

TECHNICAL NOTE

PROJECT NAME	DAPPERS LANE, ANGMERING
PROJECT NUMBER	23 086
NOTE REF	TN 002
DATE	10 TH JUNE 2026
PREPARED BY	Andrew Rimmington
SUBJECT	RESPONSE TO ARUN DISTRICT COUNCIL
PLANNING REF	A/43/26/PL

This technical note has been prepared to address the comments raised by Arun District Council in their memo note of the 18th May 2026. This note has been structured to address the comments in the context of the National Standards for SuDS as issued in July 2026.

1. PROPOSED DRAINAGE

1.1 STANDARD 1 – RUNOFF DESTINATIONS

REQUIREMENTS

- 1.1 A 'SuDS approach' shall be adopted to address the management of surface water by the development and where it should be discharged. Runoff shall be treated as a resource and managed in a way that avoids negative impacts of the development on flood risk, the morphology and water quality of receiving waters and the associated ecology.
- 1.2 Runoff from the development shall be discharged to the following final destinations, to the maximum extent practicable, in accordance with the below hierarchy:
- Priority 1: Collected for non-potable use
 - Priority 2: Infiltrated to ground
 - Priority 3: Discharged to an above ground surface water body
 - Priority 4: Discharged to a surface water sewer, or another piped surface water drainage system
 - Priority 5: Discharged to a combined sewer

The proposed scheme utilises soakaways to infiltrate surface water to ground; Priority 2.

From the comments received this is noted as acceptable to the LLFA and this item is closed.

1.2 STANDARD 2 – MANAGEMENT OF EVERYDAY RAINFALL (INTERCEPTION)

REQUIREMENTS

- 2.1 Apply a 'SuDS approach' so that at least the first 5mm of rainfall for the majority of rainfall events does not result in runoff from the site to surface waters or piped drainage systems.
- 2.2 Evidence shall be provided that the approach to managing runoff from 'everyday' rainfall has been developed alongside and in support of the management of runoff quality (standard 4) and the delivery of amenity and biodiversity benefits (standards 5 and 6).
- 2.6 Evidence shall be provided to demonstrate that the runoff from each positively drained surface, for at least 5mm of rainfall, is either collected for use, infiltrated into the ground, or else captured, conveyed and stored within SuDS features.
- These features shall naturally absorb or retain runoff and from these the runoff will be 'lost' to soils or the atmosphere and will not discharge off the site
- 2.7 The interception methods below shall be considered to be compliant for zero runoff for the first 5mm rainfall for 80% of events during the summer and 50% in winter, with the following presumptions for each system.

The proposed scheme utilises soakaways to infiltrate surface water to ground. As such, interception of the first 5mm of rainfall is appropriately managed with no run-off leaving site.

From the comments received this is noted as acceptable to the LLFA and this item is closed.

1.3 STANDARD 3 – MANAGEMENT OF EXTREME RAINFALL AND FLOODING

REQUIREMENTS

- 3.1 A 'SuDS approach' shall be adopted to address the management of development runoff during extreme rainfall, including allowances for climate change and urban creep to:
 - Protect people and property on the development from flooding of the surface water drainage system
 - Mitigate any increased flood risk to people and property adjacent to or downstream of the development
 - Protect the receiving water body from morphological damage or minimise the impact on sewer capacity
- 3.2 When discharging to an infiltration feature, the system shall be appropriately sized to accommodate the design event based on ground conditions and contributing areas.
- 3.3 When discharging to an above ground surface water body, sewer or other piped drainage system, the surface water runoff (rate and volume) for the 1% annual exceedance probability (AEP) event shall be controlled to ensure the runoff from the development does not increase flood risk elsewhere.
- 3.4 When discharging to an above ground surface water body, sewer or other piped drainage system, the surface water runoff rate for the 50% AEP event shall be controlled to ensure development runoff from an event of this magnitude has no negative impact.
- 3.5 Any flooding from the surface water drainage system for events up to the 1% AEP event shall be managed within the development.
- 3.6 Any flooding from off-site sources for the 1% AEP event should be managed on site or safely routed through the site, ensuring any downstream risks are not increased compared to the pre-development scenario.
- 3.7 The risks (both on and off the development) associated with flooding from the surface water drainage system for exceedance events greater than the 1% AEP event shall be appropriately managed.

Managing Development Runoff

A number of items have been raised in respect of Standard 3 – Management of Extreme Rainfall and Flooding. These have been addressed as noted below.

- 3.1. Calculations are provided in **Appendix A** for all areas including the permeable pavement design.
- 3.2. All calculations demonstrate the 50% half drain down time lies within 24hrs for the 30yr event.
- 3.3. Drawing 5010 has been amended to shown the overall, total impermeable area including the permeable paved areas.

- 3.4. An impermeable service corridor route is to be provided and is shown on drawing 5001. This will shed water to the adjacent permeable area. A cross section detail has also been added to show the transition detail.
- 3.5. Root Protection Zones have been added to drawing D 5001, which corresponds with the proposed landscaping GAB24003 – 201G.
- 3.6. Permeable pavement collector drain shown in the Key on drawing D 5001 has been removed as it is not required.
- 3.7. Drawing 5001 key shows a flow control – this has been removed as it is not required.
- 3.8. A water-stop kerb detail has been added to Drawing 5001 to show the construction where the permeable pavement abuts the spine road to mitigate overland and sub-base flows?
- 3.9 Drawing 5011 with exceedance routing has been updated with additional levels and contours.

Based on the above amendments and clarifications, it is considered that the proposed SuDS strategy complies with Standard 3. Updated drawings are also provided in **Appendix B**.

1.4 STANDARD 4 – WATER QUALITY

REQUIREMENTS

- 4.1 Apply a 'SuDS approach' that protects surface waters, groundwater and coastal waters by managing the quality of the surface water runoff to adequately address water quality risks from the development.
- 4.2 The proposed SuDS management train(s) shall be based on a robust water quality risk assessment, appropriate to the pollution hazard and sensitivity of receiving waters, reflecting industry recognised guidance²¹ or other quantitative assessment as agreed with the approving body and permitting requirements²²

21 - As set out in Chapter 4, Chapter 26 and Chapter 27 of C753 The SuDS Manual (CIRIA) or any future version.

22 – An Environmental Permit may be required under Paragraph 3 (1) (c) of Schedule 22 of the Environmental Permitting (England and Wales) Regulations 2016 if there is a risk of mobilising existing contamination.

The proposed scheme utilises permeable paving prior to disposal of surface water by means of soakaway.

From the comments received this is noted as acceptable to the LLFA and this item is closed.

1.5 STANDARD 5 – AMENITY

REQUIREMENTS

- 5.1 A 'SuDS approach' shall be adopted that maximises benefits for amenity through the creation of multi-functional places and landscapes.

The proposed SuDS strategy has been developed in conjunction with the landscape proposals, to maximise amenity and placemaking benefits. The inclusion of landscape based features has largely been discounted due to the potential for damage in the area of Public Open Space which is expected to be used for recreational purposes / light ball-games.

Due to the topography of the site, it is not considered appropriate to place SuDs feature any closer to the south east boundary as these would have the potential for water to manifest through the embankment and cause flooding on Water Lane

1.6 STANDARD 6 – BIODIVERSITY

REQUIREMENTS

6.1 A 'SuDS approach' shall be adopted to ensure the surface water drainage system maximises biodiversity benefits throughout the development lifecycle.

6.2 The surface water drainage system shall add biodiversity value by:

- Creating diverse, self-sustaining, resilient local ecosystems³⁴ which contribute to net gains in biodiversity.
- Supporting and promoting natural local habitat and species, for example, through local nature recovery strategies (LNRS)³⁵
- Contributing to the delivery of local biodiversity strategies.
- Contributing to habitat connectivity.

34 - The UK Climate Resilience Programme

35 - Local Nature Recovery Strategies: how to prepare and what to include - Defra - Citizen Space

SuDS components have been integrated into the site layout in the form of permeable paving. This has been done to maximize the usable area of public open space. The Public Open Space will, in itself provide a degree of bio diversity and habitat for local species on the grassed embankment.

1.7 STANDARD 7 – DESIGN OF DRAINAGE FOR CONSTRUCTION, OPERATION, MAINTENANCE, DECOMMISSIONING AND STRUCTURAL INTEGRITY

REQUIREMENTS

- 7.1 A 'SuDS approach' shall be adopted to ensure that surface water drainage systems are designed ³⁷ so they can be easily and safely constructed, operated and maintained taking account of the need to minimise negative impacts on natural resources and the environment.
- 7.2 The designer shall provide a management and maintenance plan ³⁸ that supports the design objectives detailed in standards 1 to 6 and ensures the performance of the surface water drainage system with regards to runoff destinations, everyday and extreme rainfall, water quality, amenity and biodiversity is maintained throughout the lifetime of the development.
- 7.3 Surface water drainage design shall examine for the likelihood and consequences of potential failure scenarios that may occur during the operation phase and safely manage the associated risks.
- 7.4 The surface water drainage system shall be designed to ensure structural integrity of all components under anticipated loading conditions for the design life of the development so that it does not affect the structural integrity of any existing or proposed components within, or adjacent to, the development.

37 - BS EN 752:2017 Drain and sewer systems outside buildings. Sewer system management.

38 – See C753 The SuDS Manual (CIRIA) or any future version.

The proposed SuDS strategy has been designed to be constructible, operable and maintainable throughout the development lifecycle, in accordance with CIRIA C753 guidance. A maintenance strategy has previously been submitted, but is included again for record purpose in **Appendix C**.

The drainage proposals have also been reviewed against the landscaping GA for potential conflicts with proposed trees.

APPENDIX A

Design Settings

Rainfall Methodology	FEH-22	Minimum Velocity (m/s)	1.00
Return Period (years)	100	Connection Type	Level Soffits
Additional Flow (%)	45	Minimum Backdrop Height (m)	0.200
CV	1.000	Preferred Cover Depth (m)	0.750
Time of Entry (mins)	5.00	Include Intermediate Ground	✓
Maximum Time of Concentration (mins)	30.00	Enforce best practice design rules	x
Maximum Rainfall (mm/hr)	50.0		

Nodes

Name	Area (ha)	T of E (mins)	Cover Level (m)	Easting (m)	Northing (m)	Depth (m)	Invert Level (m)
Area 2			15.390	507353.639	104746.040	0.430	14.960
Area 2 Inlet	0.024	5.00	15.390	507350.639	104746.040	0.379	15.011
Area 1			15.790	507315.111	104737.843	0.430	15.360
Area 1 Inlet	0.035	5.00	15.790	507312.111	104737.843	0.379	15.411
Area 3			13.990	507388.591	104716.370	0.430	13.560
Area 3 Inlet	0.068	5.00	13.990	507385.591	104716.370	0.379	13.611

Links

Name	US Node	DS Node	Length (m)	ks (mm) / n	US IL (m)	DS IL (m)	Fall (m)	Slope (1:X)	Dia (mm)	T of C (mins)	Rain (mm/hr)
2.000	Area 2 Inlet	Area 2	3.000	0.600	15.011	14.960	0.051	58.8	100	5.05	50.0
3.000	Area 3 Inlet	Area 3	3.000	0.600	13.611	13.560	0.051	58.8	150	5.04	50.0
1.000	Area 1 Inlet	Area 1	3.000	0.600	15.411	15.360	0.051	58.8	150	5.04	50.0

Name	Vel (m/s)	Cap (l/s)	Flow (l/s)	US Depth (m)	DS Depth (m)	Σ Area (ha)	Σ Add Inflow (l/s)	Pro Depth (mm)	Pro Velocity (m/s)
2.000	1.006	7.9	6.3	0.279	0.330	0.024	0.0	68	1.116
3.000	1.314	23.2	17.8	0.229	0.280	0.068	0.0	99	1.446
1.000	1.314	23.2	9.2	0.229	0.280	0.035	0.0	65	1.236

Pipeline Schedule

Link	Length (m)	Slope (1:X)	Dia (mm)	Link Type	US CL (m)	US IL (m)	US Depth (m)	DS CL (m)	DS IL (m)	DS Depth (m)
2.000	3.000	58.8	100	Circular	15.390	15.011	0.279	15.390	14.960	0.330
3.000	3.000	58.8	150	Circular	13.990	13.611	0.229	13.990	13.560	0.280
1.000	3.000	58.8	150	Circular	15.790	15.411	0.229	15.790	15.360	0.280

Link	US Node	Node Type	DS Node	Node Type
2.000	Area 2 Inlet	Junction	Area 2	Junction
3.000	Area 3 Inlet	Junction	Area 3	Junction
1.000	Area 1 Inlet	Junction	Area 1	Junction

Simulation Settings

Rainfall Methodology	FEH-22	Analysis Speed	Detailed	Starting Level (m)	
Rainfall Events	Singular	Skip Steady State	x	Check Discharge Rate(s)	x
Summer CV	1.000	Drain Down Time (mins)	1440	Check Discharge Volume	x
Winter CV	1.000	Additional Storage (m ³ /ha)	0.0		

Storm Durations

15 | 30 | 60 | 120 | 180 | 240 | 360 | 480 | 600 | 720 | 960 | 1440

Return Period (years)	Climate Change (CC %)	Additional Area (A %)	Additional Flow (Q %)
2	0	10	0
10	0	10	0
30	0	10	0
100	45	10	0

Node Area 2 Carpark Storage Structure

Base Inf Coefficient (m/hr)	0.28440	Invert Level (m)	14.960	Slope (1:X)	9999.9
Side Inf Coefficient (m/hr)	0.00000	Time to half empty (mins)	12	Depth (m)	0.300
Safety Factor	3.0	Width (m)	15.371	Inf Depth (m)	
Porosity	0.30	Length (m)	15.371		

Node Area 1 Carpark Storage Structure

Base Inf Coefficient (m/hr)	0.28440	Invert Level (m)	15.360	Slope (1:X)	9999.9
Side Inf Coefficient (m/hr)	0.00000	Time to half empty (mins)	12	Depth (m)	0.300
Safety Factor	3.0	Width (m)	18.647	Inf Depth (m)	
Porosity	0.30	Length (m)	18.647		

Node Area 3 Carpark Storage Structure

Base Inf Coefficient (m/hr)	0.28440	Invert Level (m)	13.560	Slope (1:X)	9999.9
Side Inf Coefficient (m/hr)	0.00000	Time to half empty (mins)	12	Depth (m)	0.300
Safety Factor	3.0	Width (m)	26.170	Inf Depth (m)	
Porosity	0.30	Length (m)	26.170		

Results for 2 year +10% A Critical Storm Duration. Lowest mass balance: 100.00%

Node Event	US Node	Peak (mins)	Level (m)	Depth (m)	Inflow (l/s)	Node Vol (m³)	Flood (m³)	Status
30 minute summer	Area 2	21	14.978	0.018	4.5	1.2343	0.0000	OK
15 minute summer	Area 2 Inlet	10	15.069	0.058	5.0	0.0000	0.0000	OK
30 minute summer	Area 1	21	15.378	0.018	6.5	1.7926	0.0000	OK
15 minute summer	Area 1 Inlet	10	15.470	0.059	7.3	0.0000	0.0000	OK
30 minute summer	Area 3	21	13.578	0.018	12.6	3.4051	0.0000	OK
15 minute summer	Area 3 Inlet	10	13.695	0.084	14.1	0.0000	0.0000	OK

Link Event (Upstream Depth)	US Node	Link	DS Node	Outflow (l/s)	Velocity (m/s)	Flow/Cap	Link Vol (m³)
30 minute summer	Area 2	Infiltration		2.3			
15 minute summer	Area 2 Inlet	2.000	Area 2	5.0	2.094	0.629	0.0078
30 minute summer	Area 1	Infiltration		3.3			
15 minute summer	Area 1 Inlet	1.000	Area 1	7.3	2.370	0.313	0.0105
30 minute summer	Area 3	Infiltration		6.5			
15 minute summer	Area 3 Inlet	3.000	Area 3	14.0	2.974	0.605	0.0162

Results for 10 year +10% A Critical Storm Duration. Lowest mass balance: 100.00%

Node Event	US Node	Peak (mins)	Level (m)	Depth (m)	Inflow (l/s)	Node Vol (m³)	Flood (m³)	Status
30 minute summer	Area 2	21	14.997	0.037	8.9	2.5220	0.0000	OK
15 minute summer	Area 2 Inlet	10	15.108	0.097	9.9	0.0000	0.0000	OK
30 minute summer	Area 1	21	15.396	0.036	12.9	3.6652	0.0000	OK
15 minute summer	Area 1 Inlet	10	15.496	0.085	14.4	0.0000	0.0000	OK
30 minute summer	Area 3	21	13.596	0.036	25.0	7.0487	0.0000	OK
15 minute summer	Area 3 Inlet	11	13.755	0.144	27.9	0.0000	0.0000	OK
Link Event (Upstream Depth)	US Node	Link	DS Node	Outflow (l/s)	Velocity (m/s)	Flow/Cap	Link Vol (m³)	
30 minute summer	Area 2	Infiltration		4.6				
15 minute summer	Area 2 Inlet	2.000	Area 2	9.7	2.210	1.226	0.0143	
30 minute summer	Area 1	Infiltration		6.6				
15 minute summer	Area 1 Inlet	1.000	Area 1	14.4	2.705	0.618	0.0185	
30 minute summer	Area 3	Infiltration		12.9				
15 minute summer	Area 3 Inlet	3.000	Area 3	27.8	3.070	1.196	0.0295	

Results for 30 year +10% A Critical Storm Duration. Lowest mass balance: 100.00%

Node Event	US Node	Peak (mins)	Level (m)	Depth (m)	Inflow (l/s)	Node Vol (m³)	Flood (m³)	Status
30 minute summer	Area 2	21	15.008	0.048	11.7	3.3567	0.0000	OK
15 minute summer	Area 2 Inlet	10	15.179	0.167	12.9	0.0000	0.0000	FLOOD RISK
30 minute summer	Area 1	21	15.408	0.048	17.1	4.8855	0.0000	OK
15 minute summer	Area 1 Inlet	10	15.513	0.102	18.7	0.0000	0.0000	OK
30 minute summer	Area 3	21	13.607	0.047	34.5	9.4146	0.0000	OK
15 minute summer	Area 3 Inlet	10	13.784	0.173	36.4	0.0000	0.0000	FLOOD RISK

Link Event (Upstream Depth)	US Node	Link	DS Node	Outflow (l/s)	Velocity (m/s)	Flow/Cap	Link Vol (m³)
30 minute summer	Area 2	Infiltration		6.0			
15 minute summer	Area 2 Inlet	2.000	Area 2	12.6	2.269	1.591	0.0165
30 minute summer	Area 1	Infiltration		8.8			
15 minute summer	Area 1 Inlet	1.000	Area 1	18.6	2.772	0.803	0.0236
30 minute summer	Area 3	Infiltration		17.1			
15 minute summer	Area 3 Inlet	3.000	Area 3	36.1	3.133	1.555	0.0323

Results for 100 year +45% CC +10% A Critical Storm Duration. Lowest mass balance: 100.00%

Node Event	US Node	Peak (mins)	Level (m)	Depth (m)	Inflow (l/s)	Node Vol (m³)	Flood (m³)	Status
30 minute summer	Area 2	23	15.065	0.105	21.5	7.3976	0.0000	OK
15 minute summer	Area 2 Inlet	9	15.390	0.379	23.5	0.0000	0.0000	FLOOD RISK
30 minute summer	Area 1	23	15.464	0.104	32.4	10.7461	0.0000	OK
15 minute summer	Area 1 Inlet	10	15.565	0.154	34.3	0.0000	0.0000	FLOOD RISK
30 minute summer	Area 3	23	13.663	0.103	61.5	20.7666	0.0000	OK
15 minute summer	Area 3 Inlet	9	13.990	0.379	66.6	0.0000	0.0000	FLOOD RISK

Link Event (Upstream Depth)	US Node	Link	DS Node	Outflow (l/s)	Velocity (m/s)	Flow/Cap	Link Vol (m³)
30 minute summer	Area 2	Infiltration		6.2			
15 minute summer	Area 2 Inlet	2.000	Area 2	23.1	3.341	2.918	0.0231
30 minute summer	Area 1	Infiltration		9.2			
15 minute summer	Area 1 Inlet	1.000	Area 1	34.0	2.764	1.466	0.0406
30 minute summer	Area 3	Infiltration		18.0			
15 minute summer	Area 3 Inlet	3.000	Area 3	66.0	4.808	2.845	0.0425

Design Settings

Rainfall Methodology	FEH-22	Minimum Velocity (m/s)	1.00
Return Period (years)	100	Connection Type	Level Soffits
Additional Flow (%)	45	Minimum Backdrop Height (m)	0.200
CV	1.000	Preferred Cover Depth (m)	0.750
Time of Entry (mins)	5.00	Include Intermediate Ground	✓
Maximum Time of Concentration (mins)	30.00	Enforce best practice design rules	x
Maximum Rainfall (mm/hr)	50.0		

Nodes

Name	Area (ha)	T of E (mins)	Cover Level (m)	Diameter (mm)	Easting (m)	Northing (m)	Depth (m)
MHSW02	0.026	5.00	15.770	450	507305.919	104742.146	1.090
Soakaway 1			15.720	1	507311.611	104742.146	1.100
MHSW07	0.041	5.00	15.360	450	507353.639	104730.067	1.075
Soakaway 2			15.390	1	507353.639	104731.628	1.140
MHSW15	0.038	5.00	14.125	450	507378.686	104710.965	1.065
Soakaway 3			13.990	1	507377.686	104710.965	0.940

Links

Name	US Node	DS Node	Length (m)	ks (mm) / n	US IL (m)	DS IL (m)	Fall (m)	Slope (1:X)	Dia (mm)	T of C (mins)	Rain (mm/hr)
1.002	MHSW02	Soakaway 1	5.692	0.600	14.680	14.620	0.060	94.9	150	5.09	50.0
3.003	MHSW07	Soakaway 2	1.561	0.600	14.285	14.250	0.035	44.6	150	5.02	50.0
7.006	MHSW15	Soakaway 3	1.000	0.600	13.060	13.050	0.010	100.0	150	5.02	50.0

Name	Vel (m/s)	Cap (l/s)	Flow (l/s)	US Depth (m)	DS Depth (m)	Σ Area (ha)	Σ Add Inflow (l/s)	Pro Depth (mm)	Pro Velocity (m/s)
1.002	1.032	18.2	6.8	0.940	0.950	0.026	0.0	64	0.959
3.003	1.511	26.7	10.7	0.925	0.990	0.041	0.0	66	1.427
7.006	1.005	17.8	10.0	0.915	0.790	0.038	0.0	81	1.034

Pipeline Schedule

Link	Length (m)	Slope (1:X)	Dia (mm)	Link Type	US CL (m)	US IL (m)	US Depth (m)	DS CL (m)	DS IL (m)	DS Depth (m)
1.002	5.692	94.9	150	Circular	15.770	14.680	0.940	15.720	14.620	0.950
3.003	1.561	44.6	150	Circular	15.360	14.285	0.925	15.390	14.250	0.990
7.006	1.000	100.0	150	Circular	14.125	13.060	0.915	13.990	13.050	0.790

Link	US Node	Dia (mm)	Node Type	MH Type	DS Node	Dia (mm)	Node Type	MH Type
1.002	MHSW02	450	Manhole	Adoptable	Soakaway 1	1	Manhole	Adoptable
3.003	MHSW07	450	Manhole	Adoptable	Soakaway 2	1	Manhole	Adoptable
7.006	MHSW15	450	Manhole	Adoptable	Soakaway 3	1	Manhole	Adoptable

Manhole Schedule

Node	Easting (m)	Northing (m)	CL (m)	Depth (m)	Dia (mm)	Connections	Link	IL (m)	Dia (mm)
MHSW02	507305.919	104742.146	15.770	1.090	450				
						0	1.002	14.680	150
Soakaway 1	507311.611	104742.146	15.720	1.100	1		1	1.002	14.620
								14.620	150
MHSW07	507353.639	104730.067	15.360	1.075	450				
						0	3.003	14.285	150
Soakaway 2	507353.639	104731.628	15.390	1.140	1		1	3.003	14.250
								14.250	150
MHSW15	507378.686	104710.965	14.125	1.065	450				
						0	7.006	13.060	150
Soakaway 3	507377.686	104710.965	13.990	0.940	1		1	7.006	13.050
								13.050	150

Simulation Settings

Rainfall Methodology	FEH-22	Analysis Speed	Detailed	Starting Level (m)
Rainfall Events	Singular	Skip Steady State	x	Check Discharge Rate(s) x
Summer CV	1.000	Drain Down Time (mins)	1440	Check Discharge Volume x
Winter CV	1.000	Additional Storage (m³/ha)	0.0	

Storm Durations

15	60	180	360	600	960	2160	4320	7200	10080
30	120	240	480	720	1440	2880	5760	8640	

Return Period (years)	Climate Change (CC %)	Additional Area (A %)	Additional Flow (Q %)
2	0	10	0
10	0	10	0
30	0	10	0
100	45	10	0

Node Soakaway 1 Soakaway Storage Structure

Base Inf Coefficient (m/hr)	0.00000	Invert Level (m)	13.970	Depth (m)	0.800
Side Inf Coefficient (m/hr)	0.28440	Time to half empty (mins)	877	Inf Depth (m)	
Safety Factor	3.0	Pit Width (m)	5.000	Number Required	1
Porosity	0.95	Pit Length (m)	7.000		

Node Soakaway 2 Soakaway Storage Structure

Base Inf Coefficient (m/hr)	0.00000	Invert Level (m)	13.600	Depth (m)	0.800
Side Inf Coefficient (m/hr)	0.28440	Time to half empty (mins)	1061	Inf Depth (m)	
Safety Factor	3.0	Pit Width (m)	5.000	Number Required	1
Porosity	0.95	Pit Length (m)	12.000		

Node Soakaway 3 Soakaway Storage Structure

Base Inf Coefficient (m/hr)	0.00000	Invert Level (m)	12.800	Depth (m)	0.400
Side Inf Coefficient (m/hr)	0.28440	Time to half empty (mins)	1519	Inf Depth (m)	
Safety Factor	3.0	Pit Width (m)	8.000	Number Required	1
Porosity	1.00	Pit Length (m)	12.000		

Results for 2 year +10% A Critical Storm Duration. Lowest mass balance: 100.00%

Node Event	US Node	Peak (mins)	Level (m)	Depth (m)	Inflow (l/s)	Node Vol (m³)	Flood (m³)	Status
15 minute summer	MHSW02	10	14.740	0.060	5.4	0.0096	0.0000	OK
960 minute summer	Soakaway 1	645	14.173	-0.447	0.6	6.7575	0.0000	OK
15 minute summer	MHSW07	10	14.357	0.072	8.5	0.0114	0.0000	OK
2880 minute summer	Soakaway 2	1800	13.797	-0.453	0.5	11.2263	0.0000	OK
15 minute summer	MHSW15	10	13.143	0.083	7.9	0.0133	0.0000	OK
1440 minute summer	Soakaway 3	990	12.921	-0.129	0.7	11.5808	0.0000	OK

Link Event (Upstream Depth)	US Node	Link	DS Node	Outflow (l/s)	Velocity (m/s)	Flow/Cap	Link Vol (m³)
15 minute summer	MHSW02	1.002	Soakaway 1	5.4	0.854	0.294	0.0358
960 minute summer	Soakaway 1	Infiltration		0.1			
15 minute summer	MHSW07	3.003	Soakaway 2	8.5	1.157	0.317	0.0114
2880 minute summer	Soakaway 2	Infiltration		0.2			
15 minute summer	MHSW15	7.006	Soakaway 3	7.9	0.870	0.442	0.0090
1440 minute summer	Soakaway 3	Infiltration		0.1			

Results for 10 year +10% A Critical Storm Duration. Lowest mass balance: 100.00%

Node Event	US Node	Peak (mins)	Level (m)	Depth (m)	Inflow (l/s)	Node Vol (m³)	Flood (m³)	Status
15 minute summer	MHSW02	10	14.772	0.092	10.7	0.0146	0.0000	OK
600 minute winter	Soakaway 1	465	14.293	-0.327	1.0	10.7379	0.0000	OK
15 minute summer	MHSW07	10	14.397	0.112	16.8	0.0178	0.0000	OK
600 minute winter	Soakaway 2	555	13.906	-0.344	1.6	17.4419	0.0000	OK
15 minute summer	MHSW15	10	13.194	0.134	15.6	0.0213	0.0000	OK
600 minute winter	Soakaway 3	585	12.981	-0.069	1.4	17.3758	0.0000	OK

Link Event (Upstream Depth)	US Node	Link	DS Node	Outflow (l/s)	Velocity (m/s)	Flow/Cap	Link Vol (m³)
15 minute summer	MHSW02	1.002	Soakaway 1	10.6	1.006	0.584	0.0602
600 minute winter	Soakaway 1	Infiltration		0.2			
15 minute summer	MHSW07	3.003	Soakaway 2	16.7	1.356	0.627	0.0192
600 minute winter	Soakaway 2	Infiltration		0.3			
15 minute summer	MHSW15	7.006	Soakaway 3	15.5	1.017	0.872	0.0151
600 minute winter	Soakaway 3	Infiltration		0.2			

Results for 30 year +10% A Critical Storm Duration. Lowest mass balance: 100.00%

Node Event	US Node	Peak (mins)	Level (m)	Depth (m)	Inflow (l/s)	Node Vol (m³)	Flood (m³)	Status
15 minute summer	MHSW02	10	14.791	0.111	13.9	0.0176	0.0000	OK
600 minute summer	Soakaway 1	480	14.367	-0.253	1.8	13.2088	0.0000	OK
15 minute summer	MHSW07	10	14.425	0.140	22.0	0.0222	0.0000	OK
480 minute winter	Soakaway 2	464	13.979	-0.271	2.3	21.6138	0.0000	OK
15 minute summer	MHSW15	10	13.237	0.177	20.4	0.0282	0.0000	SURCHARGED
720 minute winter	Soakaway 3	690	13.027	-0.023	1.6	21.8253	0.0000	OK

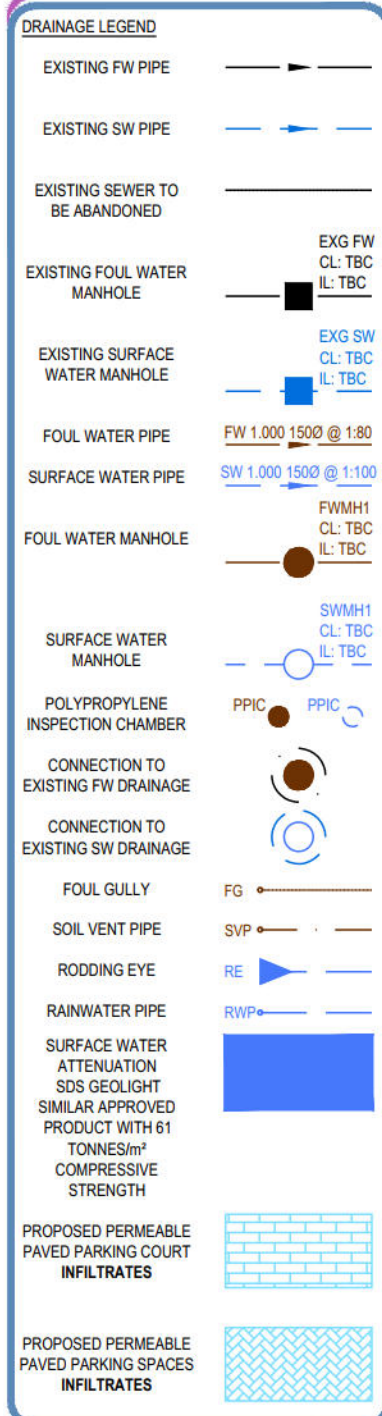
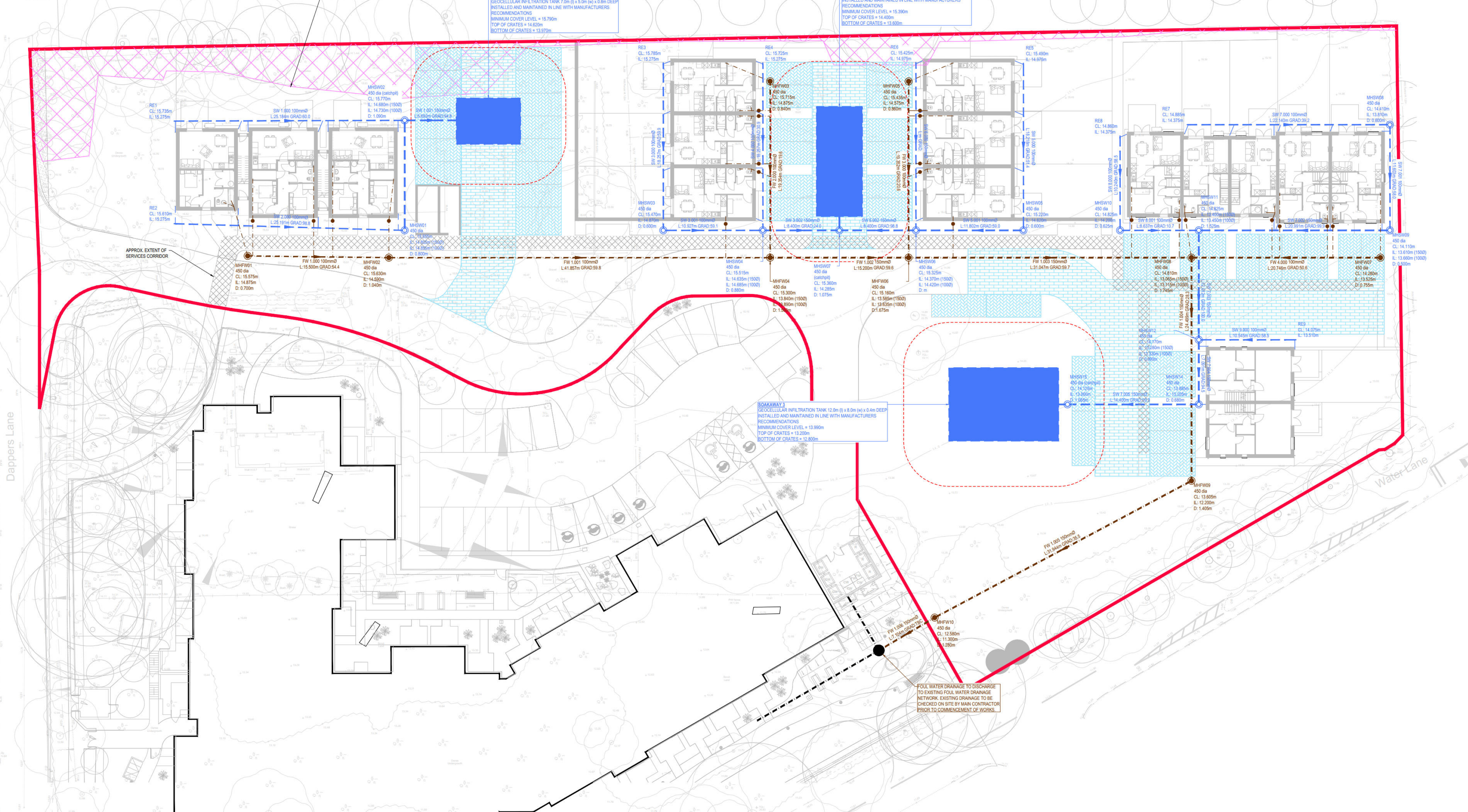
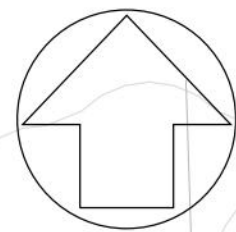
Link Event (Upstream Depth)	US Node	Link	DS Node	Outflow (l/s)	Velocity (m/s)	Flow/Cap	Link Vol (m³)
15 minute summer	MHSW02	1.002	Soakaway 1	13.8	1.062	0.759	0.0740
600 minute summer	Soakaway 1	Infiltration		0.3			
15 minute summer	MHSW07	3.003	Soakaway 2	21.9	1.433	0.820	0.0234
480 minute winter	Soakaway 2	Infiltration		0.3			
15 minute summer	MHSW15	7.006	Soakaway 3	20.3	1.156	1.142	0.0169
720 minute winter	Soakaway 3	Infiltration		0.2			

Results for 100 year +45% CC +10% A Critical Storm Duration. Lowest mass balance: 100.00%

Node Event	US Node	Peak (mins)	Level (m)	Depth (m)	Inflow (l/s)	Node Vol (m³)	Flood (m³)	Status
15 minute summer	MHSW02	10	14.933	0.253	25.4	0.0402	0.0000	SURCHARGED
480 minute winter	Soakaway 1	440	14.689	0.069	2.7	23.8904	0.0000	OK
15 minute summer	MHSW07	10	14.613	0.328	40.1	0.0521	0.0000	SURCHARGED
480 minute winter	Soakaway 2	456	14.284	0.034	4.2	39.0105	0.0000	OK
600 minute winter	MHSW15	585	13.396	0.336	3.3	0.0534	0.0000	SURCHARGED
600 minute winter	Soakaway 3	585	13.396	0.346	3.3	38.4480	0.0000	OK

Link Event (Upstream Depth)	US Node	Link	DS Node	Outflow (l/s)	Velocity (m/s)	Flow/Cap	Link Vol (m³)
15 minute summer	MHSW02	1.002	Soakaway 1	25.1	1.427	1.377	0.0986
480 minute winter	Soakaway 1	Infiltration		0.5			
15 minute summer	MHSW07	3.003	Soakaway 2	39.7	2.258	1.489	0.0272
480 minute winter	Soakaway 2	Infiltration		0.6			
600 minute winter	MHSW15	7.006	Soakaway 3	3.3	0.693	0.186	0.0176
600 minute winter	Soakaway 3	Infiltration		0.4			

APPENDIX B



DO NOT SCALE

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GENERAL NOTES

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- ALL WORK TO COMPLY TO CURRENT BUILDING REGULATIONS AND TO BE IN ACCORDANCE WITH THE RELEVANT BRITISH STANDARDS AND APPROPRIATE CODES OF PRACTICE.
- FOR FURTHER NOTES REFER TO PROJECT SPECIFICATIONS.

SAFETY, HEALTH & ENVIRONMENTAL INFORMATION

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Risks listed here are not exhaustive. Refer to Risk Assessment Register: 23086

Construction:

ADD PROJECT NOTES HERE

Demolition
No Unusual hazards/risks
For information relating to Use, Cleaning and Maintenance see Health & Safety File

It is assumed that all works will be carried out by a competent contractor working, where appropriate, to an approved method statement

INDICATES A RESIDUAL RISK REQUIRING A COMPULSORY ACTION

INDICATES A RESIDUAL RISK FOR INFORMATION

INDICATES A RESIDUAL RISK REQUIRING PROHIBITIVE ACTION

INDICATES A RESIDUAL RISK AS A WARNING

P9	UPDATED TO RECEIVED COMMENTS.	JBD	CS	AR	09.06.2026
P8	SITE LAYOUT PLAN UPDATED	SG	MG	MG	27.10.2025
P7	DRAINAGE LAYOUT UPDATED IN LINE WITH CLIENT COMMENTS	SG	MG	MG	21.10.2025
P6	DRAINAGE LAYOUT UPDATED TO SUIT REVISED SITE PLAN AND CALCS	SG	MG	MG	09.10.2025
P5	ALTERATIONS TO SUIT LLFA COMMENTS SOAKAWAY TANK 4 DIMENSIONS ALTERED. GRADIENTS OF PIPES 1.000 TO 1.003 AND 2.000 ALTERED.	JE	JE	MG	24.02.2025
P4	ALTERATIONS TO SUIT LLFA COMMENTS. PIPE REDUCED TO 600mm. CATCHPIT NOTES ADDED. RC SLAB TO TANKS ADDED. UPDATED TO SUIT DRAINAGE CALCS.	JE	JE	MG	21.11.2024
P3	EXISTING ACCESS ROAD DRAINAGE ADDED. DRAINAGE LAYOUT UPDATED TO SUIT REVISED SITE PLAN AND CALCS.	JE	JE	AR	05.07.2024
P2	ALTERATIONS TO SUIT LLFA COMMENTS	PJH	JE	AR	25.01.2024
C1	UPDATED TO DESIGN	PJH	JE	AR	20.12.2023
P1	INITIAL ISSUE				
REV	DETAIL	BY	CHK	APP	DATE

CONSULTING STRUCTURAL AND CIVIL ENGINEERS

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CLIENT: NATTA

PROJECT: DAPPERS LANE ANGMERING

TITLE: DRAINAGE GENERAL ARRANGEMENT

DRAWING STATUS: PRELIMINARY

DRAWN:	DATE:	CHECKED:	DATE:	APPROVED:	DATE:
PJH	20.12.2023	JE	20.12.2023	AR	20.12.2023

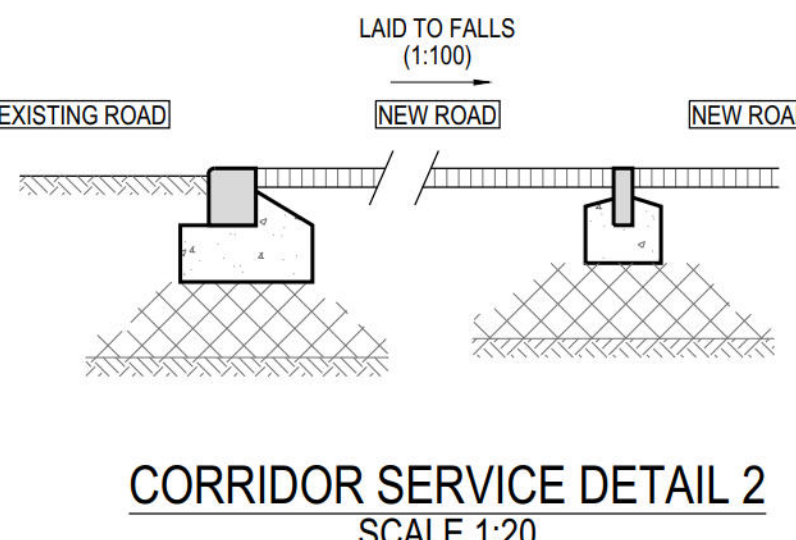
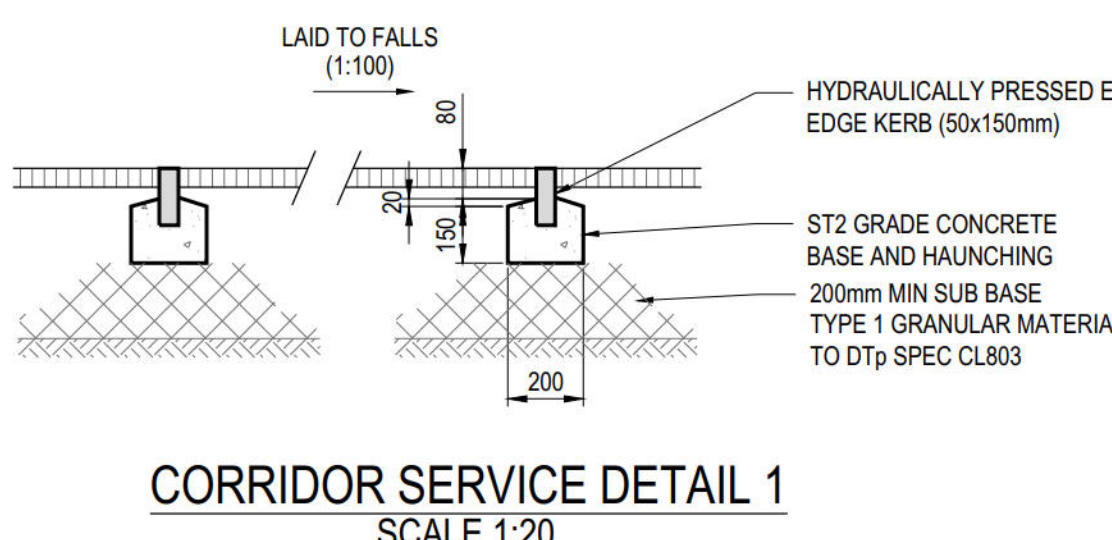
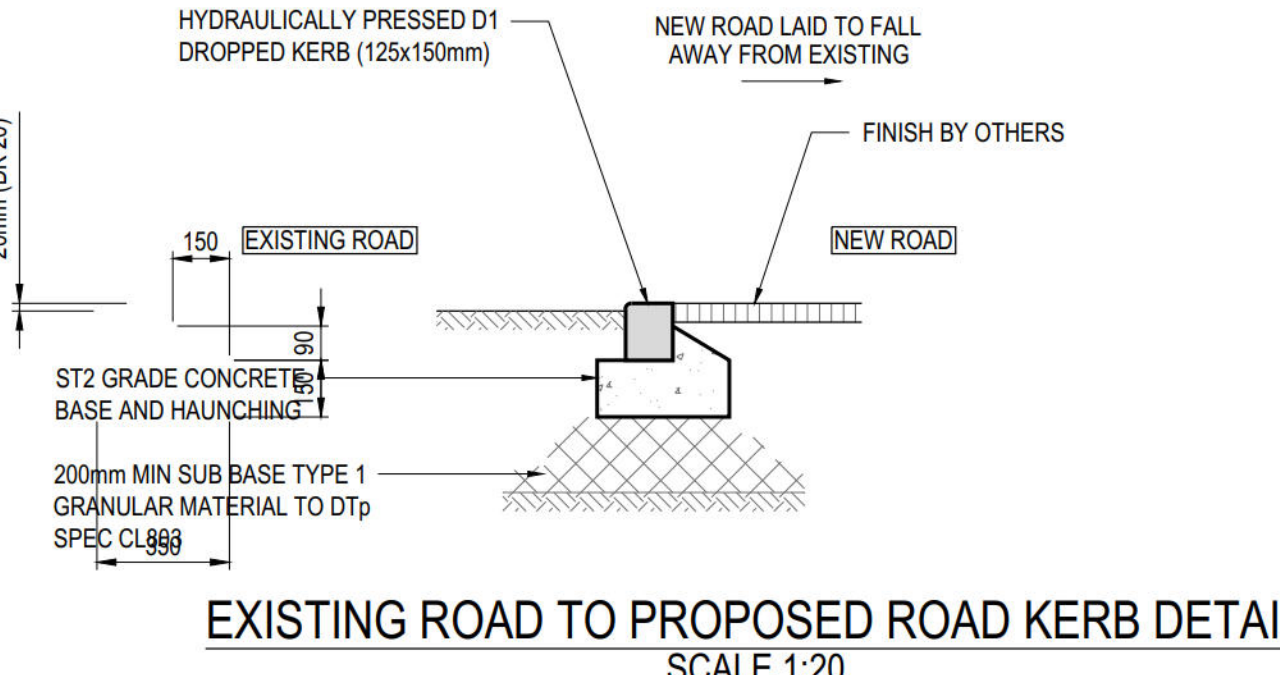
CONTRACT No: 23086

SCALE @ A1: 1:250

PROJECT No: ORIGINATOR: ZONE: LEVEL: TYPE: DISCIPLINE: NUMBER: REVISION:

23086 - ARC - XX - XX - DR - D - 5001 - P9

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INFILTRATION TEST RESULTS 1:

ST CONSULT ENVIRONMENTAL AND GEOTECHNICAL

DATE: 24th MARCH 2021

TP201 - 1.38×10^{-4} m/sec (0.497m/hr)

TP202 - 2.87×10^{-4} m/sec (1.033m/hr)

TP203 - 7.9×10^{-5} m/sec (0.284m/hr)

TP204 - 2.37×10^{-4} m/sec (0.853m/hr)

NOTE: NO GROUND WATER ENCOUNTERED DURING TESTING.

CALCULATION INFILTRATION RATE:

CIRIA SUDS GUIDANCE RECOMMENDS USING THE LOWEST INFILTRATION VALUE RECORDED ON SITE. THEREFORE, THE INFILTRATION RATE FROM TP203 AT 7.9×10^{-5} m/sec (0.284m/hr) HAS BEEN USED WITHIN THE DRAINAGE CALCULATIONS.

INFILTRATION TEST RESULTS 2:

NATTA CONSTRUCTION

DATE: 21ST NOVEMBER 2023

SA T1 - LOCATION OBSTRUCTED BY SITE ACTIVITY

SA T2 - 1.6×10^{-4} m/sec (0.576m/hr)

SA T3 - 1.13×10^{-4} m/sec (0.408m/hr)

SA T4 - 1.11×10^{-4} m/sec (0.3996m/hr)

SA T5 - WATER ENCOUNTERED AT +11.800

NOTE: GROUND WATER ENCOUNTERED IN TRIAL PIT SATS AT A DEPTH OF 11.800m. CIRIA SUDS GUIDANCE RECOMMENDS A FREE BOARD OF 1.0m ABOVE GROUND WATER LEVEL FOR SUDS FEATURES. A MINIMUM DEPTH OF DRAINAGE OF 12.800m HAS BEEN IMPLEMENTED.

DO NOT SCALE

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Risks listed here are not exhaustive. Refer to Risk Assessment Register: 23086

Construction:



ADD PROJECT NOTES HERE

Demolition

No Unusual hazards/risks

For information relating to Use, Cleaning and Maintenance see Health & Safety File

It is assumed that all works will be carried out by a competent contractor working, where appropriate, to an approved method statement



INDICATES A RESIDUAL RISK REQUIRING A COMPULSORY ACTION



INDICATES A RESIDUAL RISK FOR INFORMATION



INDICATES A RESIDUAL RISK REQUIRING PROHIBITIVE ACTION



INDICATES A RESIDUAL RISK AS A WARNING

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REV	DETAIL	BY	CHK	APP	DATE
P4	UPDATED TO RECEIVED COMMENTS. DRAWING TITLE AMENDED	CS	AR	AR	09.06.26
P3	SITE LAYOUT PLAN UPDATED	SG	MG	MG	27.10.25
P2	IMPERMEABLE AREA REVISED IN LINE WITH UPDATED DRAINAGE LAYOUT PLAN	SG	MG	MG	21.10.25
P1	INITIAL ISSUE	SG	MG	MG	09.10.25

REV	DETAIL	BY	CHK	APP	DATE
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Phone: 0113 253 3904
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CLIENT:

NATTA

PROJECT:

DAPPERS LANE
ANGMERING

TITLE:

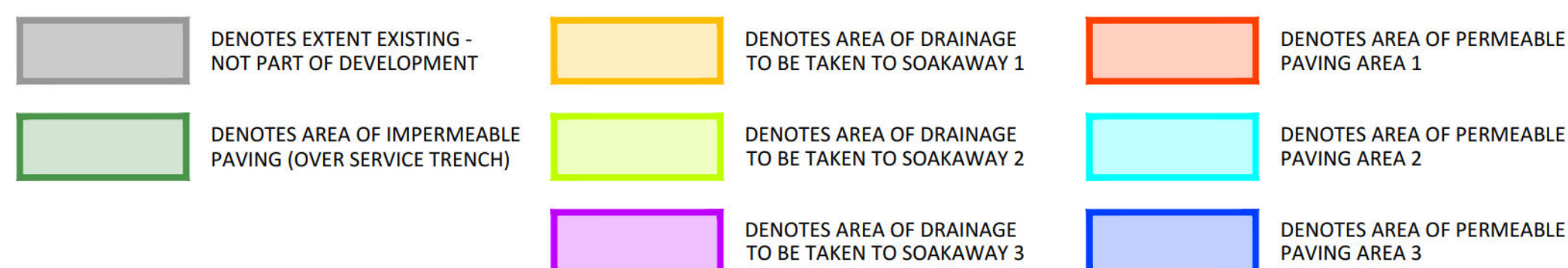
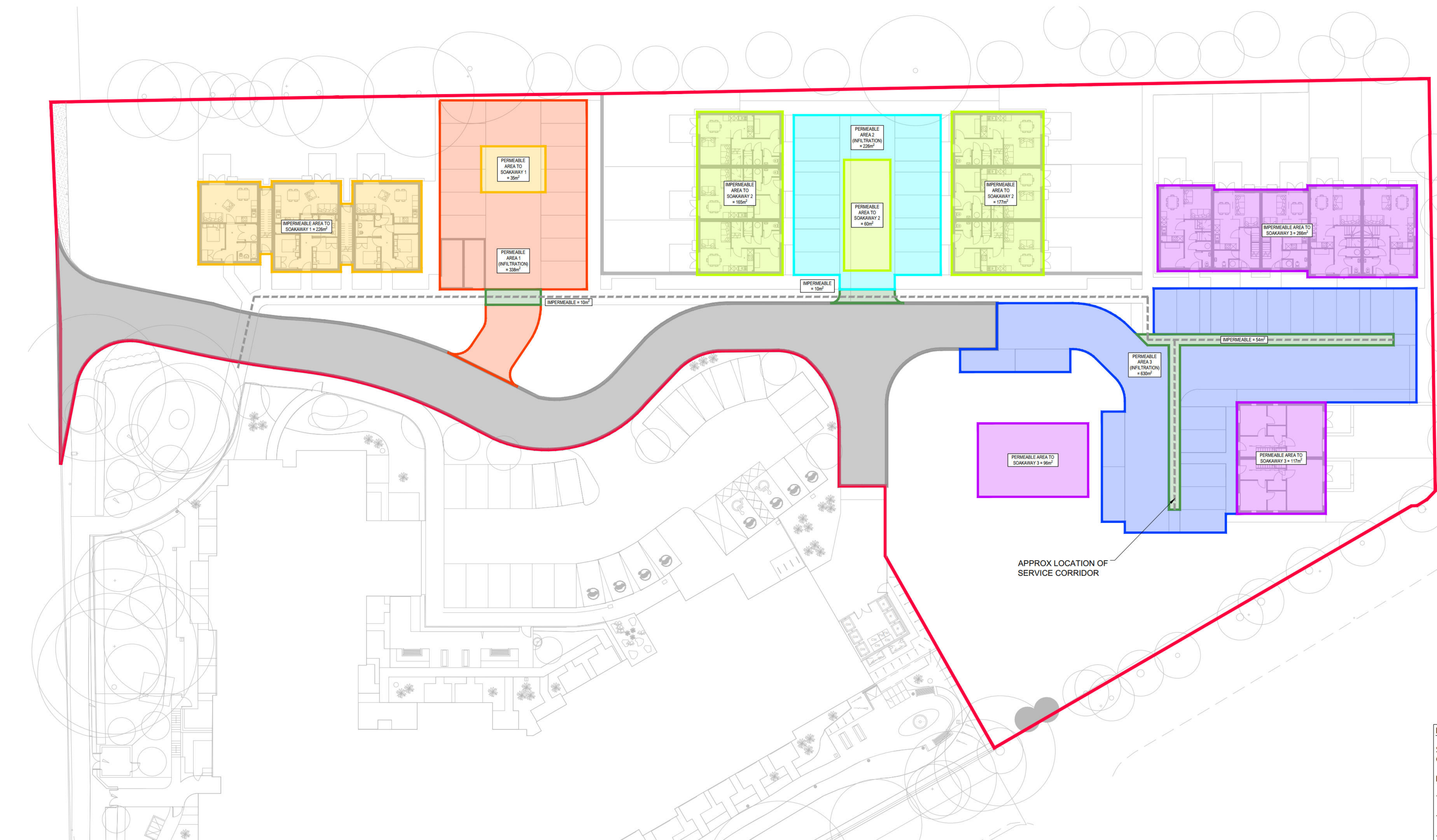
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DRAWING STATUS:

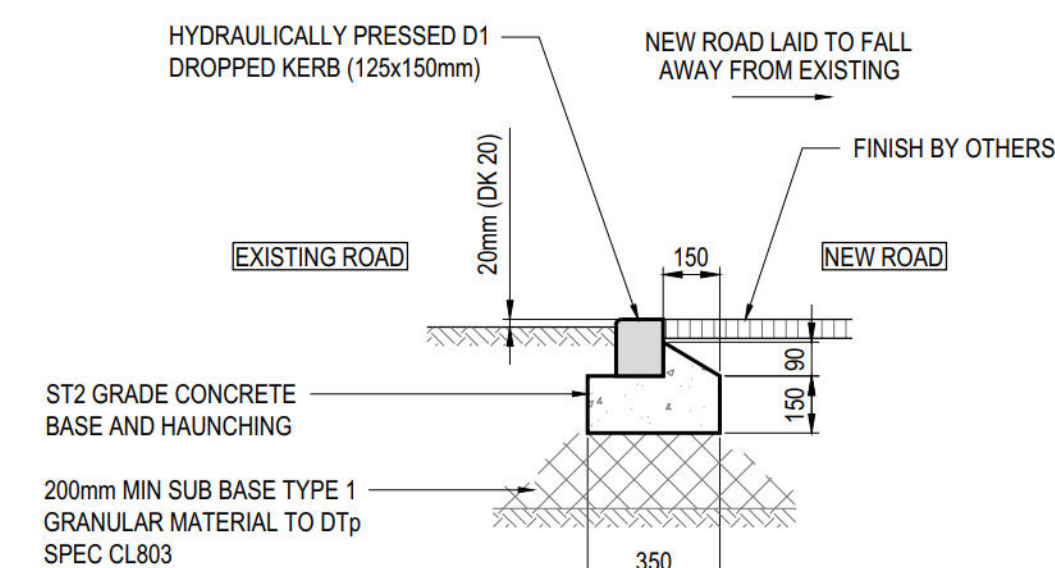
PRELIMINARY

DRAWN	DATE	CHECKED	DATE	APPROVED	DATE
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CONTRACT NO:	23086				
PROJECT NO.	ORIGINATOR	ZONE	LEVEL	TYPE	DISCIPLINE NUMBER
23086	ARC	XX	XX	DR	D - 5010 - P4

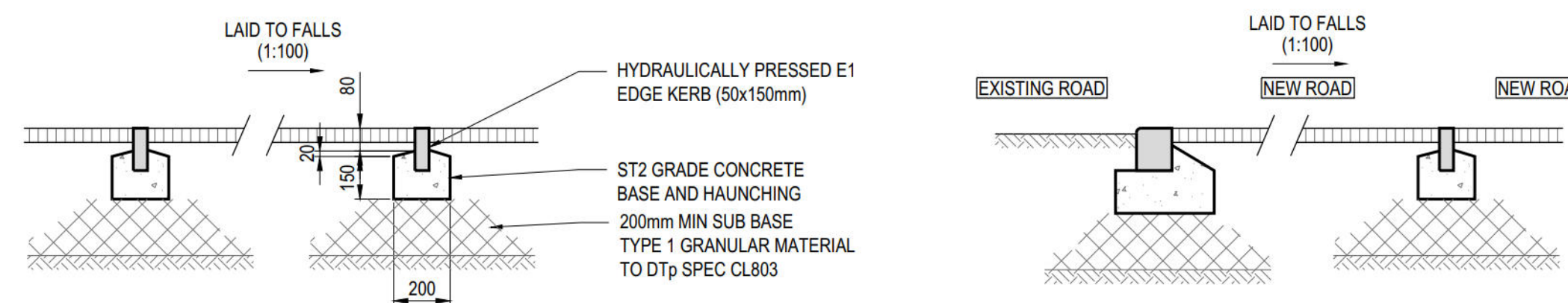
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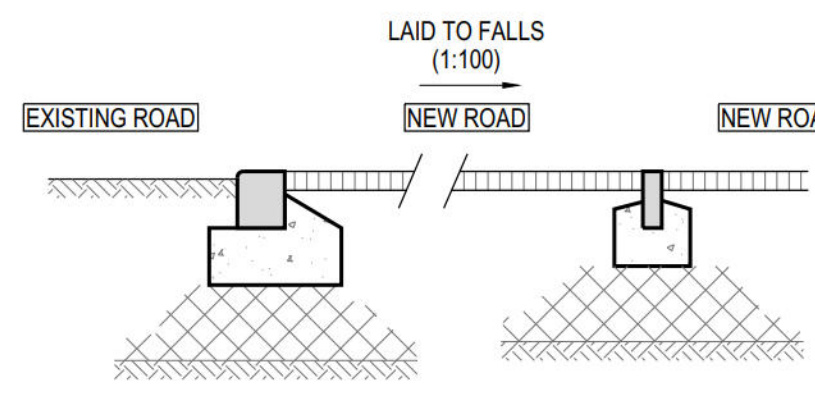
- AREAS OF PERMEABLE PAVING TO BE INFILTRATION METHOD AND SOAKS AWAY INTO GROUND
- ACCES PATHS AROUND BUILDINGS AND GARDEN PATIONS LAID TO FALLS TO ENSURE SURFACE DRAINS INTO SURROUNDING LANDSCAPING
- ENSURE NON-PERMEABLE SURFACE AT SURFACE CORRIDOR LOCATIONS



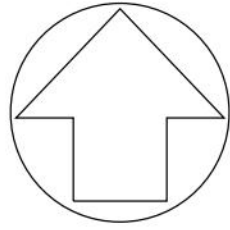
EXISTING ROAD TO PROPOSED ROAD KERB DETAIL
SCALE 1:20



CORRIDOR SERVICE DETAIL 1
SCALE 1:20



CORRIDOR SERVICE DETAIL 2
SCALE 1:20



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Risks listed here are not exhaustive. Refer to Risk Assessment Register: 23086

Construction:



ADD PROJECT NOTES HERE

Demolition

No Unusual hazards/risks

For information relating to Use, Cleaning and Maintenance see Health & Safety File

It is assumed that all works will be carried out by a competent contractor working, where appropriate, to an approved method statement



INDICATES A RESIDUAL RISK REQUIRING A COMPULSORY ACTION



INDICATES A RESIDUAL RISK FOR INFORMATION



INDICATES A RESIDUAL RISK REQUIRING PROHIBITIVE ACTION



INDICATES A RESIDUAL RISK AS A WARNING

P5	LEVELS ADDED	CS	CS	AR	10.06.26
P4	SITE LAYOUT PLAN UPDATED	SG	MG	MG	27.10.25
P3	UPDATED TO SUIT REVISED ARCHITECTS SITE PLAN	SG	MG	MG	09.10.25
P2	UPDATED TO SUIT LLFA COMMENTS	JE	JE	MG	21.11.24
P1	INITIAL ISSUE	JE	JE	AR	04.07.24
REV	DETAIL	BY	CHK	APP	DATE

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CLIENT:

NATTA

PROJECT:

DAPPERS LANE
ANGMERING

TITLE:

FLOOD EXCEEDANCE ROUTES

DRAWING STATUS:

PRELIMINARY

DRAWN:	DATE:	CHECKED:	DATE:	APPROVED:	DATE:
JE	JULY 24	JE	JULY 24	AR	JULY 24
CONTRACT NO:	SCALE @ A1:				
23086	1:250				

PROJECT NO.	ORIGINATOR	ZONE	LEVEL	TYPE	DISCIPLINE	NUMBER	REVISION
23086	ARC	XX	XX	DR	C	5011	P5

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APPENDIX C



SUDS MANAGEMENT AND MAINTENANCE STRATEGY

PROJECT	DAPPERS LANE, ANGMERING
PROJECT NUMBER	23 086
REFERENCE	23 086 ARC-XX-XX-RP-S-9502
CLIENT	NATTA BUILDING COMPANY
DATE WRITTEN	JUNE 2026

REVISION	DATE	WRITTEN BY	REVIEWED BY
P0	9 TH JUNE 2026	ARC	AR

Contents

1.	INTRODUCTION	3
2.	SUDS PROVISION WITHIN THE DEVELOPMENT	3
3.	MAINTENANCE RESPONSIBILITIES	3
4.	MAINTENANCE REGIME	4
5.	INSPECTION LOGS	9

1. INTRODUCTION

This management strategy has been prepared by ARC Engineers on behalf of Natta Building Company in respect of the proposed development at Dappers Lane, Angmering to detail the operational requirements and functionality of the development whilst it is in use.

2. SUDS PROVISION WITHIN THE DEVELOPMENT

The SUDS incorporated into this development are employed to:

- Reduce and manage surface water runoff.
- Improve the quality of water being discharged from the site.

The above provided for a design return period of one hundred years plus climate change, such that the development will not increase the risk of flooding to the site and its environs.

SUDS methods incorporated are:

- Permeable paving
- Geocellular system for soakaway purposes.

This document outlines the long-term maintenance of the proposed surface water system and will make reference to the following documents, some of which provide further detail on the maintenance operations required:

- CIRIA Report C753 'The SUDS Manual', 2015
- CIRIA Report C625 'Model Agreements for Sustainable Water Management Systems', 2004
- Interpave, 'Permeable pavements: Guide to the Design, Construction and Maintenance of Concrete Block Permeable Pavements', ed. 4, 2006

3. MAINTENANCE RESPONSIBILITIES

The maintenance of the unadopted highway, parking and drainage will be the responsibility of the management company, which is expected to be a social housing provider.

4. MAINTENANCE REGIME

The inspection of the SUDS features will be carried out by the site-based maintenance operative.

This will include inspecting:

- All external gullies draining hard landscaping areas.
- Inspecting external Rainwater Pipe connections.
- Inspecting hard landscaping areas with permeable paving to ensure there is no vegetation growth or trapped debris in the gaps between pavements.
- Lifting the PPIC inspection covers to all silt traps in the car parking and access road area to determine when to instruct specialist contractor to clear them.
- Inspection of the swale outfall to ensure there is no debris blocking the head wall.

The following section describes the required maintenance for each feature in turn. The SUDS Maintenance requirements listed below should be reviewed after the first 5 years, with a view to agreeing a new regime for the ongoing maintenance.

Notwithstanding the routine inspections and maintenance requirements, after severe storm events all features shall be inspected to clear debris and repair damaged structures or features. Records of the maintenance carried out shall be prepared by the maintenance manager / team.

GEOCELLULAR SYSTEM (SOAKAWAY)		
Maintenance Schedule	Required Action	Typical Frequency
Regular Maintenance	Inspect and identify any areas that are not operating correctly. If required, take remedial action	Monthly for 3 months then annually
	Remove debris from the catchment surface (where it may cause risks to performance)	Monthly
	For systems where rainfall infiltrates into the tank from above, check surface of filter for blockage by sediment, algae or other matter; remove and replace surface infiltration medium as necessary	Annually
	Remove sediment from pre-treatment structures and/or internal forebays	Annually, or as required
Remedial Actions	Repair/rehabilitate inlets, outlets, overflows and vents	As required
Monitoring	Inspect/check all inlets, outlets, vents and overflows to ensure that they are in good condition and operating as designed	Annually
	Survey inside of tank for sediment build up and remove if necessary	Every 5 years or as required

PERMEABLE PAVEMENTS		
Maintenance Schedule	Required Action	Typical Frequency
Regular Maintenance	Brushing and vacuuming (standard cosmetic sweep over the whole surface)	Once a year, after autumn leaf fall, or reduced frequency as required, based on site-specific observations of clogging or manufacturer's recommendations – pay particular attention to areas where water runs onto pervious surface from adjacent impermeable areas as this area is most likely to collect the most sediment
Occasional Maintenance	Stabilise and mow contributing and adjacent areas	As required
	Removal of weeds or management using glyphosate applied directly into the weeds by an applicator rather than spraying	As required- once per year on less frequently used pavements
Remedial Actions	Remediate any landscaping which, through vegetation maintenance or soil slip, has been raised to within 50mm of the level of the paving	As required
	Remedial work to any depressions, rutting and cracked or broken blocks considered detrimental to the structural performance or a hazard to users, and replace lost jointing material	As required
	Rehabilitation of surface and upper substructure by remedial sweeping	Every 10 to 15 years or as required (if infiltration performance is reduced due to significant clogging)

Monitoring	Initial inspection	Monthly for three months after inspection
	Inspect for evidence of poor operation and/or weed growth – if required take remedial action	Three monthly 48h after large storms in first six months
	Inspect slit accumulation rates and establish appropriate brushing frequencies	Annually
	Monitor inspection chambers	Annually

MANHOLES AND PIPES		
Maintenance Schedule	Required Action	Typical Frequency
Regular Maintenance	Remove and surface litter and detritus.	Monthly
	Inspect manhole covers for damage.	Annually
	Clean sumps and remove silt.	Annually
Occasional Maintenance	Carry out CCTV of outgoing pipework to identify problems.	Ten yearly
Remedial Actions	Replace damaged manhole covers.	As required
	Repair damaged manholes.	As required and as identified by CCTV
	Remove any root ingress or damage to pipework.	As required and as identified by CCTV

GULLIES		
Maintenance Schedule	Required Action	Typical Frequency
Regular Maintenance	Visually inspect gullies for surface damage.	Six monthly
	Remove and surface litter and detritus.	Monthly
	Inspect external covers for damage.	Annually
	Clean gullies and remove silt.	Annually
Occasional Maintenance	Carry out CCTV of outgoing pipework to identify problems.	Ten yearly
Remedial Actions	Replace damaged covers and gratings.	As required
	Replace or repair damaged outgoing pipework.	As required and as identified by CCTV
	Remove any root ingress or damage to outgoing pipework	

5. INSPECTION LOGS

SUDS / EXTERNAL SURFACE WATER DRAINAGE MAINTENANCE INSPECTION CHECKLIST				
SITE NAME AND ADDRESS				
Date of inspection		Details	Action Required (y/n)	Date Completed
Undertaken by				
Page 1 of 3				
Is there any evidence of erosion, channelling, ponding (where not desirable) or other poor hydraulic performance to car parking / access road?				
Is there any evidence of accidental spillages, oils, poor water quality, odours or nuisance insects?				
Have health and safety risks been identified to either the public or maintenance operatives?				
Silt/Sediment Accumulation				
Is there any sediment accumulation at inlets (or other defined accumulation zones such as the surface of filter drains, swales or infiltration basins and within proprietary devices)? If yes, state depth (mm) and extent. Is removal required? If yes, state waste disposal requirements and confirm that all waste management requirements have been complied with (consult environmental regulator)				

SUDS / EXTERNAL SURFACE WATER DRAINAGE MAINTENANCE INSPECTION CHECKLIST				
SITE NAME AND ADDRESS				
Date of inspection		Details	Action Required (y/n)	Date Completed
Undertaken by				
Page 2 of 3				
Is surface clogging visible potentially problematic where water has to soak into the underlying construction or ground (e.g. under drained swale or infiltration basin)?				
Does permeable or porous surfacing require sweeping to remove silt?				
Is there evidence of litter accumulation in the system? If yes, is blockage a risk?				
Is there any evidence of any other clogging or blockage of outlets or drainage paths?				
Is the vegetation condition satisfactory (density, weed growth, coverage, etc.)? (check against approved planning regime)				
Does any part of the system require weeding, pruning or mowing? (check against maintenance frequency state in approved design)				
Is there any evidence of invasive species becoming established? If yes, state action required				
Are any check dams or weirs in good condition?				

SUDS / EXTERNAL SURFACE WATER DRAINAGE MAINTENANCE INSPECTION CHECKLIST				
SITE NAME AND ADDRESS				
Date of inspection		Details	Action Required (y/n)	Date Completed
Undertaken by				
Page 3 of 3				
Is there any evidence of any accidental damage to the system (eg wheel ruts)?				
Is there any evidence of cross connections or other unauthorised inflows?				
Is there any evidence of tampering with the flow control?				
Are there any other matters that could affect the performance of the system in relation to the design objectives for hydraulic, water quality, biodiversity and visual aspects?				
OTHER OBSERVATIONS				
Information appended (eg photos)				
Continue as current				
Increase maintenance				
Decrease maintenance				
Proposed date for next inspection				