32	1.6	41.9	38.8

Project: 57 River Road Littlehampton	Date: 16/06/2026			
	Designed by: MJD	Checked by: HL	Approved By: HL	
Report Details: Type: Inflows Storm Phase: Phase	Company Address: AKT II, White Collar Factory 1 Old Street Yard London EC1Y 8AF			

Main Roof

Type : Catchment Area

Area (m²)	310.37
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Dynamic Sizing	
Runoff Method	Time of Concentration
Summer Volumetric Runoff Coefficient	0.950
Winter Volumetric Runoff Coefficient	0.950
Time of Concentration (mins)	5
Percentage Impervious (%)	100

Hardstanding Area 01

Type : Catchment Area

Area (m²)	12.486
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Dynamic Sizing	
Runoff Method	Time of Concentration
Summer Volumetric Runoff Coefficient	0.950
Winter Volumetric Runoff Coefficient	0.950
Time of Concentration (mins)	5
Percentage Impervious (%)	100

Hardstanding Area 03

Type : Catchment Area

Area (m²)	26.61
-----------	-------

Dynamic Sizing	
Runoff Method	Time of Concentration
Summer Volumetric Runoff Coefficient	0.950
Winter Volumetric Runoff Coefficient	0.950
Time of Concentration (mins)	5
Percentage Impervious (%)	100

Car Parking Area 01

Type : Catchment Area

Area (m²)	31.661
-----------	--------

Dynamic Sizing	
Runoff Method	Time of Concentration
Summer Volumetric Runoff Coefficient	0.950
Winter Volumetric Runoff Coefficient	0.950
Time of Concentration (mins)	5
Percentage Impervious (%)	100

Project: 57 River Road Littlehampton	Date: 16/06/2026			
	Designed by: MJD	Checked by: HL	Approved By: HL	
Report Details: Type: Inflows Storm Phase: Phase	Company Address: AKT II, White Collar Factory 1 Old Street Yard London EC1Y 8AF			

Car Parking Area 02

Type : Catchment Area

Area (m²)	42.687
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Dynamic Sizing	
Runoff Method	Time of Concentration
Summer Volumetric Runoff Coefficient	0.950
Winter Volumetric Runoff Coefficient	0.950
Time of Concentration (mins)	5
Percentage Impervious (%)	100



Project: 57 River Road Littlehampton	Date: 16/06/2026			
	Designed by: MJD	Checked by: HL	Approved By: HL	
Report Details: Type: Stormwater Controls Storm Phase: Phase			Company Address: AKT II, White Collar Factory 1 Old Street Yard London EC1Y 8AF	



Porous Paving Area 02

Type : Porous Paving

Dimensions

Exceedance Level (m)	5.000
Depth (m)	0.500
Base Level (m)	4.500
Paving Layer Depth (mm)	100
Membrane Percolation (m/s)	0.00083333
Porosity (%)	30
Length (m)	15.749
Long. Slope (1:X)	10000.00
Width (m)	4.572
Total Volume (m³)	8.640

Inlets

Inlet (2)

Inlet Type	Point Inflow
Incoming Item(s)	Hardstanding Area 03
Bypass Destination	(None)
Capacity Type	No Restriction

Inlet (3)

Inlet Type	Lateral Inflow
Incoming Item(s)	Car Parking Area 02
Bypass Destination	(None)
Capacity Type	No Restriction

Outlets

Outlet

Outgoing Connection	Porous Parking Area 02
Outlet Type	Orifice
Diameter (m)	0.100
Coefficient of Discharge	0.600
Invert Level (m)	4.500

Advanced

Conductivity (m/s)	0.01388889
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Project: 57 River Road Littlehampton	Date: 16/06/2026			
	Designed by: MJD	Checked by: HL	Approved By: HL	
Report Details: Type: Stormwater Controls Storm Phase: Phase			Company Address: AKT II, White Collar Factory 1 Old Street Yard London EC1Y 8AF	



Porous Paving Area 01

Type : Porous Paving

Dimensions

Exceedance Level (m)	5.000
Depth (m)	0.500
Base Level (m)	4.500
Paving Layer Depth (mm)	100
Membrane Percolation (m/s)	0.00083333
Porosity (%)	30
Length (m)	8.904
Long. Slope (1:X)	10000.00
Width (m)	4.043
Total Volume (m³)	4.320

Inlets

Inlet

Inlet Type	Lateral Inflow
Incoming Item(s)	Car Parking Area 01
Bypass Destination	(None)
Capacity Type	No Restriction

Outlets

Outlet

Outgoing Connection	Porous Parking Area 01
Outlet Type	Orifice
Diameter (m)	0.100
Coefficient of Discharge	0.600
Invert Level (m)	4.500

Advanced

Conductivity (m/s)	0.01388889
--------------------	------------

Project: 57 River Road Littlehampton	Date: 16/06/2026			
	Designed by: MJD	Checked by: HL	Approved By: HL	
Report Details: Type: Stormwater Controls Storm Phase: Phase	Company Address: AKT II, White Collar Factory 1 Old Street Yard London EC1Y 8AF			

 Cellular Storage

Type : Cellular Storage

Dimensions

Exceedance Level (m)	5.000
Depth (m)	0.800
Base Level (m)	3.700
Number of Crates Long	6
Number of Crates Wide	8
Number of Crates High	2
Porosity (%)	95
Crate Length (m)	1
Crate Width (m)	0.5
Crate Height (m)	0.4
Total Volume (m³)	18.740

Inlets

Inlet (1)

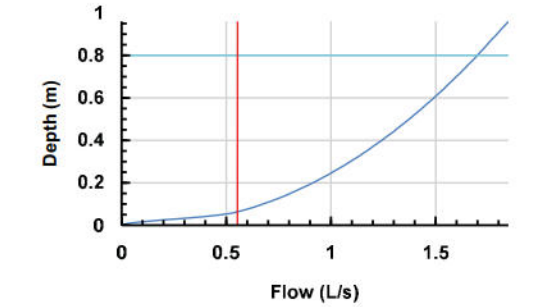
Inlet Type	Point Inflow Main Roof
Incoming Item(s)	Hardstanding Area 01 Porous Parking Area 02 Porous Parking Area 01
Bypass Destination	(None)
Capacity Type	No Restriction

Outlets

Outlet

Outgoing Connection	(None)
Outlet Type	Hydro-Brake®
Invert Level (m)	3.700
Design Depth (m)	0.800
Design Flow (L/s)	1.7
Objective	Linear Discharge Profile
Application	Surface Water Only
Sump Available	☒

Unit Reference	SCU-0042-1700-0800-1700
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Project: 57 River Road Littlehampton	Date: 16/06/2026			
	Designed by: MJD	Checked by: HL	Approved By: HL	
Report Details: Type: Connections Storm Phase: Phase	Company Address: AKT II, White Collar Factory 1 Old Street Yard London EC1Y 8AF			

Name	Length (m)	Connection Type	Slope (1:X)	Manning's n	Colebrook-White Roughness (mm)	Diameter / Base Width (mm)	Height (mm)	Side Slope (1:X)
Porous Parking Area 01	3.103	Pipe	31.034		0.6	100		
Porous Parking Area 02	3.258	Pipe	32.581		0.6	100		

Name	Curved	Upstream Cover Level (m)	Upstream Invert Level (m)	Downstream Cover Level (m)	Downstream Invert Level (m)	Lock	Flow Restriction (L/s)
Porous Parking Area 01	☐	5.000	4.500	5.000	4.400	Levels	12.3
Porous Parking Area 02	☐	5.000	4.500	5.000	4.400	Levels	12.3

Project: 57 River Road Littlehampton	Date: 16/06/2026			
	Designed by: MJD	Checked by: HL	Approved By: HL	
Report Details: Type: Network Design Criteria Storm Phase: Phase		Company Address: AKT II, White Collar Factory 1 Old Street Yard London EC1Y 8AF		

Flow Options	
Peak Flow Calculation	(UK) Modified Rational Method
Min. Time of Entry (mins)	5
Max. Travel Time (mins)	30

Pipe Options	
Lock Slope Options	None
Design Options	Minimise Excavation
Design Level	Level Soffits
Min. Cover Depth (m)	1.200
Min. Slope (1:X)	500.00
Max. Slope (1:X)	40.00
Min. Velocity (m/s)	1.0
Max. Velocity (m/s)	3.0
Use Flow Restriction	☐
Reduce Channel Depths	☐

Manhole Options	
Apply Offset	☐

Project: 57 River Road Littlehampton	Date: 16/06/2026			
	Designed by: MJD	Checked by: HL	Approved By: HL	
Report Title: Rainfall Analysis Criteria		Company Address: AKT II, White Collar Factory 1 Old Street Yard London EC1Y 8AF		

Runoff Type	Dynamic
Output Interval (mins)	5
Time Step	Default
Urban Creep	Apply Global Value
Urban Creep Global Value (%)	0
Junction Flood Risk Margin (mm)	300
Perform No Discharge Analysis	☐

Rainfall	
FEH	Type: FEH
Site Location	GB 502359 102133 TQ 02359 02133
Rainfall Version	2022
Summer	☒
Winter	☒

Return Period	
Return Period (years)	Increase Rainfall (%)
2.0	40.000
30.0	40.000
100.0	40.000

Storm Durations	
Duration (mins)	Run Time (mins)
15	30
30	60
60	120
120	240
180	360
240	480
360	720
480	960
600	1200
720	1440
960	1920
1440	2880

Project: 57 River Road Littlehampton	Date: 16/06/2026			
	Designed by: MJD	Checked by: HL	Approved By: HL	
Report Details: Type: Stormwater Controls Summary Storm Phase: Phase	Company Address: AKT II, White Collar Factory 1 Old Street Yard London EC1Y 8AF			



FEH: 2 years: Increase Rainfall (%): +40: Summary Results for Cellular Storage: Rank By: Max. US Depth

Storm Event	Max. US Level (m)	Max. DS Level (m)	Max. US Depth (m)	Max. DS Depth (m)	Max. Inflow (L/s)	Max. Resident Volume (m³)	Max. Flooded Volume (m³)	Total Lost Volume (m³)	Max. Outflow (L/s)	Total Discharge Volume (m³)	Percentage Available (%)	Status
FEH: 2 years: +40 %: 15 mins: Summer	3.855	3.855	0.155	0.155	9.3	3.536	0.000	0.000	0.8	0.994	81.132	OK
FEH: 2 years: +40 %: 15 mins: Winter	3.855	3.855	0.155	0.155	8.7	3.539	0.000	0.000	0.8	0.999	81.114	OK
FEH: 2 years: +40 %: 30 mins: Summer	3.894	3.894	0.194	0.194	6.2	4.410	0.000	0.000	0.9	2.281	76.465	OK
FEH: 2 years: +40 %: 30 mins: Winter	3.893	3.893	0.193	0.193	5.9	4.408	0.000	0.000	0.9	2.288	76.481	OK
FEH: 2 years: +40 %: 60 mins: Summer	3.920	3.920	0.220	0.220	5.5	5.022	0.000	0.000	0.9	4.758	73.200	OK
FEH: 2 years: +40 %: 60 mins: Winter	3.920	3.920	0.220	0.220	4.5	5.027	0.000	0.000	0.9	4.770	73.177	OK
FEH: 2 years: +40 %: 120 mins: Summer	3.956	3.956	0.256	0.256	4.5	5.828	0.000	0.000	1.0	9.075	68.898	OK
FEH: 2 years: +40 %: 120 mins: Winter	3.952	3.952	0.252	0.252	3.2	5.753	0.000	0.000	1.0	9.094	69.303	OK
FEH: 2 years: +40 %: 180 mins: Summer	3.961	3.961	0.261	0.261	3.7	5.958	0.000	0.000	1.0	11.172	68.207	OK
FEH: 2 years: +40 %: 180 mins: Winter	3.952	3.952	0.252	0.252	2.5	5.751	0.000	0.000	1.0	11.194	69.313	OK
FEH: 2 years: +40 %: 240 mins: Summer	3.956	3.956	0.256	0.256	3.1	5.837	0.000	0.000	1.0	12.421	68.854	OK
FEH: 2 years: +40 %: 240 mins: Winter	3.940	3.940	0.240	0.240	2.1	5.470	0.000	0.000	0.9	12.411	70.810	OK
FEH: 2 years: +40 %: 360 mins: Summer	3.935	3.935	0.235	0.235	2.4	5.364	0.000	0.000	0.9	14.164	71.376	OK
FEH: 2 years: +40 %: 360 mins: Winter	3.910	3.910	0.210	0.210	1.6	4.777	0.000	0.000	0.9	14.165	74.511	OK
FEH: 2 years: +40 %: 480 mins: Summer	3.912	3.912	0.212	0.212	2.0	4.832	0.000	0.000	0.9	15.428	74.215	OK
FEH: 2 years: +40 %: 480 mins: Winter	3.878	3.878	0.178	0.178	1.3	4.068	0.000	0.000	0.8	15.430	78.292	OK
FEH: 2 years: +40 %: 600 mins: Summer	3.891	3.891	0.191	0.191	1.7	4.353	0.000	0.000	0.8	16.413	76.771	OK
FEH: 2 years: +40 %: 600 mins: Winter	3.853	3.853	0.153	0.153	1.1	3.481	0.000	0.000	0.8	16.437	81.423	OK
FEH: 2 years: +40 %: 720 mins: Summer	3.873	3.873	0.173	0.173	1.4	3.934	0.000	0.000	0.8	17.239	79.009	OK
FEH: 2 years: +40 %: 720 mins: Winter	3.830	3.830	0.130	0.130	0.9	2.968	0.000	0.000	0.7	17.239	84.161	OK
FEH: 2 years: +40 %: 960 mins: Summer	3.842	3.842	0.142	0.142	1.2	3.229	0.000	0.000	0.7	18.584	82.770	OK
FEH: 2 years: +40 %: 960 mins: Winter	3.798	3.798	0.098	0.098	0.8	2.229	0.000	0.000	0.6	18.641	88.104	OK
FEH: 2 years: +40 %: 1440 mins: Summer	3.802	3.802	0.102	0.102	0.8	2.318	0.000	0.000	0.6	20.958	87.630	OK
FEH: 2 years: +40 %: 1440 mins: Winter	3.762	3.762	0.062	0.062	0.6	1.413	0.000	0.000	0.5	20.779	92.462	OK

Project: 57 River Road Littlehampton	Date: 16/06/2026			
	Designed by: MJD	Checked by: HL	Approved By: HL	
Report Details: Type: Stormwater Controls Summary Storm Phase: Phase	Company Address: AKT II, White Collar Factory 1 Old Street Yard London EC1Y 8AF			



FEH: 2 years: Increase Rainfall (%): +40: Summary Results for Porous Paving Area 02: Rank By: Max. US Depth

Storm Event	Max. US Level (m)	Max. DS Level (m)	Max. US Depth (m)	Max. DS Depth (m)	Max. Inflow (L/s)	Max. Resident Volume (m³)	Max. Flooded Volume (m³)	Total Lost Volume (m³)	Max. Outflow (L/s)	Total Discharge Volume (m³)	Percentage Available (%)	Status
FEH: 2 years: +40 %: 15 mins: Summer	4.571	4.519	0.069	0.019	2.0	0.804	0.000	0.000	0.1	0.083	90.694	OK
FEH: 2 years: +40 %: 15 mins: Winter	4.571	4.519	0.069	0.019	1.9	0.803	0.000	0.000	0.1	0.083	90.711	OK
FEH: 2 years: +40 %: 30 mins: Summer	4.592	4.522	0.091	0.022	1.3	1.018	0.000	0.000	0.1	0.169	88.214	OK
FEH: 2 years: +40 %: 30 mins: Winter	4.593	4.521	0.091	0.021	1.2	1.024	0.000	0.000	0.1	0.169	88.143	OK
FEH: 2 years: +40 %: 60 mins: Summer	4.614	4.523	0.113	0.023	1.1	1.224	0.000	0.000	0.1	0.266	85.831	OK
FEH: 2 years: +40 %: 60 mins: Winter	4.615	4.522	0.113	0.022	0.9	1.235	0.000	0.000	0.1	0.270	85.705	OK
FEH: 2 years: +40 %: 120 mins: Summer	4.648	4.523	0.147	0.023	0.9	1.585	0.000	0.000	0.1	0.416	81.660	OK
FEH: 2 years: +40 %: 120 mins: Winter	4.650	4.521	0.148	0.021	0.7	1.599	0.000	0.000	0.1	0.418	81.499	OK
FEH: 2 years: +40 %: 180 mins: Summer	4.665	4.522	0.163	0.022	0.8	1.799	0.000	0.000	0.1	0.544	79.179	OK
FEH: 2 years: +40 %: 180 mins: Winter	4.666	4.520	0.165	0.020	0.5	1.813	0.000	0.000	0.1	0.548	79.022	OK
FEH: 2 years: +40 %: 240 mins: Summer	4.673	4.521	0.172	0.021	0.6	1.932	0.000	0.000	0.1	0.666	77.634	OK
FEH: 2 years: +40 %: 240 mins: Winter	4.675	4.518	0.174	0.018	0.4	1.951	0.000	0.000	0.1	0.671	77.417	OK
FEH: 2 years: +40 %: 360 mins: Summer	4.681	4.518	0.179	0.018	0.5	2.109	0.000	0.000	0.1	0.922	75.595	OK
FEH: 2 years: +40 %: 360 mins: Winter	4.681	4.516	0.179	0.016	0.3	2.094	0.000	0.000	0.1	0.913	75.759	OK
FEH: 2 years: +40 %: 480 mins: Summer	4.683	4.517	0.181	0.017	0.4	2.162	0.000	0.000	0.1	1.140	74.974	OK
FEH: 2 years: +40 %: 480 mins: Winter	4.686	4.514	0.185	0.014	0.3	2.223	0.000	0.000	0.1	1.172	74.270	OK
FEH: 2 years: +40 %: 600 mins: Summer	4.685	4.515	0.184	0.015	0.3	2.223	0.000	0.000	0.1	1.374	74.273	OK
FEH: 2 years: +40 %: 600 mins: Winter	4.685	4.513	0.184	0.013	0.2	2.211	0.000	0.000	0.0	1.349	74.407	OK
FEH: 2 years: +40 %: 720 mins: Summer	4.688	4.515	0.186	0.015	0.3	2.264	0.000	0.000	0.1	1.588	73.792	OK
FEH: 2 years: +40 %: 720 mins: Winter	4.686	4.512	0.185	0.012	0.2	2.247	0.000	0.000	0.0	1.550	73.991	OK
FEH: 2 years: +40 %: 960 mins: Summer	4.688	4.513	0.187	0.013	0.2	2.279	0.000	0.000	0.0	1.908	73.620	OK
FEH: 2 years: +40 %: 960 mins: Winter	4.689	4.511	0.187	0.011	0.2	2.322	0.000	0.000	0.0	1.964	73.131	OK
FEH: 2 years: +40 %: 1440 mins: Summer	4.692	4.511	0.191	0.011	0.2	2.360	0.000	0.000	0.0	2.572	72.681	OK
FEH: 2 years: +40 %: 1440 mins: Winter	4.689	4.510	0.187	0.010	0.1	2.345	0.000	0.000	0.0	2.497	72.853	OK

Project: 57 River Road Littlehampton	Date: 16/06/2026			
	Designed by: MJD	Checked by: HL	Approved By: HL	
Report Details: Type: Stormwater Controls Summary Storm Phase: Phase	Company Address: AKT II, White Collar Factory 1 Old Street Yard London EC1Y 8AF			



FEH: 2 years: Increase Rainfall (%): +40: Summary Results for Porous Paving Area 01: Rank By: Max. US Depth

Storm Event	Max. US Level (m)	Max. DS Level (m)	Max. US Depth (m)	Max. DS Depth (m)	Max. Inflow (L/s)	Max. Resident Volume (m³)	Max. Flooded Volume (m³)	Total Lost Volume (m³)	Max. Outflow (L/s)	Total Discharge Volume (m³)	Percentage Available (%)	Status
FEH: 2 years: +40 %: 15 mins: Summer	4.537	4.526	0.036	0.026	0.9	0.317	0.000	0.000	0.1	0.120	92.655	OK
FEH: 2 years: +40 %: 15 mins: Winter	4.537	4.525	0.036	0.025	0.8	0.314	0.000	0.000	0.1	0.120	92.729	OK
FEH: 2 years: +40 %: 30 mins: Summer	4.548	4.528	0.047	0.028	0.6	0.379	0.000	0.000	0.1	0.217	91.230	OK
FEH: 2 years: +40 %: 30 mins: Winter	4.548	4.527	0.047	0.027	0.6	0.379	0.000	0.000	0.1	0.217	91.216	OK
FEH: 2 years: +40 %: 60 mins: Summer	4.560	4.528	0.060	0.028	0.5	0.421	0.000	0.000	0.2	0.319	90.256	OK
FEH: 2 years: +40 %: 60 mins: Winter	4.560	4.527	0.060	0.027	0.4	0.421	0.000	0.000	0.1	0.319	90.261	OK
FEH: 2 years: +40 %: 120 mins: Summer	4.579	4.528	0.078	0.028	0.4	0.491	0.000	0.000	0.2	0.476	88.623	OK
FEH: 2 years: +40 %: 120 mins: Winter	4.579	4.525	0.079	0.025	0.3	0.490	0.000	0.000	0.1	0.476	88.649	OK
FEH: 2 years: +40 %: 180 mins: Summer	4.589	4.526	0.088	0.026	0.3	0.527	0.000	0.000	0.1	0.592	87.803	OK
FEH: 2 years: +40 %: 180 mins: Winter	4.588	4.522	0.088	0.022	0.2	0.526	0.000	0.000	0.1	0.583	87.822	OK
FEH: 2 years: +40 %: 240 mins: Summer	4.594	4.524	0.093	0.024	0.3	0.550	0.000	0.000	0.1	0.683	87.278	OK
FEH: 2 years: +40 %: 240 mins: Winter	4.593	4.520	0.092	0.020	0.2	0.542	0.000	0.000	0.1	0.665	87.461	OK
FEH: 2 years: +40 %: 360 mins: Summer	4.598	4.521	0.098	0.021	0.2	0.570	0.000	0.000	0.1	0.828	86.813	OK
FEH: 2 years: +40 %: 360 mins: Winter	4.600	4.517	0.099	0.017	0.1	0.582	0.000	0.000	0.1	0.837	86.534	OK
FEH: 2 years: +40 %: 480 mins: Summer	4.601	4.519	0.100	0.019	0.2	0.583	0.000	0.000	0.1	0.959	86.508	OK
FEH: 2 years: +40 %: 480 mins: Winter	4.601	4.515	0.100	0.015	0.1	0.584	0.000	0.000	0.1	0.944	86.477	OK
FEH: 2 years: +40 %: 600 mins: Summer	4.601	4.517	0.100	0.017	0.2	0.587	0.000	0.000	0.1	1.031	86.423	OK
FEH: 2 years: +40 %: 600 mins: Winter	4.603	4.514	0.102	0.014	0.1	0.596	0.000	0.000	0.1	1.061	86.193	OK
FEH: 2 years: +40 %: 720 mins: Summer	4.602	4.516	0.101	0.016	0.1	0.590	0.000	0.000	0.1	1.107	86.334	OK
FEH: 2 years: +40 %: 720 mins: Winter	4.602	4.513	0.101	0.013	0.1	0.589	0.000	0.000	0.0	1.096	86.373	OK
FEH: 2 years: +40 %: 960 mins: Summer	4.603	4.514	0.102	0.014	0.1	0.599	0.000	0.000	0.1	1.252	86.132	OK
FEH: 2 years: +40 %: 960 mins: Winter	4.602	4.512	0.101	0.012	0.1	0.589	0.000	0.000	0.0	1.230	86.371	OK
FEH: 2 years: +40 %: 1440 mins: Summer	4.606	4.512	0.105	0.012	0.1	0.615	0.000	0.000	0.0	1.555	85.760	OK
FEH: 2 years: +40 %: 1440 mins: Winter	4.602	4.510	0.101	0.010	0.1	0.590	0.000	0.000	0.0	1.492	86.338	OK

Project: 57 River Road Littlehampton	Date: 16/06/2026			
	Designed by: MJD	Checked by: HL	Approved By: HL	
Report Details: Type: Stormwater Controls Summary Storm Phase: Phase	Company Address: AKT II, White Collar Factory 1 Old Street Yard London EC1Y 8AF			



FEH: 30 years: Increase Rainfall (%): +40: Summary Results for Cellular Storage: Rank By: Max. US Depth

Storm Event	Max. US Level (m)	Max. DS Level (m)	Max. US Depth (m)	Max. DS Depth (m)	Max. Inflow (L/s)	Max. Resident Volume (m³)	Max. Flooded Volume (m³)	Total Lost Volume (m³)	Max. Outflow (L/s)	Total Discharge Volume (m³)	Percentage Available (%)	Status
FEH: 30 years: +40 %: 15 mins: Summer	4.065	4.065	0.365	0.365	20.6	8.327	0.000	0.000	1.1	1.494	55.563	OK
FEH: 30 years: +40 %: 15 mins: Winter	4.065	4.065	0.365	0.365	19.4	8.323	0.000	0.000	1.1	1.501	55.588	OK
FEH: 30 years: +40 %: 30 mins: Summer	4.174	4.174	0.474	0.474	14.2	10.800	0.000	0.000	1.3	3.501	42.370	OK
FEH: 30 years: +40 %: 30 mins: Winter	4.173	4.173	0.473	0.473	13.4	10.796	0.000	0.000	1.3	3.507	42.393	OK
FEH: 30 years: +40 %: 60 mins: Summer	4.262	4.262	0.562	0.562	12.6	12.816	0.000	0.000	1.4	7.588	31.612	OK
FEH: 30 years: +40 %: 60 mins: Winter	4.264	4.264	0.564	0.564	10.2	12.853	0.000	0.000	1.4	7.595	31.413	OK
FEH: 30 years: +40 %: 120 mins: Summer	4.291	4.291	0.591	0.591	9.2	13.472	0.000	0.000	1.4	14.891	28.112	OK
FEH: 30 years: +40 %: 120 mins: Winter	4.290	4.290	0.590	0.590	6.7	13.442	0.000	0.000	1.4	14.891	28.273	OK
FEH: 30 years: +40 %: 180 mins: Summer	4.287	4.287	0.587	0.587	7.2	13.383	0.000	0.000	1.4	20.552	28.585	OK
FEH: 30 years: +40 %: 180 mins: Winter	4.280	4.280	0.580	0.580	5.0	13.226	0.000	0.000	1.4	20.552	29.423	OK
FEH: 30 years: +40 %: 240 mins: Summer	4.271	4.271	0.571	0.571	6.0	13.016	0.000	0.000	1.4	24.064	30.542	OK
FEH: 30 years: +40 %: 240 mins: Winter	4.257	4.257	0.557	0.557	4.1	12.700	0.000	0.000	1.4	24.105	32.232	OK
FEH: 30 years: +40 %: 360 mins: Summer	4.228	4.228	0.528	0.528	4.5	12.031	0.000	0.000	1.3	27.066	35.802	OK
FEH: 30 years: +40 %: 360 mins: Winter	4.197	4.197	0.497	0.497	3.0	11.339	0.000	0.000	1.3	27.056	39.494	OK
FEH: 30 years: +40 %: 480 mins: Summer	4.183	4.183	0.483	0.483	3.6	11.020	0.000	0.000	1.3	29.070	41.194	OK
FEH: 30 years: +40 %: 480 mins: Winter	4.138	4.138	0.438	0.438	2.4	9.997	0.000	0.000	1.2	29.079	46.653	OK
FEH: 30 years: +40 %: 600 mins: Summer	4.143	4.143	0.443	0.443	3.0	10.100	0.000	0.000	1.2	30.668	46.106	OK
FEH: 30 years: +40 %: 600 mins: Winter	4.087	4.087	0.387	0.387	2.0	8.832	0.000	0.000	1.2	30.650	52.869	OK
FEH: 30 years: +40 %: 720 mins: Summer	4.107	4.107	0.407	0.407	2.6	9.272	0.000	0.000	1.2	32.050	50.523	OK
FEH: 30 years: +40 %: 720 mins: Winter	4.043	4.043	0.343	0.343	1.7	7.818	0.000	0.000	1.1	31.995	58.284	OK
FEH: 30 years: +40 %: 960 mins: Summer	4.045	4.045	0.345	0.345	2.1	7.876	0.000	0.000	1.1	34.178	57.975	OK
FEH: 30 years: +40 %: 960 mins: Winter	3.971	3.971	0.271	0.271	1.4	6.187	0.000	0.000	1.0	34.191	66.986	OK
FEH: 30 years: +40 %: 1440 mins: Summer	3.960	3.960	0.260	0.260	1.5	5.921	0.000	0.000	1.0	37.285	68.405	OK
FEH: 30 years: +40 %: 1440 mins: Winter	3.879	3.879	0.179	0.179	1.0	4.074	0.000	0.000	0.8	37.310	78.263	OK

Project: 57 River Road Littlehampton	Date: 16/06/2026		
	Designed by: MJD	Checked by: HL	Approved By: HL
Report Details: Type: Stormwater Controls Summary Storm Phase: Phase	Company Address: AKT II, White Collar Factory 1 Old Street Yard London EC1Y 8AF		



FEH: 30 years: Increase Rainfall (%): +40: Summary Results for Perous Paving Area 02: Rank By: Max. US Depth

Storm Event	Max. US Level (m)	Max. DS Level (m)	Max. US Depth (m)	Max. DS Depth (m)	Max. Inflow (L/s)	Max. Resident Volume (m³)	Max. Flooded Volume (m³)	Total Lost Volume (m³)	Max. Outflow (L/s)	Total Discharge Volume (m³)	Percentage Available (%)	Status
FEH: 30 years: +40 %: 15 mins: Summer	4.655	4.539	0.153	0.039	4.4	1.751	0.000	0.000	0.2	0.233	79.730	OK
FEH: 30 years: +40 %: 15 mins: Winter	4.655	4.539	0.154	0.039	4.1	1.754	0.000	0.000	0.2	0.233	79.701	OK
FEH: 30 years: +40 %: 30 mins: Summer	4.704	4.543	0.202	0.043	2.9	2.241	0.000	0.000	0.3	0.439	74.062	OK
FEH: 30 years: +40 %: 30 mins: Winter	4.704	4.543	0.202	0.043	2.8	2.237	0.000	0.000	0.3	0.437	74.107	OK
FEH: 30 years: +40 %: 60 mins: Summer	4.750	4.545	0.248	0.045	2.6	2.711	0.000	0.000	0.3	0.666	68.620	OK
FEH: 30 years: +40 %: 60 mins: Winter	4.750	4.542	0.249	0.042	2.1	2.717	0.000	0.000	0.3	0.663	68.554	OK
FEH: 30 years: +40 %: 120 mins: Summer	4.785	4.541	0.283	0.041	1.9	3.169	0.000	0.000	0.3	0.990	63.324	OK
FEH: 30 years: +40 %: 120 mins: Winter	4.786	4.536	0.285	0.036	1.3	3.179	0.000	0.000	0.2	0.989	63.207	OK
FEH: 30 years: +40 %: 180 mins: Summer	4.799	4.537	0.297	0.037	1.5	3.444	0.000	0.000	0.2	1.324	60.136	OK
FEH: 30 years: +40 %: 180 mins: Winter	4.800	4.532	0.299	0.032	1.0	3.442	0.000	0.000	0.2	1.314	60.167	OK
FEH: 30 years: +40 %: 240 mins: Summer	4.802	4.534	0.300	0.034	1.2	3.586	0.000	0.000	0.2	1.652	58.500	OK
FEH: 30 years: +40 %: 240 mins: Winter	4.805	4.529	0.303	0.029	0.8	3.599	0.000	0.000	0.2	1.649	58.343	OK
FEH: 30 years: +40 %: 360 mins: Summer	4.807	4.529	0.305	0.029	0.9	3.723	0.000	0.000	0.2	2.253	56.908	OK
FEH: 30 years: +40 %: 360 mins: Winter	4.807	4.524	0.305	0.024	0.6	3.732	0.000	0.000	0.1	2.233	56.807	OK
FEH: 30 years: +40 %: 480 mins: Summer	4.808	4.526	0.307	0.026	0.7	3.751	0.000	0.000	0.1	2.757	56.584	OK
FEH: 30 years: +40 %: 480 mins: Winter	4.808	4.522	0.307	0.022	0.5	3.784	0.000	0.000	0.1	2.748	56.202	OK
FEH: 30 years: +40 %: 600 mins: Summer	4.808	4.524	0.307	0.024	0.6	3.760	0.000	0.000	0.1	3.174	56.485	OK
FEH: 30 years: +40 %: 600 mins: Winter	4.808	4.520	0.306	0.020	0.4	3.797	0.000	0.000	0.1	3.189	56.054	OK
FEH: 30 years: +40 %: 720 mins: Summer	4.809	4.523	0.308	0.023	0.5	3.798	0.000	0.000	0.1	3.595	56.041	OK
FEH: 30 years: +40 %: 720 mins: Winter	4.803	4.519	0.302	0.019	0.3	3.771	0.000	0.000	0.1	3.536	56.356	OK
FEH: 30 years: +40 %: 960 mins: Summer	4.802	4.520	0.301	0.020	0.4	3.755	0.000	0.000	0.1	4.100	56.542	OK
FEH: 30 years: +40 %: 960 mins: Winter	4.799	4.518	0.298	0.018	0.3	3.773	0.000	0.000	0.1	4.160	56.326	OK
FEH: 30 years: +40 %: 1440 mins: Summer	4.795	4.518	0.293	0.018	0.3	3.725	0.000	0.000	0.1	4.976	56.888	OK
FEH: 30 years: +40 %: 1440 mins: Winter	4.786	4.517	0.285	0.017	0.2	3.663	0.000	0.000	0.1	4.895	57.604	OK

Project: 57 River Road Littlehampton	Date: 16/06/2026		
	Designed by: MJD	Checked by: HL	Approved By: HL
Report Details: Type: Stormwater Controls Summary Storm Phase: Phase	Company Address: AKT II, White Collar Factory 1 Old Street Yard London EC1Y 8AF		



FEH: 30 years: Increase Rainfall (%): +40: Summary Results for Perous Paving Area 01: Rank By: Max. US Depth

Storm Event	Max. US Level (m)	Max. DS Level (m)	Max. US Depth (m)	Max. DS Depth (m)	Max. Inflow (L/s)	Max. Resident Volume (m³)	Max. Flooded Volume (m³)	Total Lost Volume (m³)	Max. Outflow (L/s)	Total Discharge Volume (m³)	Percentage Available (%)	Status
FEH: 30 years: +40 %: 15 mins: Summer	4.581	4.551	0.080	0.051	2.0	0.673	0.000	0.000	0.4	0.319	84.413	OK
FEH: 30 years: +40 %: 15 mins: Winter	4.581	4.550	0.080	0.050	1.9	0.664	0.000	0.000	0.4	0.317	84.637	OK
FEH: 30 years: +40 %: 30 mins: Summer	4.606	4.553	0.105	0.053	1.3	0.796	0.000	0.000	0.4	0.534	81.573	OK
FEH: 30 years: +40 %: 30 mins: Winter	4.607	4.552	0.106	0.052	1.3	0.792	0.000	0.000	0.4	0.537	81.674	OK
FEH: 30 years: +40 %: 60 mins: Summer	4.633	4.553	0.132	0.053	1.2	0.881	0.000	0.000	0.4	0.770	79.618	OK
FEH: 30 years: +40 %: 60 mins: Winter	4.633	4.550	0.132	0.050	1.0	0.877	0.000	0.000	0.4	0.770	79.689	OK
FEH: 30 years: +40 %: 120 mins: Summer	4.653	4.548	0.152	0.048	0.9	0.929	0.000	0.000	0.3	1.050	78.485	OK
FEH: 30 years: +40 %: 120 mins: Winter	4.654	4.541	0.153	0.041	0.6	0.933	0.000	0.000	0.3	1.049	78.410	OK
FEH: 30 years: +40 %: 180 mins: Summer	4.662	4.542	0.161	0.042	0.7	0.958	0.000	0.000	0.3	1.269	77.820	OK
FEH: 30 years: +40 %: 180 mins: Winter	4.664	4.536	0.163	0.036	0.5	0.970	0.000	0.000	0.2	1.273	77.553	OK
FEH: 30 years: +40 %: 240 mins: Summer	4.665	4.538	0.165	0.038	0.6	0.975	0.000	0.000	0.2	1.451	77.427	OK
FEH: 30 years: +40 %: 240 mins: Winter	4.669	4.532	0.168	0.032	0.4	0.994	0.000	0.000	0.2	1.469	76.994	OK
FEH: 30 years: +40 %: 360 mins: Summer	4.668	4.533	0.167	0.033	0.4	0.992	0.000	0.000	0.2	1.722	77.047	OK
FEH: 30 years: +40 %: 360 mins: Winter	4.669	4.527	0.168	0.027	0.3	1.001	0.000	0.000	0.1	1.731	76.836	OK
FEH: 30 years: +40 %: 480 mins: Summer	4.670	4.529	0.169	0.029	0.3	1.003	0.000	0.000	0.2	1.935	76.777	OK
FEH: 30 years: +40 %: 480 mins: Winter	4.669	4.523	0.168	0.023	0.2	0.997	0.000	0.000	0.1	1.928	76.910	OK
FEH: 30 years: +40 %: 600 mins: Summer	4.669	4.526	0.168	0.026	0.3	0.995	0.000	0.000	0.1	2.075	76.969	OK
FEH: 30 years: +40 %: 600 mins: Winter	4.668	4.521	0.167	0.021	0.2	0.990	0.000	0.000	0.1	2.085	77.074	OK
FEH: 30 years: +40 %: 720 mins: Summer	4.669	4.525	0.168	0.025	0.2	0.993	0.000	0.000	0.1	2.206	77.020	OK
FEH: 30 years: +40 %: 720 mins: Winter	4.667	4.520	0.166	0.020	0.2	0.980	0.000	0.000	0.1	2.239	77.322	OK
FEH: 30 years: +40 %: 960 mins: Summer	4.666	4.522	0.165	0.022	0.2	0.976	0.000	0.000	0.1	2.437	77.417	OK
FEH: 30 years: +40 %: 960 mins: Winter	4.661	4.518	0.161	0.018	0.1	0.945	0.000	0.000	0.1	2.385	78.116	OK
FEH: 30 years: +40 %: 1440 mins: Summer	4.660	4.518	0.159	0.018	0.1	0.933	0.000	0.000	0.1	2.628	78.395	OK
FEH: 30 years: +40 %: 1440 mins: Winter	4.654	4.515	0.153	0.015	0.1	0.901	0.000	0.000	0.1	2.686	79.144	OK

Project: 57 River Road Littlehampton	Date: 16/06/2026			
	Designed by:	Checked by:	Approved By:	
	MJD	HL	HL	
Report Details: Type: Stormwater Controls Summary Storm Phase: Phase	Company Address: AKT II, White Collar Factory 1 Old Street Yard London EC1Y 8AF			



FEH: 100 years: Increase Rainfall (%): +40: Summary Results for Cellular Storage: Rank By: Max. US Depth

Storm Event	Max. US Level (m)	Max. DS Level (m)	Max. US Depth (m)	Max. DS Depth (m)	Max. Inflow (L/s)	Max. Resident Volume (m³)	Max. Flooded Volume (m³)	Total Lost Volume (m³)	Max. Outflow (L/s)	Total Discharge Volume (m³)	Percentage Available (%)	Status
FEH: 100 years: +40 %: 15 mins: Summer	4.166	4.166	0.466	0.466	26.0	10.627	0.000	0.000	1.3	1.671	43.292	OK
FEH: 100 years: +40 %: 15 mins: Winter	4.166	4.166	0.466	0.466	24.4	10.623	0.000	0.000	1.3	1.679	43.314	OK
FEH: 100 years: +40 %: 30 mins: Summer	4.315	4.315	0.615	0.615	18.2	14.018	0.000	0.000	1.4	3.961	25.196	OK
FEH: 100 years: +40 %: 30 mins: Winter	4.314	4.314	0.614	0.614	17.1	14.007	0.000	0.000	1.4	3.966	25.257	OK
FEH: 100 years: +40 %: 60 mins: Summer	4.440	4.440	0.740	0.740	16.2	16.876	0.000	0.000	1.6	8.684	9.947	OK
FEH: 100 years: +40 %: 60 mins: Winter	4.443	4.443	0.743	0.743	13.1	16.927	0.000	0.000	1.6	8.687	9.672	OK
FEH: 100 years: +40 %: 120 mins: Summer	4.471	4.471	0.771	0.771	11.6	17.570	0.000	0.000	1.6	17.157	6.243	OK
FEH: 100 years: +40 %: 120 mins: Winter	4.473	4.473	0.773	0.773	8.4	17.623	0.000	0.000	1.6	17.166	5.961	OK
FEH: 100 years: +40 %: 180 mins: Summer	4.465	4.465	0.765	0.765	9.0	17.439	0.000	0.000	1.6	24.124	6.942	OK
FEH: 100 years: +40 %: 180 mins: Winter	4.460	4.460	0.760	0.760	6.3	17.317	0.000	0.000	1.6	24.130	7.594	OK
FEH: 100 years: +40 %: 240 mins: Summer	4.448	4.448	0.748	0.748	7.5	17.046	0.000	0.000	1.6	29.373	9.037	OK
FEH: 100 years: +40 %: 240 mins: Winter	4.435	4.435	0.735	0.735	5.1	16.747	0.000	0.000	1.6	29.376	10.634	OK
FEH: 100 years: +40 %: 360 mins: Summer	4.400	4.400	0.700	0.700	5.6	15.965	0.000	0.000	1.5	34.154	14.807	OK
FEH: 100 years: +40 %: 360 mins: Winter	4.369	4.369	0.669	0.669	3.7	15.260	0.000	0.000	1.5	34.114	18.571	OK
FEH: 100 years: +40 %: 480 mins: Summer	4.350	4.350	0.650	0.650	4.5	14.819	0.000	0.000	1.5	36.755	20.921	OK
FEH: 100 years: +40 %: 480 mins: Winter	4.302	4.302	0.602	0.602	3.0	13.732	0.000	0.000	1.4	36.771	26.725	OK
FEH: 100 years: +40 %: 600 mins: Summer	4.303	4.303	0.603	0.603	3.8	13.741	0.000	0.000	1.4	38.818	26.674	OK
FEH: 100 years: +40 %: 600 mins: Winter	4.242	4.242	0.542	0.542	2.5	12.357	0.000	0.000	1.4	38.819	34.063	OK
FEH: 100 years: +40 %: 720 mins: Summer	4.261	4.261	0.561	0.561	3.3	12.788	0.000	0.000	1.4	40.525	31.758	OK
FEH: 100 years: +40 %: 720 mins: Winter	4.188	4.188	0.488	0.488	2.1	11.135	0.000	0.000	1.3	40.532	40.581	OK
FEH: 100 years: +40 %: 960 mins: Summer	4.187	4.187	0.487	0.487	2.6	11.096	0.000	0.000	1.3	43.253	40.790	OK
FEH: 100 years: +40 %: 960 mins: Winter	4.099	4.099	0.399	0.399	1.7	9.087	0.000	0.000	1.2	43.314	51.510	OK
FEH: 100 years: +40 %: 1440 mins: Summer	4.078	4.078	0.378	0.378	1.9	8.621	0.000	0.000	1.1	47.310	53.999	OK
FEH: 100 years: +40 %: 1440 mins: Winter	3.977	3.977	0.277	0.277	1.2	6.318	0.000	0.000	1.0	47.338	66.287	OK

Project: 57 River Road Littlehampton	Date: 16/06/2026			
	Designed by:	Checked by:	Approved By:	
	MJD	HL	HL	
Report Details: Type: Stormwater Controls Summary Storm Phase: Phase	Company Address: AKT II, White Collar Factory 1 Old Street Yard London EC1Y 8AF			



FEH: 100 years: Increase Rainfall (%): +40: Summary Results for Porous Paving Area 02: Rank By: Max. US Depth

Storm Event	Max. US Level (m)	Max. DS Level (m)	Max. US Depth (m)	Max. DS Depth (m)	Max. Inflow (L/s)	Max. Resident Volume (m³)	Max. Flooded Volume (m³)	Total Lost Volume (m³)	Max. Outflow (L/s)	Total Discharge Volume (m³)	Percentage Available (%)	Status
FEH: 100 years: +40 %: 15 mins: Summer	4.694	4.548	0.193	0.048	5.5	2.190	0.000	0.000	0.3	0.311	74.649	OK
FEH: 100 years: +40 %: 15 mins: Winter	4.694	4.547	0.193	0.047	5.2	2.190	0.000	0.000	0.3	0.310	74.648	OK
FEH: 100 years: +40 %: 30 mins: Summer	4.757	4.553	0.255	0.053	3.7	2.820	0.000	0.000	0.4	0.576	67.365	OK
FEH: 100 years: +40 %: 30 mins: Winter	4.758	4.552	0.256	0.052	3.5	2.832	0.000	0.000	0.4	0.577	67.227	OK
FEH: 100 years: +40 %: 60 mins: Summer	4.815	4.554	0.314	0.054	3.3	3.447	0.000	0.000	0.4	0.884	60.102	OK
FEH: 100 years: +40 %: 60 mins: Winter	4.816	4.551	0.315	0.051	2.7	3.457	0.000	0.000	0.4	0.883	59.988	OK
FEH: 100 years: +40 %: 120 mins: Summer	4.851	4.549	0.349	0.049	2.3	3.969	0.000	0.000	0.3	1.338	54.068	OK
FEH: 100 years: +40 %: 120 mins: Winter	4.852	4.543	0.350	0.043	1.7	3.966	0.000	0.000	0.3	1.331	54.099	OK
FEH: 100 years: +40 %: 180 mins: Summer	4.860	4.544	0.358	0.044	1.8	4.239	0.000	0.000	0.3	1.809	50.943	OK
FEH: 100 years: +40 %: 180 mins: Winter	4.865	4.538	0.363	0.038	1.3	4.266	0.000	0.000	0.2	1.810	50.621	OK
FEH: 100 years: +40 %: 240 mins: Summer	4.865	4.540	0.363	0.040	1.5	4.394	0.000	0.000	0.3	2.268	49.141	OK
FEH: 100 years: +40 %: 240 mins: Winter	4.868	4.534	0.366	0.034	1.0	4.432	0.000	0.000	0.2	2.273	48.705	OK
FEH: 100 years: +40 %: 360 mins: Summer	4.872	4.534	0.371	0.034	1.1	4.538	0.000	0.000	0.2	3.107	47.473	OK
FEH: 100 years: +40 %: 360 mins: Winter	4.871	4.529	0.369	0.029	0.7	4.565	0.000	0.000	0.2	3.087	47.160	OK
FEH: 100 years: +40 %: 480 mins: Summer	4.871	4.531	0.370	0.031	0.9	4.540	0.000	0.000	0.2	3.746	47.448	OK
FEH: 100 years: +40 %: 480 mins: Winter	4.871	4.526	0.370	0.026	0.6	4.583	0.000	0.000	0.1	3.764	46.952	OK
FEH: 100 years: +40 %: 600 mins: Summer	4.871	4.529	0.370	0.029	0.8	4.573	0.000	0.000	0.2	4.309	47.073	OK
FEH: 100 years: +40 %: 600 mins: Winter	4.865	4.524	0.364	0.024	0.5	4.558	0.000	0.000	0.1	4.256	47.250	OK
FEH: 100 years: +40 %: 720 mins: Summer	4.870	4.527	0.368	0.027	0.7	4.581	0.000	0.000	0.1	4.771	46.983	OK
FEH: 100 years: +40 %: 720 mins: Winter	4.866	4.523	0.365	0.023	0.4	4.610	0.000	0.000	0.1	4.815	46.644	OK
FEH: 100 years: +40 %: 960 mins: Summer	4.866	4.524	0.365	0.024	0.5	4.598	0.000	0.000	0.1	5.560	46.779	OK
FEH: 100 years: +40 %: 960 mins: Winter	4.857	4.522	0.355	0.022	0.3	4.551	0.000	0.000	0.1	5.528	47.326	OK
FEH: 100 years: +40 %: 1440 mins: Summer	4.854	4.522	0.352	0.022	0.4	4.510	0.000	0.000	0.1	6.484	47.801	OK
FEH: 100 years: +40 %: 1440 mins: Winter	4.844	4.522	0.342	0.022	0.2	4.432	0.000	0.000	0.1	6.622	48.700	OK

Project: 57 River Road Littlehampton	Date: 16/06/2026		
	Designed by: MJD	Checked by: HL	Approved By: HL
Report Details: Type: Stormwater Controls Summary Storm Phase: Phase	Company Address: AKT II, White Collar Factory 1 Old Street Yard London EC1Y 8AF		



FEH: 100 years: Increase Rainfall (%): +40: Summary Results for Porous Paving Area 01: Rank By: Max. US Depth

Storm Event	Max. US Level (m)	Max. DS Level (m)	Max. US Depth (m)	Max. DS Depth (m)	Max. Inflow (L/s)	Max. Resident Volume (m³)	Max. Flooded Volume (m³)	Total Lost Volume (m³)	Max. Outflow (L/s)	Total Discharge Volume (m³)	Percentage Available (%)	Status
FEH: 100 years: +40 %: 15 mins: Summer	4.602	4.562	0.101	0.062	2.5	0.835	0.000	0.000	0.5	0.419	80.664	OK
FEH: 100 years: +40 %: 15 mins: Winter	4.601	4.561	0.100	0.061	2.4	0.822	0.000	0.000	0.5	0.416	80.965	OK
FEH: 100 years: +40 %: 30 mins: Summer	4.635	4.563	0.134	0.063	1.7	0.996	0.000	0.000	0.5	0.701	76.948	OK
FEH: 100 years: +40 %: 30 mins: Winter	4.635	4.562	0.134	0.062	1.6	0.982	0.000	0.000	0.5	0.697	77.274	OK
FEH: 100 years: +40 %: 60 mins: Summer	4.668	4.564	0.168	0.064	1.5	1.101	0.000	0.000	0.5	1.011	74.516	OK
FEH: 100 years: +40 %: 60 mins: Winter	4.668	4.560	0.167	0.060	1.2	1.094	0.000	0.000	0.5	1.007	74.671	OK
FEH: 100 years: +40 %: 120 mins: Summer	4.690	4.556	0.189	0.056	1.1	1.147	0.000	0.000	0.4	1.373	73.450	OK
FEH: 100 years: +40 %: 120 mins: Winter	4.691	4.549	0.190	0.049	0.8	1.154	0.000	0.000	0.4	1.377	73.285	OK
FEH: 100 years: +40 %: 180 mins: Summer	4.697	4.550	0.196	0.050	0.8	1.173	0.000	0.000	0.4	1.649	72.853	OK
FEH: 100 years: +40 %: 180 mins: Winter	4.698	4.542	0.197	0.042	0.6	1.176	0.000	0.000	0.3	1.638	72.781	OK
FEH: 100 years: +40 %: 240 mins: Summer	4.700	4.545	0.199	0.045	0.7	1.193	0.000	0.000	0.3	1.883	72.381	OK
FEH: 100 years: +40 %: 240 mins: Winter	4.701	4.537	0.200	0.037	0.5	1.196	0.000	0.000	0.2	1.872	72.326	OK
FEH: 100 years: +40 %: 360 mins: Summer	4.703	4.539	0.202	0.039	0.5	1.208	0.000	0.000	0.2	2.226	72.036	OK
FEH: 100 years: +40 %: 360 mins: Winter	4.704	4.531	0.203	0.031	0.3	1.211	0.000	0.000	0.2	2.235	71.965	OK
FEH: 100 years: +40 %: 480 mins: Summer	4.703	4.534	0.203	0.034	0.4	1.209	0.000	0.000	0.2	2.475	72.021	OK
FEH: 100 years: +40 %: 480 mins: Winter	4.703	4.528	0.202	0.028	0.3	1.199	0.000	0.000	0.1	2.479	72.235	OK
FEH: 100 years: +40 %: 600 mins: Summer	4.703	4.531	0.202	0.031	0.3	1.202	0.000	0.000	0.2	2.666	72.171	OK
FEH: 100 years: +40 %: 600 mins: Winter	4.701	4.525	0.200	0.025	0.2	1.187	0.000	0.000	0.1	2.682	72.512	OK
FEH: 100 years: +40 %: 720 mins: Summer	4.703	4.529	0.202	0.029	0.3	1.198	0.000	0.000	0.2	2.846	72.263	OK
FEH: 100 years: +40 %: 720 mins: Winter	4.699	4.523	0.198	0.023	0.2	1.170	0.000	0.000	0.1	2.823	72.909	OK
FEH: 100 years: +40 %: 960 mins: Summer	4.698	4.526	0.197	0.026	0.2	1.167	0.000	0.000	0.1	3.053	72.983	OK
FEH: 100 years: +40 %: 960 mins: Winter	4.693	4.521	0.192	0.021	0.2	1.137	0.000	0.000	0.1	3.122	73.686	OK
FEH: 100 years: +40 %: 1440 mins: Summer	4.692	4.522	0.191	0.022	0.2	1.130	0.000	0.000	0.1	3.451	73.854	OK
FEH: 100 years: +40 %: 1440 mins: Winter	4.681	4.518	0.180	0.018	0.1	1.063	0.000	0.000	0.1	3.444	75.384	OK

B.11 Foul Water Flows

One method of calculating the peak foul flow is to use the statistical or "discharge unit" method outlined in BS 12056. This method of design is based on probability theory to estimate the maximum peak flows from appliances. It assumes that only a proportion of appliances will be in use at any one time and that appliances on a factory, school, etc. would be used more frequently than in domestic installations. For this reason discharge units (DU) are given higher values in congested and commercial use than they are for domestic.

Alternatively, foul flows can be computed using the "population method". This method calculates a daily discharge volume from typical figures of discharge per person, per day. It then factors it evenly over the day, to calculate base flow. A peaking factor is then applied (conservatively 6 or a variable factor based upon population such as the Babbitt Formula) to determine peak flow.

Estimates can also be based on internal floor area, which can also be arrived at as a product of an assumed population density and daily discharge per person.

As to which method is the more appropriate, typically, outside the building and at the street and city scale, the population method is considered more accurate and is the preferred method utilised by sewer companies. For sizing pipes inside the building, and in drains near to the point of discharge from the building, the statistical method should be used. Consideration of high peak flows from pump stations is also required.

BS 16933-2 recommends that the statistical method can be used for the design of gravity drains and sewers where the population is less than 1000 and the population method can be used where the population is greater than 500.

B.12 Existing Foul Water

There are currently no record drawings available for the existing building in order to estimate the number of foul appliances. However, the existing use as an auto repair facility would likely generate a discharge of around **5 litres/sec**.

B.13 Proposed Foul Water

Discharge Unit Method

The architectural layouts have been used to estimate the number of foul appliances in the proposed office space. Using the guidelines for commercial developments given in BS EN 12056-2:2000 - "Gravity Drainage Systems Inside Buildings - Part 2: Sanitary Pipework, layout and calculation", the proposed foul flow is calculated from the formula:

$$Q = K \times \overline{DU}$$

For 'intermittent use' (representing dwellings, offices, etc.) K has a value of 0.5 giving:

Appliance	No.	Discharge units per appliance	Total number of discharge units
Wash hand basin	12	0.5	6.0
Bath	12	0.8	9.6
Sinks	9	0.8	7.2
Dishwasher	9	0.8	7.2
Washing machine	9	0.8	7.2
WC	12	2.0	24.0
Total discharge units for site =			61.2
Therefore, total flow from site =			3.91 litres / sec

This is likely to be comparable to the existing flow rates and therefore should not represent an increase in discharge to the public sewer.

Appendix C

SuDS Option Selection



Disposal methods

C.1 SuDS management train

A useful concept used in the development of sustainable drainage systems is the SuDS management train (sometimes referred to as the treatment train). Just as in a natural catchment, drainage techniques can be used in series to change flow and quality characteristics of the runoff in stages. There are a variety of measures that can be implemented to achieve these goals:

C.1.1 Site management / Prevention

Site management procedures are used to limit or prevent runoff and pollution and include:

- Minimising the hardened areas within the site
- Frequent maintenance of impermeable surfaces
- Minimising the use of de-icing products

C.1.2 Source control

Source control techniques will be used where possible as they control runoff at source in smaller catchments. They can also provide effective pollution control and treatment, thereby improving the quality of the effluent discharged to the receiving waters.

C.1.3 Site control

Where source control techniques do not provide adequate protection to the receiving watercourses in terms of flood protection and pollution control, site control may be required.

C.1.4 Regional control

Where large areas of public space are available regional control can be incorporated to provide additional 'communal' storage and treatment to runoff from a number of sites. However, in this case, all storage and treatment will be implemented on site.

C.2 Drainage hierarchy

Based on the above and in line with the London Plan and the Sustainable Drainage Manual published by CIRIA, the following drainage hierarchy will therefore need to be considered when preparing the surface water disposal strategy:

1. Rainwater use as a resource (for example rainwater harvesting, blue roofs for irrigation)
2. Rainwater infiltration to ground at or close to source
3. Rainwater attenuation in green infrastructure features for gradual release (for example green roofs, rain gardens)
4. Rainwater discharge direct to a watercourse (unless not appropriate)
5. Controlled rainwater discharge to a surface water sewer or drain
6. Controlled rainwater discharge to a combined sewer

C.3 Assessment of SuDS techniques

C.3.1 Rainwater harvesting

This involves the capture of rainwater into a tank for re-use (usually non-potable) such as irrigation, toilet flushing or vehicle cleaning. Systems are now available which combine rain water harvesting with tanked attenuation. This means that water is stored during dry periods for re-use but released ahead of predicted storms in order to ensure that the full attenuation capacity remains available when it is needed.

As the proposed development has a small plan area, it does not appear to be practical to install a rainwater harvesting system as the actual roof area available would not generate sufficient water yield to make the system viable.

C.3.2 Green / brown / blue roofs

These are used on flat or shallow pitched roofs to provide a durable roof covering which also provides thermal insulation, amenity space, biodiversity habitat as well as attenuation of rainwater. Depending on the design, these roofs can attenuate differing volumes of rainwater. The term 'blue roof' is reserved for those roofs designed to maximise water retention. This is a relatively recent area of increased focus and can involve effectively an attenuation tank at roof level which reduces (or avoids) the need for pumping of basement tanks.

The pitched roofs proposed on the scheme do not lend themselves to implementing blue or green roofs.

C.3.3 Raingardens

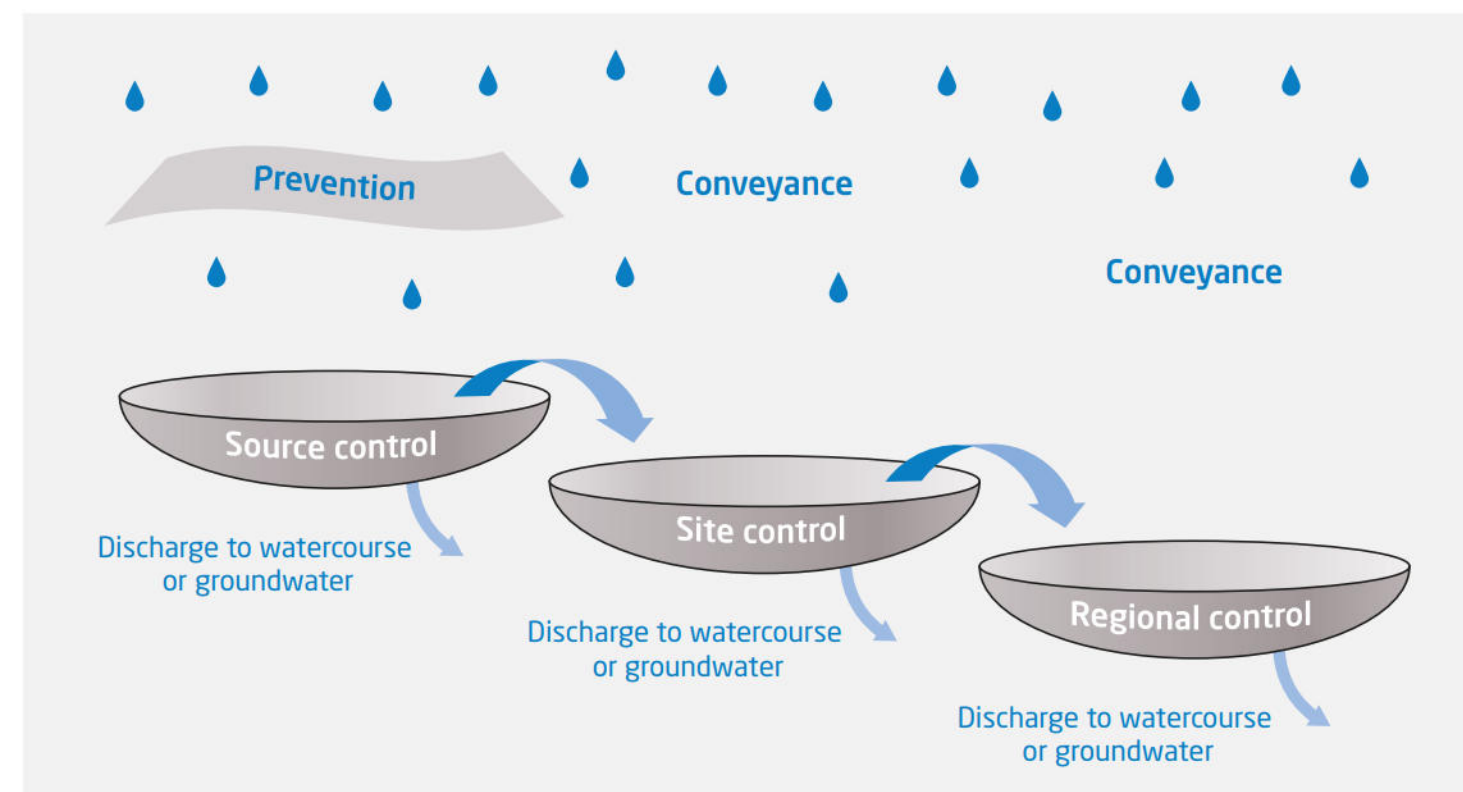
Raingardens are planted areas (usually close to buildings but not immediately adjacent) that allow the diversion of a portion of rainwater from either downpipes or the surrounding paved surfaces. These techniques can be incorporated into the landscaping plans for a site and are most effective where the landscaping regime is designed with the aim of capturing as much rainfall as possible. They can either allow infiltration into the ground or have tanked systems for water retention, depending on the site and soil conditions. There are also a number of vertical raingardens attached to building walls with rainwater downpipes diverted through a stacked series of planters.

Small areas of soft landscaping have been incorporated into the scheme which will collect runoff from adjacent areas of hard landscaping. Approximately 20 m² of hard landscaping will be shed into these areas.

C.3.4 Bio-retention

This refers to a chain of landscaped features, potentially including reed beds, filter drains, etc. designed to hold and treat surface water. They are often used where there is a high risk of low-level pollution, for example from road runoff. However, it does require areas of open space. The design of a bio-retention system can vary widely depending on site conditions and available space. At a small scale this could include flow through planters or tree pits.

Bio-retention cannot be incorporated into the scheme due to the lack of available external space.



SuDS management train

C.3.5 Infiltration

There are many different types of drainage component that can be used to facilitate infiltration. These include soakaways, infiltration trenches, infiltration blankets and infiltration basins.

Infiltration can contribute to reducing runoff rates and volumes while supporting baseflow and groundwater recharge processes. The rate at which water can be infiltrated depends on the infiltration capacity (permeability) of the surrounding soils.

The available borehole information suggests there may be limited porosity in the underlying soil. However, as there is no available external space and the building covers the full site footprint, it is not possible to locate infiltration devices that comply with Building Regulations. Also, there would be a risk of undermining the Environment Agency flood wall due to it's proximity to any soakaway that would be proposed. Therefore, disposal by infiltration will not be possible.

C.3.6 Permeable surfacing

Permeable hard surfaces which work in much the same way as traditional impermeable surfaces apart from the ability to allow rainwater to pass through. Permeable blocks are traditionally used but there are now a range of permeable asphalt and resin bound gravel pavings being used increasingly commonly. Permeable surfaces can either allow infiltration into the ground or have tanked systems for water retention, depending on the site and soil conditions. They are suitable in even the most densely built-up development. However, they're not well suited to roads carrying heavy or fast motor traffic.

The car parking bays will be constructed as lined permeable paving which will provide filtration of pollutants before discharging to the attenuation tank. It is not possible to provide an infiltrating solution due to the lack of porosity.

C.3.7 Swales

These are dry ditches used as landscape features to allow the storage, carriage and infiltration of rainwater and are often used as linear features alongside roads, footpaths or rail lines. They can also be integrated into the design of many open spaces.

Swales cannot be incorporated into the scheme due to the lack of available external space.

C.3.8 Detention basin / ponds

Landscape features designed to store and in some cases infiltrate rainwater. Detentions basins are usually dry, whereas a pond should retain water. These features need areas of open space but can often be combined with other sustainable drainage techniques.

As the site is heavily developed with limited external areas there is insufficient space to provide a basin or pond.

C.3.9 Discharge to tidal river / dock / canals

Discharging clean rainwater directly to tidal rivers, canals or docks isn't normally a sustainable drainage technique. Other more productive techniques should be used first. However, it is generally more sustainable than discharging to the combined or surface drainage systems. Residual surface water can be discharged to tidal / large waterbodies, in some cases with no limitation on volumes. Some storage may be required to allow for outfalls becoming tide locked. Care is needed to prevent scour in the receiving waterbody and potentially to prevent pollution. Consent from the Environment Agency, the asset owner and where applicable the Canal and River Trust is required.

The River Arun is immediately adjacent to the site and as part of the Environment Agency's improvements to the flood defences a new section of flood wall has been implemented along the site frontage (Planning Ref. LU/291/17/PL). Due to the potential to increase flood risk to the site (and adjacent properties) if holes were to be cored through or under the flood wall, it is not considered appropriate to discharge to the River Arun.

C.3.10 Storage tanks / geocellular storage

Storage tanks are single GRP units usually located (but not necessarily) below ground level which attenuate rainwater for later slow release back into the drainage system but do not provide the wider benefits of green infrastructure sustainable drainage. They can also have the disadvantage that pumping may be required to empty the tank into the drainage system - especially if the tank is located at or below basement level. Where tanks are designed for large storm events, care is needed to ensure that they still perform a useful sustainable drainage function for low order storms.

Geocellular storage tanks are similar to storage tanks except that the volume is made up from multiple units rather than a single tank meaning they can be more flexible in terms of shape to suit constrained sites.

It is believed that this is the most feasibly disposal option for the roof proportion of the site and a storage volume of 18m³ should be provided in order to limit the discharge from the site to 1.6 litres/sec (30% of the pre-development 1 in 1 year rate).

The attenuation tank should be located at a high enough level so as to allow a connection to be made to the public sewer by gravity. Locating the tank below this level would result in a pumped surface water system which is both unsustainable and uneconomic.

C.3.11 Oversized piping

Using larger than necessary pipework creates more room to store rainwater. Potentially more sustainable than storage tanks / geocellular storage if the pipes drain by gravity and do not require pumping. However, lacks the wider benefits of the green infrastructure based techniques.

Due to the restricted nature of the site the pipework would become impractically large to provide the volume of storage required to achieve the required runoff rate.

C.4 Summary of the proposed SuDS strategy

Approximately 20 m2 of hardstanding will shed into planted areas. An area of 108 m2 of lined permeable paving will be provided which will discharge to an attenuation tank of 18 m3 which will reduce the peak discharge rate from the site to 1.6 litres/sec (30% of the existing 1 in 1 year rate) using a Hydrobrake.

The outfall from the site will connect to the existing public sewer either with a new connection or, preferably, by reusing the existing connection.

A schematic SuDS layout is contained in Appendix D for reference.

Element	Management stage	Water quantity	Water quality	Amenity & biodiversity	Proposed in scheme
Rainwater harvesting	Prevention	✓	✗	✗	✗
Green / brown / blue roof	Source control	✓	✓	✓	✗
Raingardens	Source control	✓	✓	✓	✗
Bio-retention	Source control	✓	✓	✓	✗
Permeable surfacing	Source control	✓	✓	✗	✗
Swales	Source control	✓	✓	✓	✗
Detention basin / ponds	Source control	✓	✓	✓	✗
Discharge to tidal river / dock / canals	Site control	✓	✗	✗	✓
Storage tanks / Geocellular storage	Site control	✓	✗	✗	✓
Oversized piping	Site control	✓	✗	✗	✗
Design for exceedance	Site control	✓	✗	✗	✗

Figure C.1 Summary of proposed SuDS devices

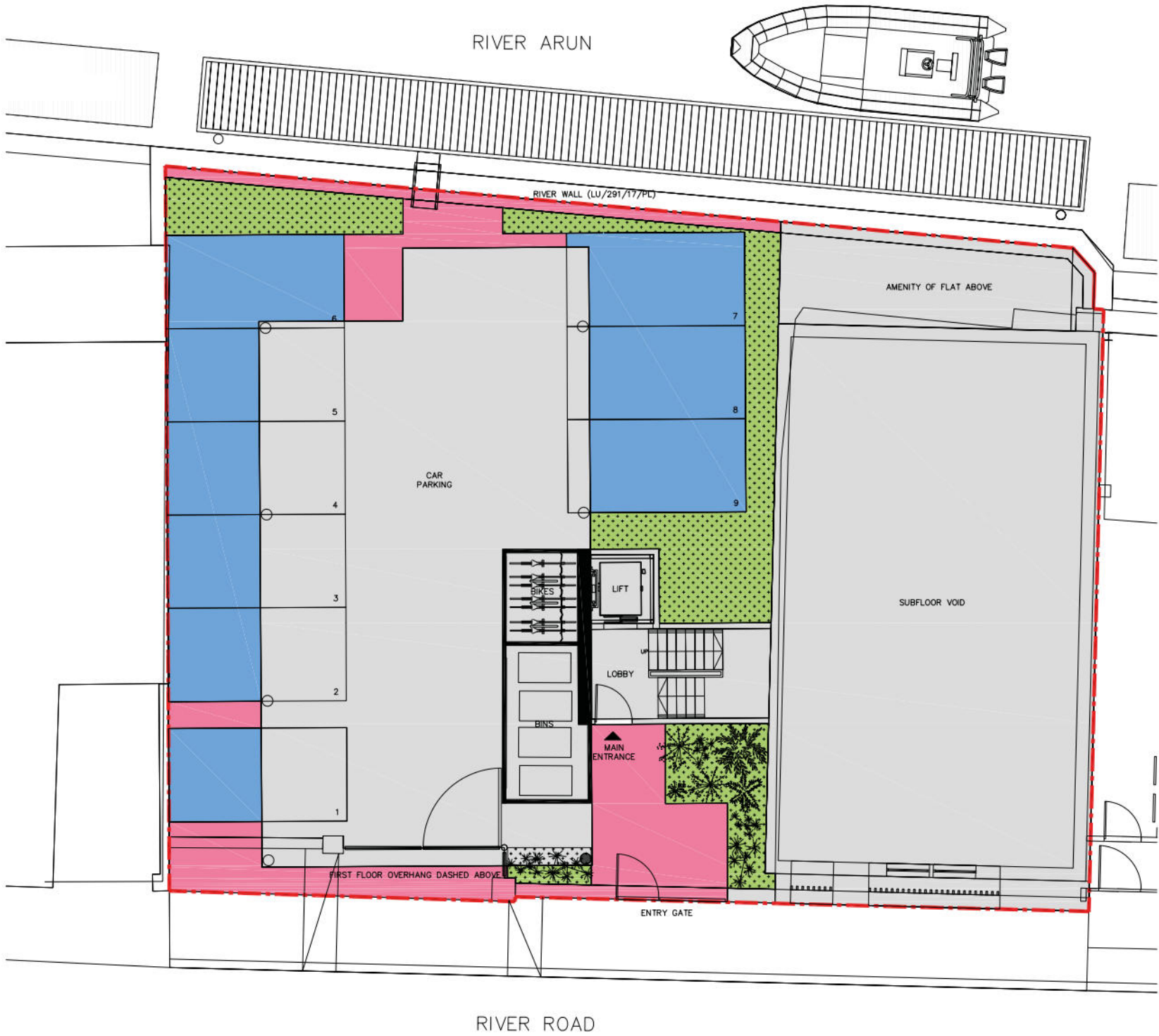
Appendix D

Proposed Drawings



NOTES

- DENOTES SITE BOUNDARY CONSIDERED (475 m²)
- DENOTES PROPOSED ROOF AREA (310 m²)
- DENOTES PROPOSED EXPOSED CAR PARK AREA (80 m²)
- DENOTES PROPOSED HARDSTANDING AREA (50 m²)
- DENOTES PROPOSED SOFT LANDSCAPING AREA (35 m²)



P01	16/06/26	PRELIMINARY ISSUE	MD	HL
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S2 - FOR INFORMATION

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PROPOSED AREAS

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