

Soil Investigations Statement for #1 Burley Road Summerley Estate Felpham

#1 Burley Road
Summerley Estate
Felpham
BOGNOR REGIS
PO22 7NF

Mate GeoTechnic Services has been instructed to carry out Soil Investigations relating to discharging a Surface Water Drainage Condition #3 within Planning Application FP/176/24/PL.

The investigations are for Groundwater Monitoring and Soil Infiltration (Percolation) Testing.

An initial submission for Discharge of Conditions (DoC) resulted in an 'Objection' from the Arun DC Drainage Engineers (External Consultation). Basically these were inadequate details for Groundwater Monitoring and insufficient details regarding the Soil Infiltration Testing. For full details refer to Arun DC document FP/78/25/DOC, issue date 17/11/2025 comment number 1.

Further breakdown of the Groundwater Monitoring exercise included the impact of Sea Tidal Range Effects, during a 'Spring Tide' as required by Arun DC Principal Drainage Engineer (reference email November 2025).

Utilising the data from the Groundwater Monitoring, the peak groundwater level can be realised, which then informs as to the viability (or not) of ground infiltration as a method of Surface Water Disposal and importantly its structure type and maximum depth. The latter taking into account the Freeboard (defined as the amount of unsaturated soil between the peak recorded groundwater level and the base of the infiltration structure).

The site was an active operational building site at the time of investigations, such that the only undisturbed / untracked area was at the southern area in the original garden. The locations of the Groundwater Monitoring Point GWP #1 and the Soil Infiltration Trial Pit TP#1 is shown on the original layout drawing. This is included within the appendices as page 7.

Investigations comply with the Arun DC Surface Water Drainage Guidelines as their Ground Investigations Checklist version 1.0 which was issued October 24th 2025.

GEOLOGY :

The British Geological Survey maps the area with the Superficial Deposits of "River Terrace Deposits" (Undifferentiated) of fluvial origin, consisting of Sand, silt and clay, overlying Bedrock of the "Lewes Nodular Chalk et seq" of shallow-marine in origin.

SURFACE WATER DRAINAGE CONSIDERATIONS :

Any SuDS and source control solutions for a surface water disposal strategy, should follow “the hierarchy of preference for different types of sustainable surface water drainage disposal systems” as set out in Approved Document H of the Building Regulations, and the recommendations of the SuDs Manual. These being :-

1. Rainwater reuse where possible.
2. Complete discharge into the ground (infiltration).
3. Hybrid infiltration and restricted discharge to an appropriate water body or surface water sewer.
4. Restricted discharge to an appropriate water body.
5. Restricted discharge to a surface water sewer (public / private).
6. Restricted discharge to a combined sewer

Note a water body may be defined as a river, watercourse, ditch, culverted watercourse, reservoir, wetland or the sea.

In general options 1) & 2) above are always the preferred solution and options 3) to 6) above are only to be considered after rigorous groundwater monitoring and soil infiltration testing has discounted option 2) as unviable.

It should be noted that surface water disposal into the Foul Water Sewer System is not supported within the SUDs Manual and as such, here, no disposal of surface water by this method will be recommended.

The only exception to the disposal of surface water into the foul drainage system is where Southern Water specifically designate the sewer to be a ‘Combined System’ which is not the case at this location.

GROUNDWATER MONITORING

Due to the considerable variability of soils encountered in the local area and across the coastal plain, a formal Groundwater Monitoring exercise was carried out.

Installation – The monitoring point was installed on the 29th November, in the south garden area for any proposed infiltration structure, within a hand augered 75mm diameter borehole, to a maximum depth of 2260mm Below Ground Level (BGL), where it refused on block chalk. Water strike was encountered at 1900mm BGL at the start of a layer of saturated medium sand. A 32mm diameter Open Head Piezometer Well (slotted monitoring pipe) was installed to this maximum depth with an upstand of 164mm. The 1000mm base response zone of the well was wrapped in a permeable geo-sock. A dry clean granular infill of 10mm shingle fully encased the pipe, where the top-most plain section, above the response zone, was sealed at ground level with clay to minimise ground level water ingressing down the pipe.

Monitoring commenced immediately.

Tidal Effects -- Tidal Effects Monitoring was a requirement as detailed in the email in November 2025 from Paul Cann (PC) -- Principal Drainage Engineer, Coastal Engineers and Flood Prevention. His email excerpt is . . . *The sooner u get monitoring the better but capture spring tides too. Water tables are still rising so there is a good chance the peaks haven't yet arrived. **There are good spring tides 4 to 7th Dec.***

This monitoring exercise was carried out from Thursday 4th December to Sunday 7th December 2025 inclusive, in line with PC's email, at both high tide & low tide points, to inform some information as to the tidal ranges involved and to the latency – lag time after a high / low tidal point. High Tide & Low Tide levels were taken from Tidal Tables on the Internet set for the nearest location at Bognor Regis.

It should be noted that from a recent similar exercise carried out for Arun DC at Littlehampton, the latency was typically 2½ hours, so it was concluded to take readings over the 4 day period, of increasing of 1 hour per day for each pair of readings. Delay periods of +0, +1, +2, +3 hours were to garner some overall meaningful data however limited it would be.

The Tidal Effects Groundwater Monitoring readings are included within the main Groundwater Monitoring data.

Over the limited 4 day monitoring period, the maximum range of tidal effects was observed to be 128mm. This effect may be higher since it is not known at what times the actual minimum and maximum levels occurred and would have been recorded.

The Latency -- lag times between the low / high tides and their corresponding recorded minimum and maximum readings was not possible to be estimated due to the very limited number of readings.

As can be seen from the graphical data overall groundwater seems to be rising over the 4 day period, though this may be less a result of "the pulsing of the high tides" since tidal ranges are decreasing, rather than more an increase in general groundwater levels. It should be noted that in early December, during the week leading upto the monitoring period, there had been some heavy rain on odd days.

The Tidal Effects Monitoring – tabular and graphical – is within the appendices as page 1.

Winter Monitoring – Within the guidelines, for Small Site Exceptions, the proposed roof area (increase) of less than 250m² details a reduced duration of monitoring period of 2 consecutive months, within the 3-month winter period of December-January-February. This application has an impermeable roof area of 200 square metres, so complies with the reduced duration of 2 months. Since the requested Tidal Effects Monitoring (as discussed above) was in early December, the prescribed 2 months Groundwater Monitoring period was December to January (inclusive). Readings were taken manually, at a maximum weekly interval, to conform to the current Arun DC guidelines.

The monitoring exercise produced the Peak Groundwater Level of 1569mm BGL at 4.331 metres AOD during the tidal monitoring cycle on the 7th December 2025. On the same day the second maximum groundwater level of 1595mm BGL at 4.305 metres AOD was recorded. The third maximum groundwater level occurred on Saturday the 31st January 2026 with a groundwater level of 1597mm BGL at 4.303 metres AOD.

Informed by the result of the tidal monitoring exercise, all subsequent readings were taken within 2 hours just after a high tide point if possible, to best ensure that the peak groundwater levels were recorded. This is in line with, and fully complies with the current Arun DC drainage guidelines.

The overall result from the 2 month monitoring cycle is that any infiltration structure cannot be any deeper than 569mm BGL – as regards to the Freeboard of 1000mm. This informs to shallow infiltration testing (500mm to 600mm) only.

The Groundwater Monitoring – tabular and graphical – is within the appendices as page 2.

SOIL INFILTRATION TESTING

Soil Infiltration Testing carried out over the 3-day period of Wednesday 25th February to Friday 27th February 2026. Again this is in line with, and fully complies with the current Arun DC drainage guidelines for testing within the winter period of December to February. All 3 tests being completed in the best 'quick succession' on subsequent days, to replicate saturated ground conditions, since it was not possible to practicably carry out more than one test per day. Testing over a 3 day period has been historically confirmed as being acceptable to Arun DC Drainage Engineers.

The Trial Pit and testing fully complies with the principles contained with BR DG365, for a size of 1130mm Length x 490mm Width x 500mm Depth. It should be noted that BRE DG365 has a stated minimum size of 1000mm Length x 300mm Width. This was hand dug to minimise soil compaction by machine excavation for such a shallow Trial Pit and to ensure a good shaped rectangular profile.

The depth of water filled within the Trial pit was that of the approximate expected storage depth of the proposed infiltration structure of 300mm to 350mm.

Each test was allowed to drain past the acceptable minimum 90% draining depth (as per the current guidelines). However tests #2 and #3 required late evening readings to ensure that the 90% limit was exceeded, with NO extrapolations, to ensure compliance. The 90% level is shown graphically for completeness.

Trial Pit TP#1 in the south garden area			
Dimensions	Length – 1130mm	Width – 490mm	Depth – 500mm
Test #1	4.920E-06 metres/second		Wednesday 25th February 2026
Test #2	4.156E-06 metres/second		Thursday 26th February 2026
Test #3	3.745E-06 metres/second		Friday 27th February 2026

The minimum rate to be used in calculations is 3.745E-06 metres/second. This rate does indicate favourable infiltration soil conditions for 'Blanket Type' storage & infiltration solutions.

The shallow Soil Infiltration testing will be suitable not only for the proposed 'Blanket Type' storage & infiltration structure, but also for any proposed self-draining (only) permeable driveway and parking area as detailed for both their Formation Levels.

The Soil Infiltration Testing – tabular and graphical – is within the appendices as pages 3 – 6.

GENERAL DRAINAGE CONSIDERATIONS

Ideally, it is recommended that the use of water butts, with diverter kits, are to be included on all of the new rainwater down pipes. These water butts offer a small impact on the full drainage strategy, as recommended by the SUDs Manual, but are included as an environmental consideration.

It should be noted that surface water disposal into the Foul Water Sewer System is not supported within the SUDs Manual and as such, here no disposal of surface water will be recommended.

The drainage disposal option will be required to drain via a separate Silt Trap / Catch Pit, within the curtilage of the property, prior to its disposal point.

The actual point of infiltration disposal must be located a minimum distance of 5 metres from any controlled structure. This being any permanent habitable structure (house etc.) and includes garages, so as not to compromise the stability of any foundations.

Additionally all points of infiltration disposal must be located a minimum distance of 5 metres from any new or existing soakaways so as not compromise the infiltrating capacity of the soils of each other.

Additionally any new point of infiltration disposal must be located a minimum distance of 5 metres from any Public Foul or Public Surface Water Sewer on, or adjacent to, the site so as not to compromise the integrity of any sewer line.

Best practice will also need to be taken into consideration as regards a 2½ metre distancing from any existing or proposed boundary.

IMPERMEABLE AREAS :

The proposed new full roof area, of the property, to be accounted for is 200 square metres. This to use the external discharge points as detailed above.

The impermeable small pathways are to be allowed to drain their runoff to the adjacent garden areas.

The driveway & parking area is detailed to be of permeable surfaced construction and as such is to be self-draining only.

DESIGN CONSIDERATIONS :

- All the designs for drainage flows must use FEH rainfall data, preferably FEH22 though FEH13 may be considered to be acceptable by the Local Planning Authority (LPA). It should be noted that using FSR rainfall data is not accepted.
- Climate Change Addition (CCA) to be +45% as per current Environment Agency (EA) guidelines (as at 2024) for 1% AEP (100 year) storm event. All other storm events set to +40%.
- Volumetric Runoff Coefficient (C_v) to be set to 1.0 for both summer & winter design storm events.
- Water Quality Assessment is such that any formal treatment need not to be included since only clean roof areas are to be accounted for.
- Urban Creep will need to be accounted for at +10% typically, of the proposed main building roof area, in this instance.
- Infiltration design rates must be applied to the sides only for traditional soakaways or to the base only if permeable paving or infiltration blankets are utilised. Design rates must not be applied to both the base and sides of any infiltration structure.
- Half-drain times must be less than 24 hours for the 1 in 10 year (10% AEP) and 1 in 100 year (1% AEP) design storm events, plus climate change allowance, for all infiltration structures.
- A Silt Trap is required to be the final part of the drainage train, with a minimum sump depth of 435mm.
- The full drainage flow must be a gravity system – Not Pumped – to replicate natural catchments.
- Factor of Safety (FoS) will need to be used in calculations typically set at 2.0
- Tree Root Protection Areas (RPA) are to be detailed on the drainage layout if relevant – there must be no conflicts between existing or proposed trees and any of the drainage infrastructure. However in this development RPAs are not applicable.

(RC 08-06-2026 version 2)

Mate GeoTechnic Services

REFERENCES :

Building Regulations Part H.

CIRIA SuDS Manual 2015 at
www.ciria.org

Arun DC Supplementary Guidance notes regarding surface water drainage at
<https://www.arun.gov.uk/surfacewater>

Arun DC Groundwater Monitoring Guidance note at
<https://www.arun.gov.uk/groundwater-monitoring>

Arun DC Infiltration Testing Guidance note at
<https://www.arun.gov.uk/infiltration-testing>

APPENDICES :

Page 1 -- Tidal Influencing on Groundwater Levels

Page 2 -- Groundwater Monitoring

Page 3 -- Soil Infiltration Test Trial Pit TP#1 Test Number 1

Page 4 -- Soil Infiltration Test Trial Pit TP#1 Test Number 2

Page 5 -- Soil Infiltration Test Trial Pit TP#1 Test Number 3

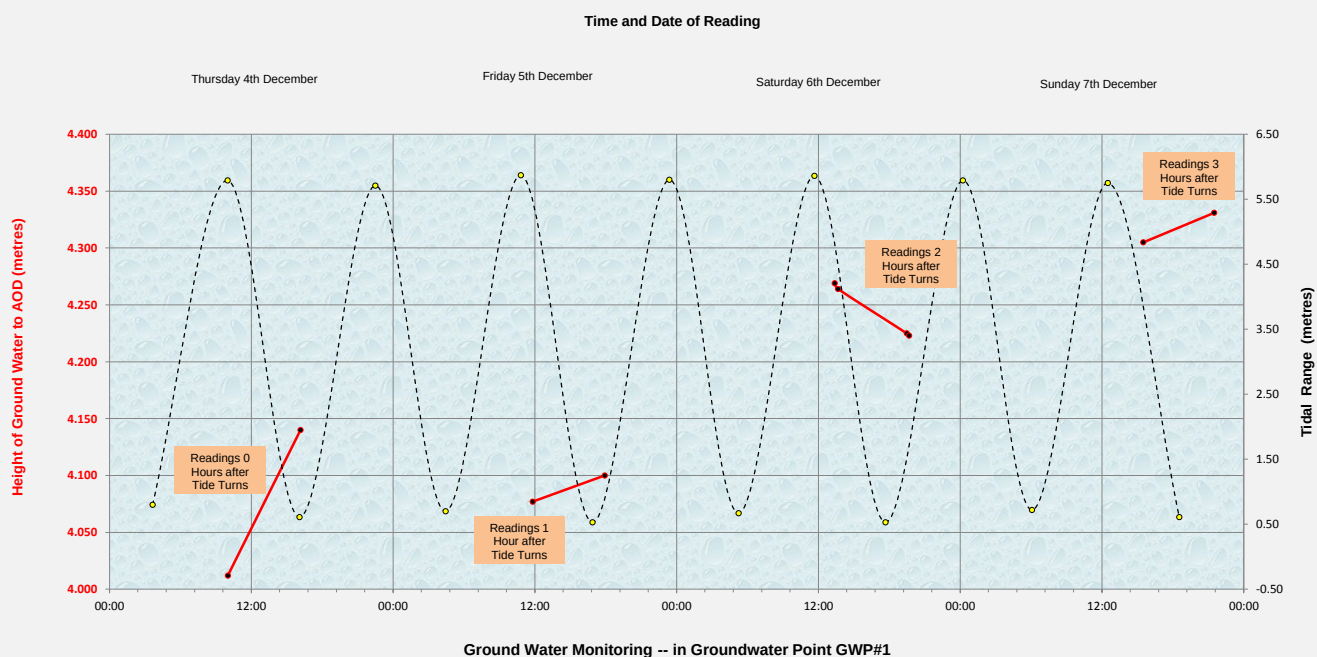
Page 6 -- Soil Infiltration Test Trial Pit TP#1 All 3 Tests

Page 7 -- Site Layout detailing Groundwater Monitoring and Soil Infiltration Testing Locations

GroundwaterPoint GWP#1
South of the Front Garden
Adjacent to the Front Entrance
From Sea Road
Summerley Estate, Felpham

	DATE	DATE	DATE	DATE	DATE	DATE	DATE	DATE	DATE	DATE	DATE	DATE	DATE	DATE	DATE
	Low Tide	High Tide	Low Tide	High Tide	Low Tide	High Tide	Low Tide	High Tide	Low Tide	High Tide	Low Tide	High Tide	Low Tide	High Tide	Low Tide
	Thursday 04-Dec-25				Friday 05-Dec-25				Saturday 06-Dec-25			Sunday 07-Dec-25			
Time of Tide Change >>>	03:39	10:00	16:05	22:30	04:27	10:49	16:53	23:23	05:15	11:40	17:41	00:14	06:05	12:30	18:33
Water Height at Bognor Regis >>>	0.80	5.79	0.61	5.71	0.70	5.87	0.53	5.80	0.67	5.86	0.53	5.79	0.72	5.75	0.61

	Pipe I.D.	Pipe Depth	Pipe Height AGL	A.O.D. Ground Level	Thursday 04-Dec-25	Thursday 04-Dec-25	Friday 05-Dec-25	Friday 05-Dec-25	Saturday 06-Dec-25	Saturday 06-Dec-25	Saturday 06-Dec-25	Saturday 06-Dec-25	Sunday 07-Dec-25	Sunday 07-Dec-25
Delay After Tidal Change (hours)					0	0	1	1	2	2	2	2	3	3
			Time of Reading > > >		10:01	16:10	11:49	17:55	13:23	13:40	19:29	19:41	15:30	21:30
Maximum Depth	GWP#1	2670	164		2052	1924	1987	1964	1795	1800	1839	1841	1759	1733
Below Ground Level					-1888	-1760	-1823	-1800	-1631	-1636	-1675	-1677	-1595	-1569
Level Above O.D.				5.900	4.012	4.140	4.077	4.100	4.269	4.264	4.225	4.223	4.305	4.331
Change from Previous						128		23			-44			26



High Tide and Low Tide levels were taken from Tidal Tables on the Internet set for the nearest location at Bognor Regis. The lowest tides occurred on Friday 5th December at 16:53 and Saturday 6th December at 17:41 both at 0.53 metres. The highest tide occurred on Friday 5th December at 10:49 at 5.87 metres.

The overall highest groundwater level was recorded on Sunday 7th December at 21:30 at 1569mm BGL at 4.331 metres AOD. Surprisingly this was after Low Tide at 16:33 -- at 0.61 metres and is possibly due to the increase in general groundwater levels being significantly more, than any smaller tidal effect.

The overall lowest groundwater level was recorded on Thursday 4th December at 10:01 at 1888mm BGL at 4.012 metres AOD.

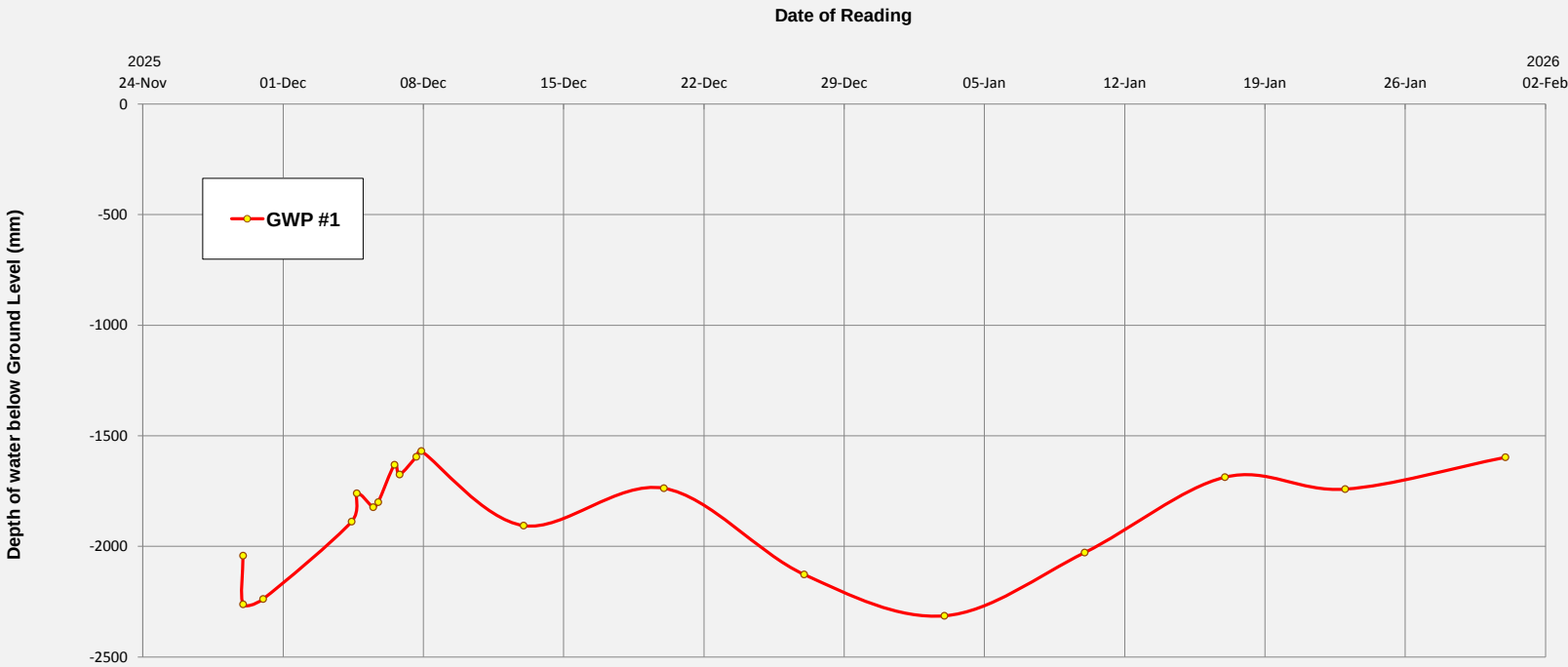
Over the limited 4 day monitoring period, the maximum range of tidal effects was 128mm. This effect may be higher since it is not known at what times the actual minimum and maximum levels occurred and would have been recorded.

The Latency -- lag times between the low / high tides and their corresponding recorded minimum and maximum readings was not possible to be estimated due to the very limited number of readings, however a similar more extensive exercise carried out at Littlehampton, during a Spring Tidal Cycle gave an estimated Latency of typically 2.5 hours.

As can be seen from the graphical data overall groundwater seems to be rising over the 4 day period, though this may be less a result of "the pulsing of the high tides" since tidal ranges are decreasing, rather than more an increase in general groundwater levels. It should be noted that in early December, during the week leading upto the monitoring period, there had been some heavy rain on odd days.

The Tidal Effects Groundwater Monitoring details are include within the main monitoring data.

LOCATION	Pipe	Pipe	Pipe	A.O.D.	DATE	DATE	DATE	DATE	DATE	DATE	DATE	DATE	DATE	DATE	DATE	DATE	DATE	DATE	DATE	DATE	DATE	DATE		
#1 Burley Road, Felpham		Depth	Height	Ground Level at Pipe Top of Pipe Level			Some rain in the week and Sat	Heavy rain on some days								Heavy rain start of week	Rained all day on Thursday	Dry mostly all week	No Rain DRY sunny cold week	Heavy rain at end of week	Rain at end of week	Some rain in the week	Heavy rain early in the week	
Weekly Rainfall Amount at BN16 (mm)						8.5	35.0 on Saturday 6th								21.5	25	3.5	0.0	37.0	35.0	17.0	45.0		
Weekly Rainfall Recording Date							High Tide	Low Tide	High Tide	Low Tide	High Tide	Low Tide	High Tide	Low Tide	on Sat 13th	on Sat 20th	on Sat 27th	on Sat 3rd January '26	on Sat 10th	on Sat 17th	on Sat 24th	on Sat 31st		
Met Office Named Storms																			Goretti	Ingrid	Chandra			
Saturday 29th November 2025					Saturday 29-Nov 2025	Saturday 29-Nov	Sunday 30-Nov	10:01 Thursday 4-Dec	16:10 Thursday	11:49 Friday 5-Dec	17:55 Friday	13:23 Saturday 6-Dec	19:29 Saturday	15:30 Sunday 7-Dec	Saturday 13-Dec	Saturday 20-Dec	Saturday 27-Dec	Saturday 3-Jan	Saturday 10-Jan	Saturday 17-Jan	Friday 23-Jan	Saturday 31-Jan		
Maximum Depth	GWP #1	2670	164		2042	2426	2402	2052	1924	1987	1964	1795	1839	1759	1733	2070	1901	2291	2478	2192	1851	1905	1761	
Below Ground Level	(south)	-2506		5.900	-2042	-2262	-2238	-1888	-1760	-1823	-1800	-1631	-1675	-1595	-1569	-1906	-1737	-2127	-2314	-2028	-1687	-1741	-1597	
Level Above O.D.		3.394		6.064	3.858	3.638	3.662	4.012	4.140	4.077	4.100	4.269	4.225	4.305	4.331	3.994	4.163	3.773	3.586	3.872	4.213	4.159	4.303	
Change from Previous						-220	24	350	128	-63	23	169	-44	80	26	-337	169	-390	-187	286	341	-54	144	
Water Strike Below Ground Level		1900																						
					Reading BGL on Installation	Reading ToP on Leaving															1972	1681	1730	1470



Groundwater Monitoring -- in Groundwater Point GWP#1

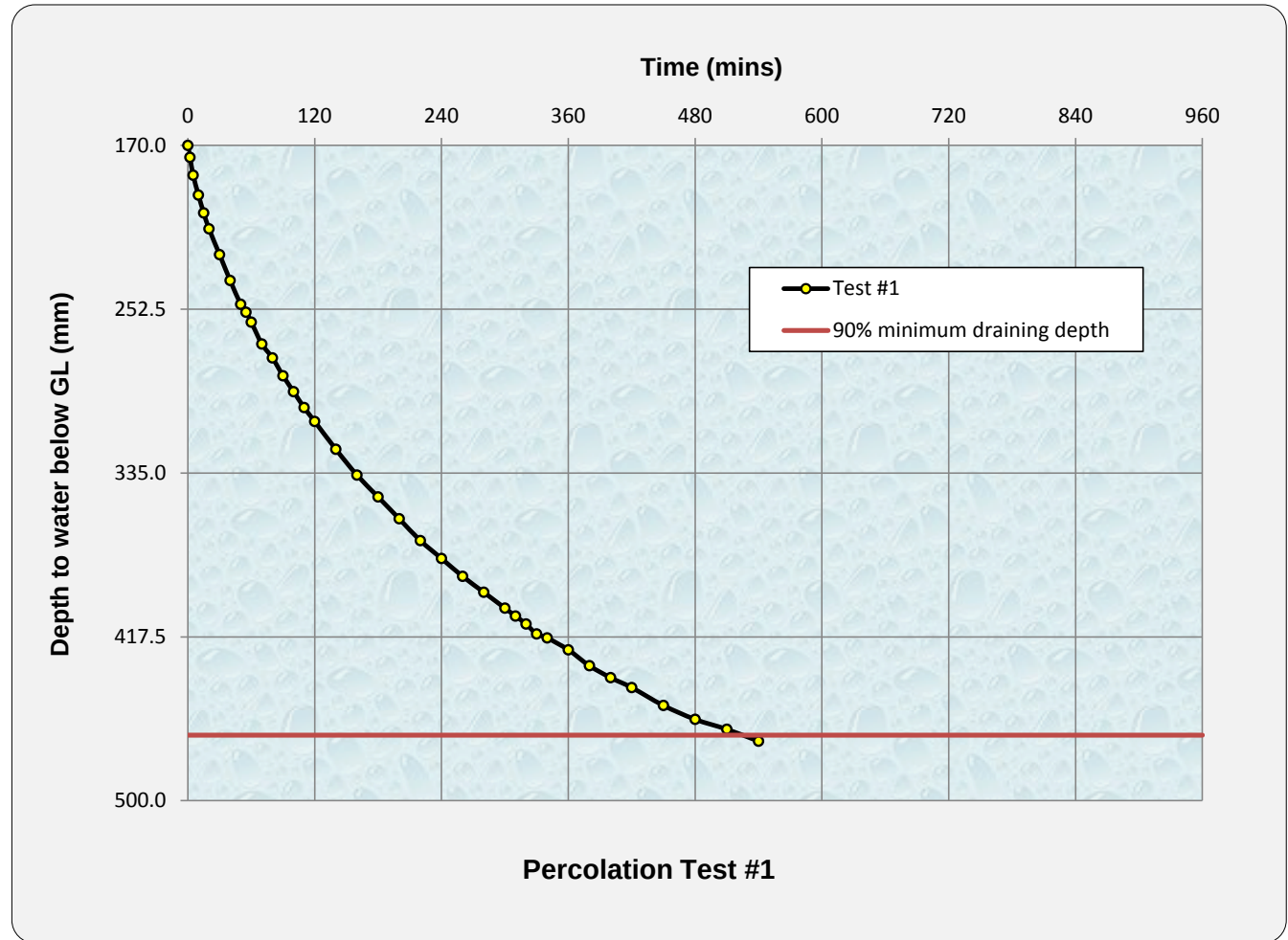
Testing Time / Date	Time (mins)	Depth to Water Surface	90% depth
08:19:00	0	170	467
	2	176	
	5	185	
	10	195	
	15	204	
	20	212	
	30	225	
	40	238	
	50	250	
	55	254	
	60	259	
	70	270	
	80	277	
	90	286	
	100	294	
	110	302	
	120	309	
	140	323	
	160	336	
	180	347	
	200	358	
13:29:00	220	369	
	240	378	
	260	387	
	280	395	
	300	403	
	310	407	
14:59:00	320	411	467
	330	416	
	340	418	
	360	424	
	380	432	
	400	438	
16:19:00	420	443	
	450	452	
	480	459	
17:19:00	510	464	
	540	470	

#1 Burley Road, Summerley Estate, Felpham

Location -- Garden South

Soil Infiltration Test Number 1

1130mm Length x 490mm Width x 500mm Depth



$$\text{Soil infiltration rate } f = \frac{V_{p75-25}}{A_{s50} \times T_{p75-25} \times 60} = \frac{0.091}{1.088 \times 284.38 \times 60} = 4.920\text{E-}06 \text{ metres / second}$$

SOIL INFILTRATION RATE IS

4.920E-06 metres/second

Volume of Water Outflowing -- (V_{p75-25}) 0.091 Cubic Metres
Area of Outflow -- (A_{s50}) 1.088 Square Metres
Time Duration from 75% to 25% -- (T_{p75-25}) 284.38 Minutes

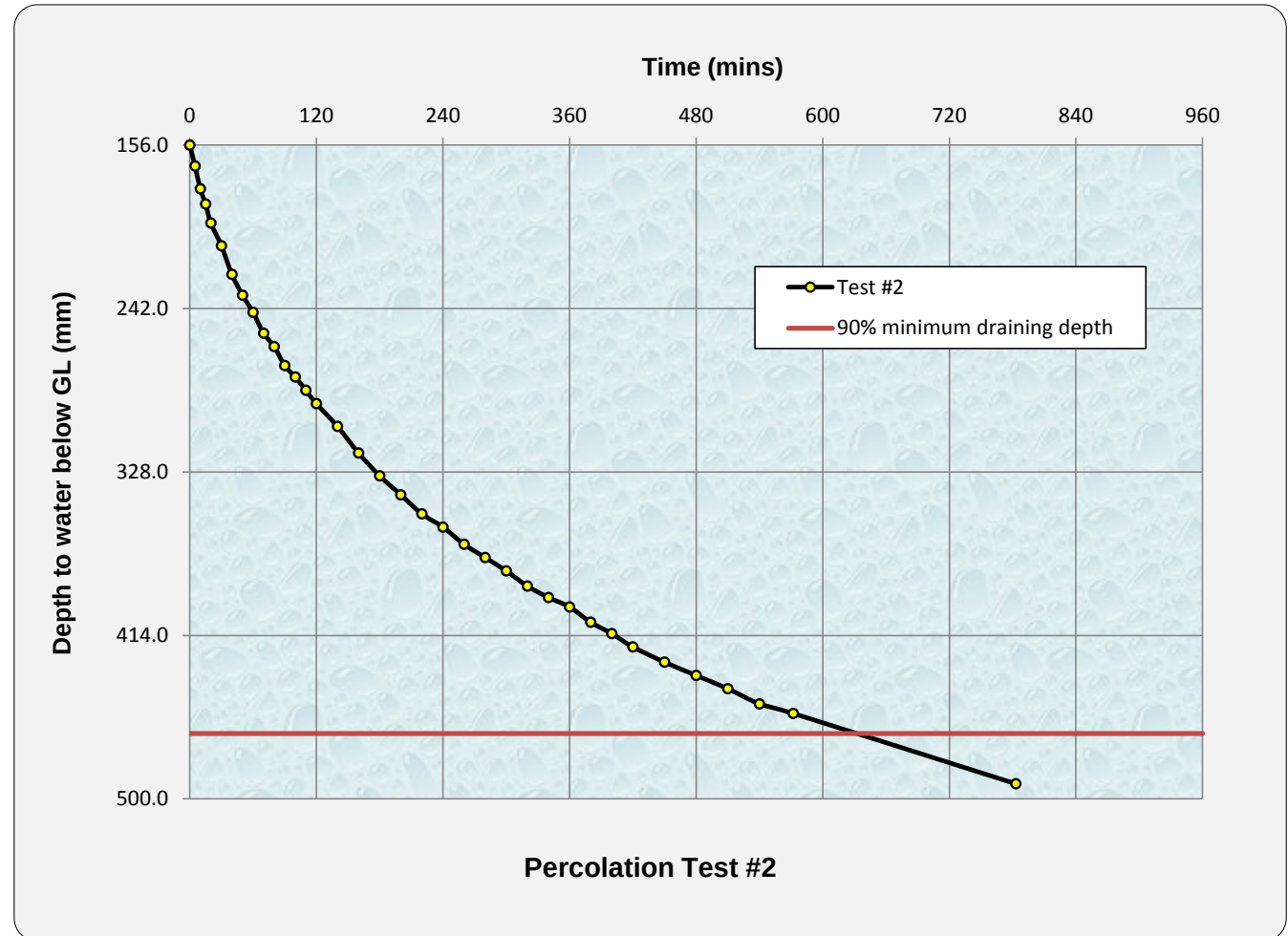
Testing Time / Date	Time (mins)	Depth to Water Surface	90% depth
08:02:00	0	156	466
	5	167	
	10	179	
	15	187	
	20	197	
	30	209	
	40	224	
	50	235	
09:02:00	60	244	
	70	255	
	80	262	
	90	272	
	100	278	
10:02:00	110	285	
	120	292	
	140	304	
11:02:00	160	318	
	180	330	
	200	340	
12:02:00	220	350	
	240	357	
	260	366	
13:02:00	280	373	
	300	380	
	320	388	
	340	394	
14:02:00	360	399	
	380	407	
	400	413	
15:02:00	420	420	
	450	428	
16:02:00	480	435	
	510	442	
17:02:00	540	450	
17:32:00	572	455	
21:05:00	783	492	

#1 Burley Road, Summerley Estate, Felpham

Location -- Garden South

Soil Infiltration Test Number 2

1130mm Length x 490mm Width x 500mm Depth



$$\text{Soil infiltration rate } f = \frac{V_{p75-25}}{A_{S50} \times T_{p75-25} \times 60} = \frac{0.095}{1.111 \times 343.76 \times 60} = 4.156\text{E-}06 \text{ metres / second}$$

SOIL INFILTRATION RATE IS

4.156E-06 metres/second

Volume of Water Outflowing -- (V_{p75-25}) 0.095 Cubic Metres
Area of Outflow -- (A_{S50}) 1.111 Square Metres
Time Duration from 75% to 25% -- (T_{p75-25}) 343.76 Minutes

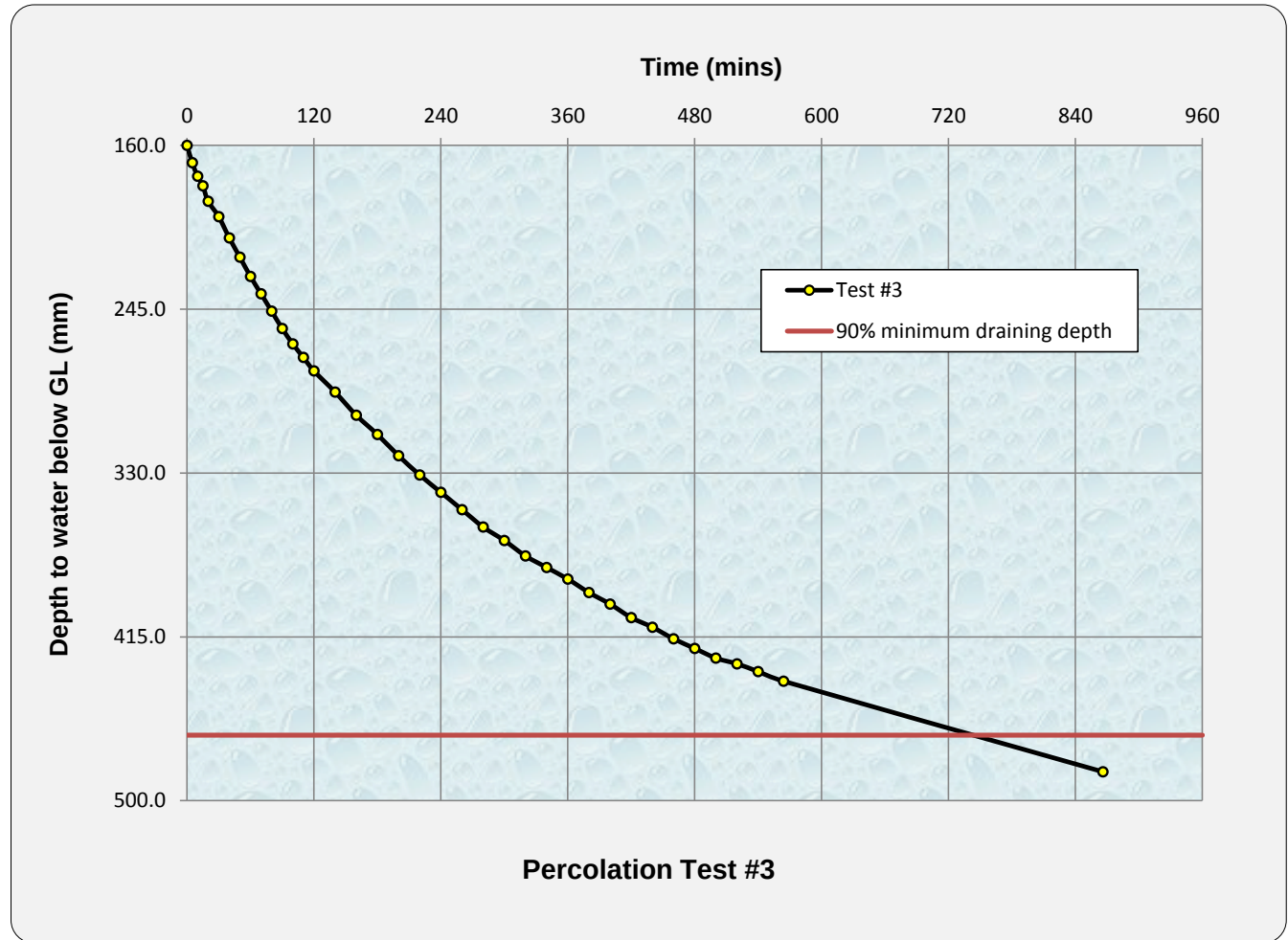
Testing Time / Date	Time (mins)	Depth to Water Surface	90% depth
08:07:00	0	160	466
	5	169	
	10	176	
	15	181	
	20	189	
	30	197	
	40	208	
	50	218	
	60	228	
	70	237	
	80	246	
	90	255	
	100	263	
	110	270	
	120	277	
	140	288	
10:07:00	160	300	
	180	310	
	200	321	
	220	331	
12:07:00	240	340	
	260	349	
	280	358	
	300	365	
	320	373	
	340	379	
	360	385	
	380	392	
	400	398	
	420	405	
	440	410	
	460	416	
	480	421	
	500	426	
	520	429	
16:07:00	540	433	
17:07:00	564	438	
22:33:00	866	485	

#1 Burley Road, Summerley Estate, Felpham

Location -- Garden South

Soil Infiltration Test Number 3

1130mm Length x 490mm Width x 500mm Depth



$$\text{Soil infiltration rate } f = \frac{V_{p75-25}}{A_{s50} \times T_{p75-25} \times 60} = \frac{0.094}{1.105 \times 379.29 \times 60} = 3.745\text{E-}06 \text{ metres / second}$$

SOIL INFILTRATION RATE IS

3.745E-06 metres/second

Volume of Water Outflowing -- (V_{p75-25}) 0.094 Cubic Metres
Area of Outflow -- (A_{s50}) 1.105 Square Metres
Time Duration from 75% to 25% -- (T_{p75-25}) 379.29 Minutes

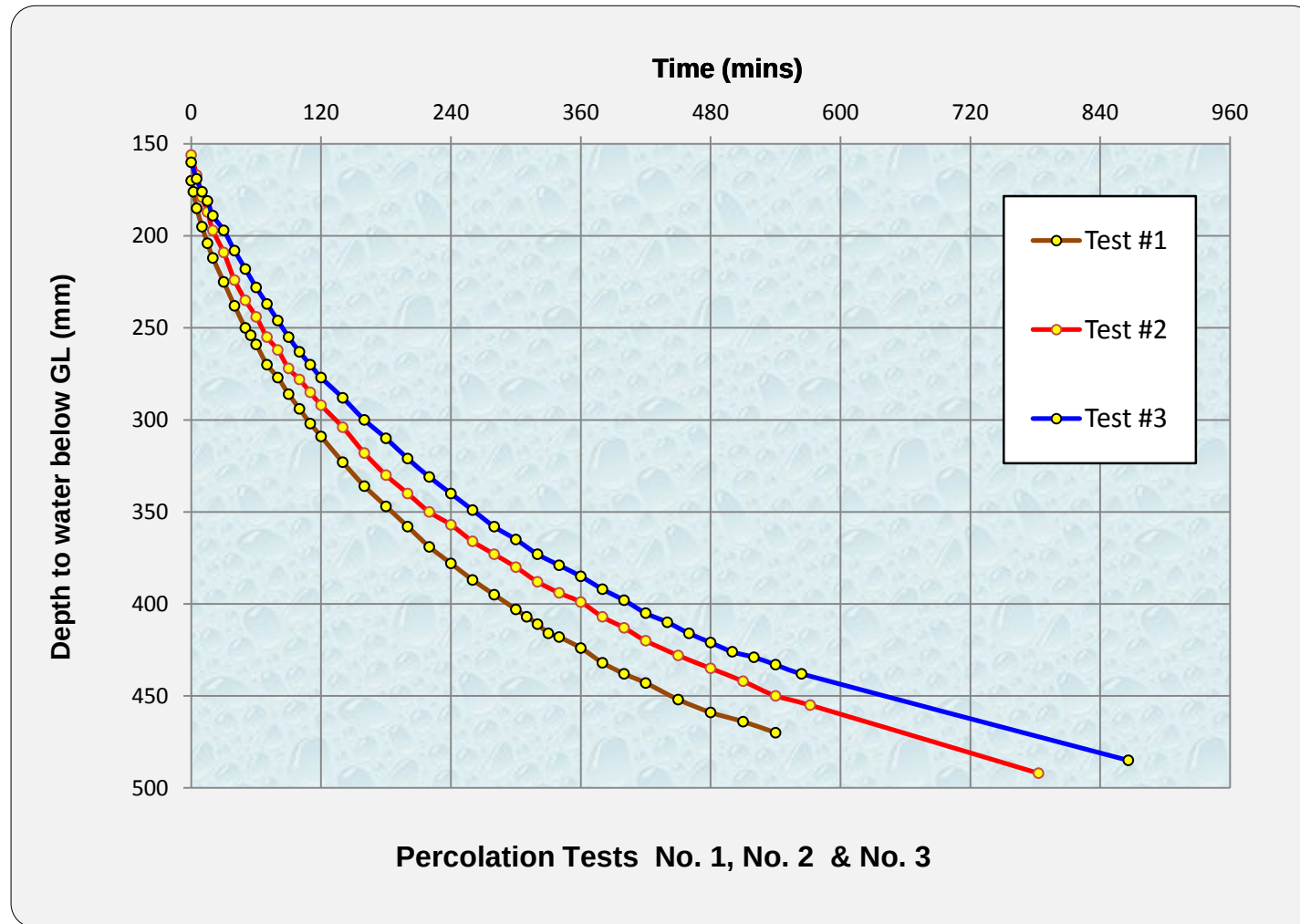
#1 Burley Road, Summerley Estate, Felpham

Soil Infiltration Test Trial Pit TP#1

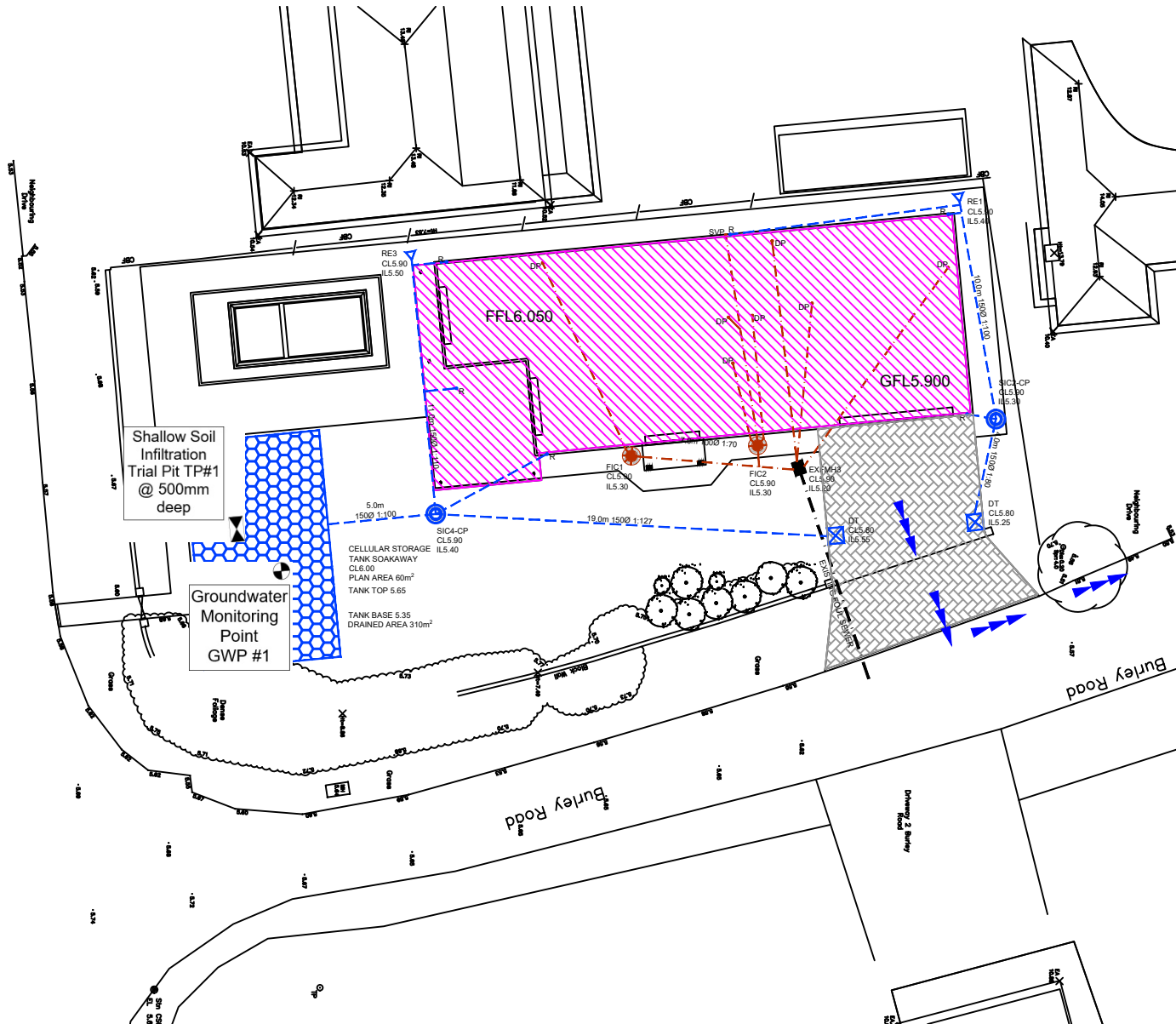
Location -- Garden South

Soil Infiltration Tests #1, #2 and #3

1130mm Length x 490mm Width x 500mm Depth



Shallow S
Infiltration
Trial Pit



DRAWING LEGEND		NOTES
SURFACE WATER DRAINAGE		CONTRACTORS MUST VERIFY ALL DIMENSIONS ON SITE BEFORE COMMENCING ANY WORK ON SHOP DRAWINGS DO NOT SCALE FROM THIS DRAWING RCD CONSULTANTS LTD COPYRIGHT
SIC	INSPECTION CHAMBER CP DENOTES CATCHPIT	NOTES 1. ALL DIMENSIONS IN MILLIMETERS UNLESS NOTED OTHERWISE. 2. THIS DRAWING TO BE READ IN CONJUNCTION WITH ALL OTHER ENGINEERING DRAWINGS AND CALCULATIONS ASSOCIATED WITH THIS PROJECT. 3. ALL COMPONENTS AND MATERIALS ARE TO BE MANUFACTURED AND SUPPLIED IN ACCORDANCE WITH THE RELEVANT BRITISH STANDARDS, AND LAD AND BACKFILLED IN ACCORDANCE WITH MANUFACTURERS' INSTRUCTIONS AND THE RELEVANT BRITISH STANDARDS. 4. THE CONTRACTOR SHALL, BEFORE COMMENCING THE WORKS, VERIFY ALL SITE AND SETTING OUT DIMENSIONS. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE TRUE AND PROPER SETTING OUT OF THE WORKS AND FOR THE CORRECTNESS OF THE POSITION, LEVELS, DIMENSIONS, AND ALIGNMENT OF ALL PARTS OF THE WORKS. 5. ALL BUILDING DRAINAGE TO BE INSTALLED AND TESTED IN COMPLIANCE WITH THE BUILDING REGULATIONS 2000 DRAINAGE AND WASTE DISPOSAL APPROVED DOCUMENT H 2002 EDITION. 6. SMALL LIGHTWEIGHT ACCESS COVERS SHOULD BE SECURED FOR EXAMPLE WITH SCREWS TO DETER UNAUTHORISED ACCESS. 7. ALL ABOVE GROUND DRAINAGE TO INCORPORATE RODDING ACCESS FACILITIES. 8. INSITU CONCRETE FOR USE IN GENERAL DRAINAGE WORKS, GRADE TO BS: 5328. 9. FOR INTERNAL DRAINAGE POSITIONS REFER TO ARCHITECT'S CURRENT WORKING DRAWINGS. 10. FOR LANDSCAPING SURFACE MATERIAL FINISHES REFER TO ARCHITECT'S CURRENT WORKING DRAWINGS. 11. ALL PIPES TO BE 100mm DIAMETER UNLESS NOTED OTHERWISE.
---	SURFACE WATER SEWER	
R+	RAINWATER PIPE	
RE	RODDING EYE 450mm THICK POROUS	
115m	PAVING SINGLE DRIVE CONSTRUCTION	
20m	ROOF AREA (INCL 10% URBAN CREEP)	
FOUL WATER DRAINAGE		
---	EXISTING FOUL WATER SEWER	
FIC	INSPECTION CHAMBER	
---	FOUL WATER SEWER	
DP	DRAINAGE POINT	

EXISTING AND PROPOSED
FLOOD EXCEEDANCE ROUTE

Shallow Soil
Infiltration
Trial Pit TP#1
@ 500mm
deep

Groundwater
Monitoring
Point
GWP #1

P7	PRELIMINARY ISSUE	05.06.2025	RAC
P6	PRELIMINARY ISSUE	07.04.2025	RAC
P5	PRELIMINARY ISSUE	14.04.2025	RAC
P4	PRELIMINARY ISSUE	23.03.2025	RAC
P3	PRELIMINARY ISSUE	16.08.2024	RAC
P2	PRELIMINARY ISSUE	10.06.2025	RAC
P1	PRELIMINARY ISSUE	09.06.2025	RAC
REV	AMENDMENT	DATE	CHRD

PRELIMINARY

RCD

RCD CONSULTANTS LTD
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CLIENT	
PROJECT	PROPOSED RESIDENTIAL DEVELOPMENT 1 BURLEY ROAD FELPHAM, PO22 7NF
DRAWING TITLE	DRAINAGE LAYOUT

CIVILS

SCALE	DRAWN BY	CHECKED	DATE
1:100 @ A1 1:200 @ A3	STC	RAC	JUN 2025
DRAWING NUMBER	REVISION		
1111-2528-CIV-10	P7		