



Consulting Civil Engineers

SuDS Maintenance and Management Plan

Land west of Arundel Road, Angmering

For

GD Group

Rev – P2

Reference C1276

Date 15th June 2020

Contents

1. Introduction.....	3
2. Sustainable Drainage Systems (SuDS)	3
3. SuDS at Arundel Road.....	4
4. Managing SuDS.....	5
5. Water Quality	6
6. SuDS Drainage Maintenance Specification	10
6.1. Permeable Paving.....	10
6.2. Attenuation Tank.....	11
7. General Drainage Maintenance Specification	12
7.1. Inlet Structures and Inspection Chambers	12
7.2. Below ground drainage pipes.....	13
7.3. Hydro-Brake Flow Control Chamber	14
7.4. TurtleEnviro Storm Shark Vortex Storm water separator	14

1. Introduction

CGS Civils Ltd have been appointed to undertake a drainage design using sustainable techniques for a proposed development in Angmering, West Sussex.

The site will consist of the construction of 9 new dwellings. There will be individual driveways as well as visitor parking spaces located along the access road. Root protection zones border the eastern edge of the site. The site falls gradually to the south-west.

2. Sustainable Drainage Systems (SuDS)

The surface water management strategy has been developed using best practice Sustainable Drainage Systems (SuDS) techniques. Guidance on suitable techniques and methods has been obtained from the EA, and the SuDS manual (CIRIA C753).

The National Planning Policy Framework (NPPF) recognises that flood risk and other environmental damage can be managed by minimising changes in the volume and rate of surface runoff from development sites and recommends that priority is given to the use of Sustainable Drainage Systems (SuDS) in new development.

The overall philosophy of SuDS is to replicate, as closely as possible, the natural drainage process of a site prior to development to mitigate the adverse effect of storm water runoff on the environment. SuDS not only provide the ability to control surface water flows, but also improve water quality, amenity and bio-diversity within the development.

The Building Regulations Requirement H3 stipulates that rainwater from roofs and paved areas is carried away from the surface to discharge to one of the following, listed in order of priority:

- 1- Discharge into the ground
- 2- Discharge to a surface water body
- 3- Discharge to a surface water sewer

3. SuDS at Arundel Road

Geotechnical reports carried out on site confirm the sub-strata to be comprised of made ground followed by London Clay Formation which comprises of blue grey silty clay and finally Lambeth Group.

Soakage tests to the BRE 365 method were undertaken on site on August 2018. Testing undertaken proved the ground to be effectively impermeable.

Exploratory Hole	Groundwater Depth (m)		Depth of Borehole at time of test (m)	Stratum	Response Rate (m/sec)
	Start	Finish			
BH01 (within standpipe)	10.60	2.50	12.00	Lambeth Group	9.8×10^{-7}
BH02 (within standpipe)	4.70	4.47	11.50	Lambeth Group	3.6×10^{-6}
BH03 (within standpipe)	3.70	2.39	10.00	London Clay Formation / Lambeth group	9.6×10^{-6}

On 20th January 2020 another set of soakage tests to BRE 365 method were undertaken on site. This series of tests confirmed the ground to be effectively impermeable.

WS	Soakage Dimensions (m)	Area	Depth (m)	Soil Description (Base of TP)	Infiltration Rate (m/sec)	Drainage Characteristics
TP01 test1	3.00 x 0.60		1.50	Light brown gravelly sandy CLAY. Sand is fine. Gravel is very occasional, angular of flint.	N/A	Practically Impermeable
TP02 test1	2.50 x 0.80		0.80	Light brown sandy CLAY. Sand is fine.	N/A	Practically Impermeable
TP03 test1	2.55 x 0.66		1.60	Light brown CLAY.	N/A	Practically Impermeable
TP04 test1	3.00 x 0.60		0.90	Light brown CLAY.	N/A	Practically Impermeable
TP05 test1	2.50 x 0.60		1.55	Light brown silty CLAY.	N/A	Practically Impermeable

Based on the above results and due to a lack of any watercourse or surface water sewer, the site will discharge all surface water into a nearby highway drain.

Greenfield runoff calculations have confirmed a QBar rate of 0.7l/s. We therefore propose restricted flows to a minimum of 1.0l/s which is the generally accepted minimum rate to ensure a functioning system.

The surface water discharge will be restricted by a Hydrobrake vortex flow control device with the outfall connected into the WSCC highway drain which has been agreed with WSCC.

The main SuDS feature itself will be in the form of a large cellular storage attenuation tank to the east of the site. A silt trap will be required upstream of the incoming connection to capture any silt and debris before entering the tank. Downstream of the tank will be a more advanced separator to filter out any remaining debris, suspended particles, and hydrocarbons prior to entering the highway drainage system.

All driveways are to be constructed with permeable block paved surface that will be tanked (fully sealed) with fin drains added to permit the water to enter into the main surface water drainage system. These areas will provide primary treatment of any pollutants whilst also providing a modest amount of storage.

To provide a degree of redundancy, it will also be necessary for a channel drain to be installed at the end of each driveway as all driveways fall towards the buildings.

All SuDS features have been designed to cater for and contain the 1 in 100-year storm + 40%.

4. Managing SuDS

The SuDS have been designed for easy maintenance and comprise:

- Regular day-to-day care – litter collection, regular gardening to control vegetation growth and checking inlets where water enters the SuDS feature
- Occasional tasks – checking the SuDS feature and removing any silt that builds up in the SuDS feature
- Remedial work – repairing damage where necessary

The client will, within the first 6-12 months of construction, form a maintenance company within the structure of the current company and funded by the client to facilitate the maintenance of the drainage system and common areas as per the requirements and specifications within this document. Once the dwellings become occupied, the occupants will pay a fixed annual fee towards this to cover the costs of the maintenance as detailed below.

Should the maintenance company fail to carry out its duties then the maintenance company will be obliged to refund any costs to the occupants and a new company is to be appointed by a representative of the occupants to carry out the maintenance duties. This will be written into the contract.

Once design life has been met on the SuDS features then these will be repaired or replaced in part in accordance with the SuDS maintenance specification in Section 6 or fully replaced in accordance with the respective manufacturer's specifications.

5. Water Quality

The new SuDS Manual (Ciria C753, November 2015) introduces slightly different approach compared to the previous version for the water quality management of surface water. The Manual describes risks posed by the surface water runoff to the receiving environment as a function of:

- The pollution hazard at a particular site (i.e. the pollution source)
- The effectiveness of SuDS treatment components in reducing levels of pollutants to environmentally acceptable levels
- The sensitivity of the receiving environment

The EA website indicates that the site does not lie within a Source Protection Zone.

The recommended approaches for water quality risk management are given in the SuDS Manual Table 26.1.

Figure 7 Approaches to Water Quality Risk Management

Design method	Hazard characterisation	Risk reduction	
		For surface water	For groundwater
Simple index approach	Simple pollution hazard indices based on land use (eg Table 26.2 or equivalent)	Simple SuDS hazard mitigation indices (eg Table 26.3 or equivalent)	Simple SuDS hazard mitigation indices (eg Table 26.4 or equivalent)
Risk screening ¹	Factors characterising traffic density and extent of infiltration likely to occur (eg Table 26.5 or equivalent)	N/A	Factors characterising unsaturated soil depth and type, and predominant flow type through the soils (eg Table 26.5 or equivalent)
Detailed risk assessment	Site specific information used to define likely pollutants and their significance	More detailed, component specific performance information used to demonstrate that the proposed SuDS components reduce the hazard to acceptable levels	
Process-based treatment modelling	Time series rainfall used with generic pollution characteristics to determine statistical distributions of likely concentrations and loadings in the runoff	Models that represent the treatment processes in the proposed SuDS components give estimates of reductions in event mean discharge concentrations and total annual load reductions delivered by the system	

As per Table 26.1 Simple Index approach will be used as a design method for this site.

Table 26.2 will provide hazard classification of different land uses. The land uses for the surface water drainage for this site are;

- Residential Roofs
- Individual Property driveways
- Low traffic roads

To deliver adequate treatment, the selected SuDS components should have a total pollution mitigation index for each contaminant type that equals or exceeds the pollution hazard index for each contaminant type. Therefore, the following must be achieved for the surface running off the site.

Total SuDS mitigation index $\geq$ pollution hazard index

Pollution Hazard Indices are given for different land uses in Table 26.2 of the SuDS manual;

Figure 8 Pollution Hazard Indices for Different Land Use Classifications

TABLE 26.2 Pollution hazard indices for different land use classifications				
Land use	Pollution hazard level	Total suspended solids (TSS)	Metals	Hydro-carbons
Residential roofs	Very low	0.2	0.2	0.05
Other roofs (typically commercial/ industrial roofs)	Low	0.3	0.2 (up to 0.8 where there is potential for metals to leach from the roof)	0.05
Individual property driveways, residential car parks, low traffic roads (eg cul de sacs, homezones and general access roads) and non-residential car parking with infrequent change (eg schools, offices) ie < 300 traffic movements/day	Low	0.5	0.4	0.4
Commercial yard and delivery areas, non-residential car parking with frequent change (eg hospitals, retail), all roads except low traffic roads and trunk roads/motorways ¹	Medium	0.7	0.6	0.7
Sites with heavy pollution (eg haulage yards, lorry parks, highly frequented lorry approaches to industrial estates, waste sites), sites where chemicals and fuels (other than domestic fuel oil) are to be delivered, handled, stored, used or manufactured; industrial sites; trunk roads and motorways ¹	High	0.8 ²	0.8 ³	0.9 ²

From Table 26.2 the following information is tabulated in Table 4

Table 4: Pollution hazard index and destination of runoff for the proposed site

Land Use	Destination of Runoff	Pollution Hazard Level	Total Suspended Solids	Metals	Hydrocarbons
Residential Roofs	Surface Waters	Very Low	0.2	0.2	0.05
Individual Driveways and low traffic roads	Surface Waters	Low	0.5	0.4	0.4

The SuDS mitigation index will be obtained from Table 26.4 (for surface water) of the SuDS manual.

Figure 9 Indicative SuDS Mitigation Indices for discharges to surface waters

TABLE 26.3 Indicative SuDS mitigation indices for discharges to surface waters				
		Mitigation indices ¹		
Type of SuDS component	TSS	Metals	Hydrocarbons	
Filter strip	0.4	0.4	0.5	
Filter drain	0.4 ²	0.4	0.4	
Swale	0.5	0.6	0.6	
Bioretention system	0.8	0.8	0.8	
Permeable pavement	0.7	0.6	0.7	
Detention basin	0.5	0.5	0.6	
Pond ⁴	0.7 ³	0.7	0.5	
Wetland	0.8 ³	0.8	0.8	
Proprietary treatment systems ^{5,6}	These must demonstrate that they can address each of the contaminant types to acceptable levels for frequent events up to approximately the 1 in 1 year return period event, for inflow concentrations relevant to the contributing drainage area.			

SuDS mitigation index are tabulated in Table 5 as followed;

Runoff Source	Destination of Runoff	Mitigation Index Source	Mitigation Indices			
			Type of SuDS Component	Total Suspended Solids (TSS)	Metals	Hydrocarbons
Individual Driveways	Surface Waters	Table 26.3 (for surface waters)	Permeable Pavement	0.7	0.6	0.7

Table 5: SuDS mitigation index

The above analysis demonstrates that the SuDS devices within the design will mitigate any pollution present within the surface water system.

6. SuDS Drainage Maintenance Specification

6.1. Permeable Paving

In order to maintain the functioning of the permeable paving, the following maintenance requirements should be adhered to:

TABLE 20.15 Operation and maintenance requirements for pervious pavements

Maintenance schedule	Required action	Typical frequency
Regular maintenance	Brushing and vacuuming (standard cosmetic sweep over whole surface)	Once a year, after autumn leaf fall, or reduced frequency as required, based on site-specific observations of clogging or manufacturer's recommendations – pay particular attention to areas where water runs onto pervious surface from adjacent impermeable areas as this area is most likely to collect the most sediment
Occasional maintenance	Stabilise and mow contributing and adjacent areas	As required
	Removal of weeds or management using glyphosate applied directly into the weeds by an applicator rather than spraying	As required – once per year on less frequently used pavements
Remedial Actions	Remediate any landscaping which, through vegetation maintenance or soil slip, has been raised to within 50 mm of the level of the paving	As required
	Remedial work to any depressions, rutting and cracked or broken blocks considered detrimental to the structural performance or a hazard to users, and replace lost jointing material	As required
	Rehabilitation of surface and upper substructure by remedial sweeping	Every 10 to 15 years or as required (if infiltration performance is reduced due to significant clogging)
Monitoring	Initial inspection	Monthly for three months after installation
	Inspect for evidence of poor operation and/or weed growth – if required, take remedial action	Three-monthly, 48 h after large storms in first six months
	Inspect silt accumulation rates and establish appropriate brushing frequencies	Annually
	Monitor inspection chambers	Annually

6.2. Attenuation Tank

In order to maintain the functioning of the attenuation tanks, the following maintenance requirements should be adhered to:

TABLE 21.3 Operation and maintenance requirements for attenuation storage tanks

Maintenance schedule	Required action	Typical frequency
Regular maintenance	Inspect and identify any areas that are not operating correctly. If required, take remedial action	Monthly for 3 months, then annually
	Remove debris from the catchment surface (where it may cause risks to performance)	Monthly
	For systems where rainfall infiltrates into the tank from above, check surface of filter for blockage by sediment, algae or other matter; remove and replace surface infiltration medium as necessary	Annually
	Remove sediment from pre-treatment structures and/or internal forebays	Annually, or as required
Remedial actions	Repair/rehabilitate inlets, outlet, overflows and vents	As required
Monitoring	Inspect/check all inlets, outlets, vents and overflows to ensure that they are in good condition and operating as designed	Annually
	Survey inside of tank for sediment build-up and remove if necessary	Every 5 years or as required

7. General Drainage Maintenance Specification

7.1. Inlet Structures and Inspection Chambers

- Inlet structures such as rain water downpipes, road gullies should be free from obstruction at all times to allow free flow through the SuDS.
- Channel Drains should be free from obstruction at all times to reduce flood risks and to allow free flow through the SuDS
- Inspection Chambers and rodding eyes are used on bends or where pipes come together. They allow access and cleaning to the system if necessary.
- Silt traps are used to capture silt within the drainage system. They are used in place of inspection chambers when located before underground storage, detention basins or even before discharging into an adopted system.

Inlet Structures, Inspection Chambers and Silt Traps	
Regular Maintenance	Frequency
Inlet Structures Inspect rainwater downpipes and road gullies, removing obstructions and silt as necessary. Check that there is no physical damage. Strim vegetation 1m min surround to structures and keep area free from silt and debris	Monthly
Inspections Chambers and below ground control chambers Remove cover and inspect, ensuring that the water is flowing freely and that the exit route for water is unobstructed. Remove debris and silt. Undertake inspection after leaf fall in Autumn	Annually
Channel Drains Inspect channel drains, removing obstructions and silt as necessary. Check that there is no physical damage. Strim vegetation 1m min surround to structures and keep area free from silt and debris	Monthly
Silt Traps	

Remove cover and inspect, ensuring that the water is flowing freely and that the exit route for water is unobstructed. Remove debris and silt from the silt bucket. Undertake inspection after leaf fall in Autumn.	Annually
Occasional Maintenance Check topsoil levels are 20mm above edges of chambers to avoid mower damage.	As necessary
Remedial Work Repair physical damage if necessary	As required

7.2. Below ground drainage pipes

- Below ground drainage pipes convey water to the SuDS system. They should always be free from obstruction to allow free flow.

Below Ground Drainage Pipes	
Regular Maintenance	Frequency
Inspect and identify any areas that are not operating correctly. If required take remedial action.	Monthly for 3 months then annually
Remove debris from the catchment surface (where it may cause risks to performance)	Monthly
Remove sediment from pre-treatment inlet structures and inspection chambers.	Annually or as required
Maintain vegetation to designed limits within the vicinity of below ground drainage pipes and tanks to avoid	Monthly or as required
Remedial Work Repair physical damage if necessary	As required
Monitoring Inspect all inlets, outlets and vents to ensure that they are in good conditions and operating as designed. Survey inside of pipe runs for sediment build up and remove if necessary.	Annually Every 5 years or as required

7.3. Hydro-Brake Flow Control Chamber

- See Appendix C

7.4. Turtle Enviro Storm Shark Vortex Storm water separator

- See appendix D

Appendix A

Drainage Strategy

DESIGN SUBJECT TO THE APPROVAL OF:
PLANNING AUTHORITY
BUILDING CONTROL
SOUTHERN WATER
WEST SUSSEX COUNTY COUNCIL

SITE SPECIFIC DESIGN NOTES:

- ALL PIPE JUNCTIONS TO BE OBLIQUE. SQUARE JUNCTIONS ARE NOT PERMITTED

DRAINAGE LEGEND (1:200)
PROPOSED FEATURES

- Storm water sewer/drain
- Storm water channel drain
- Fin Drain
- Rain water pipe
- Road Gully
- Storm water inspection chamber (3000)
- Storm water inspection chamber (4500)
- Storm water inspection chamber (6000)
- Storm water manhole
- Storm water vortex flow control chamber
- Geocellular Attenuation Tank
- Tanked porous pavements
- Soil stack (type TBC by architect/m&e engineer)
- Foul water access chamber (3000)
- Foul water inspection chamber (4500)
- Foul water manhole
- Finished floor level
- PIPE INFO - DIAMETER, LENGTH, GRADIENT, BEDDING TYPE
- ABBREVIATIONS
 - CP - CATCHPIT
 - MH - MANHOLE
 - IC - INSPECTION CHAMBER
 - AC - ACCESS CHAMBER
 - RE - RODDING EYE
 - IL - INVERT LEVEL
 - SL - SUMP LEVEL
 - CL - COVER LEVEL

STANDARD NOTES

- DO NOT SCALE FROM THIS DRAWING. REFER TO FIGURED DIMENSIONS ONLY. THE CONTRACTOR SHOULD CHECK ALL DIMENSIONS ON SITE.
- ALL DIMENSIONS IN MILLIMETRES AND ALL LEVELS ARE IN METERS UNLESS NOTED OTHERWISE.
- THIS DRAWING SHOULD BE READ IN CONJUNCTION WITH ALL OTHER RELEVANT ARCHITECT AND ENGINEERING DETAILS, DRAWINGS AND SPECIFICATIONS.
- ANY DISCREPANCIES SHOULD BE REPORTED TO THE ARCHITECT AND/OR ENGINEER IMMEDIATELY, SO THAT CLARIFICATION CAN BE SOUGHT PRIOR TO THE COMMENCEMENT OF WORK.
- BEFORE COMMENCING CONSTRUCTION THE CONTRACTOR MUST CHECK THE INVERT LEVELS OF EXISTING SEWERS TO WHICH CONNECTIONS ARE MADE. IN ADDITION THE CONTRACTOR MUST LOCATE AND DETERMINE INVERT LEVELS OF THE EXISTING SPURS TO WHICH CONNECTIONS ARE PROPOSED. ANY DISCREPANCIES ARE TO BE NOTIFIED TO THE ENGINEER IMMEDIATELY, PRIOR TO CONSTRUCTION.
- ALL DRAINAGE WORKS SHOULD COMMENCE AT THE PROPOSED DOWNSTREAM CONNECTION POINT. THE WORKS CONTINUING UPSTREAM FOLLOWING CONFIRMATION OF THE TIE-IN INVERT LEVELS TO THE ENGINEER. CONNECTIONS TO MANHOLES OR LARGER SIZED PIPES ETC. SHOULD BE SOFFIT TO SOFFIT UNLESS OTHERWISE INSTRUCTED BY THE ENGINEER. IF THIS IS NOT POSSIBLE INFORM THE ENGINEER IMMEDIATELY.
- COVER LEVELS SHOWN ARE APPROXIMATE. COVERS AND FRAMES SHALL BE SET TO FINISHED GROUND LEVELS AND FALLS.
- ALL UN-REFERENCED PIPES ARE ASSUMED TO BE 100mm DIA.
- ROAD GULLY OUTLET PIPES ARE TO BE 150mm DIA. WITH CONCRETE SURROUND AND FLEXIBLE JOINTS. ALL GULLIES SHALL BE FITTED WITH GRADE D400 GRATINGS AND FRAMES TO BS EN124, UNLESS OTHERWISE STATED.
- ALL PRIVATE DRAINAGE TO BE IN ACCORDANCE WITH THE BUILDING REGULATIONS APPROVED DOCUMENT PART-H, AND TO THE SATISFACTION OF THE BUILDING CONTROL INSPECTOR.
- THE CONTRACTOR IS TO KEEP A RECORD OF ANY VARIATIONS MADE ON SITE, INCLUDING THE RELOCATION OF SEWERS OR DRAINS. SO THAT AN AS CONSTRUCTED DRAWING CAN BE PREPARED UPON COMPLETION OF THE PROJECT.
- IF ANY SUB SOIL DRAINAGE SYSTEMS ARE UNCOVERED DURING THE WORKS CONTACT THE ENGINEER FOR INSTRUCTIONS. SUB SOIL DRAINS ARE TO BE DIVERTED AROUND NEW WORKS AND CONNECTED INTO THE SURFACE WATER.
- NO PRIVATE AREAS ARE TO DRAIN ONTO ADOPTABLE AREAS AND VICE VERSA.
- ALL EXISTING MANHOLE COVERS, GULLIES, ETC. ARE TO BE RAISED/LOWERED TO SUIT NEW LEVELS.
- MANHOLE AND CHAMBER COVER GRADES:
 - 'A15' IN ALL LANDSCAPED AREAS AND ON FOOTPATHS
 - 'B125' IN ALL DRIVEWAYS
 - 'C250' IN PRIVATE PARKING AREAS
 - 'D400' IN CARRIAGEWAY/ACCESS ROAD
- SURFACE WATER PIPES TO BE STRUCTURED TWIN WALL.
- ALL FOUL WATER PIPES TO BE UPV-C OR CLAY.
- ALL PIPE JUNCTIONS TO BE OBLIQUE. SQUARE JUNCTIONS ARE NOT PERMITTED. REFER TO DETAIL.



00

P6	11.08.20	UPDATED TO SUIT ADC COMMENTS	LH	CS	CS
P5	18.04.20	UPDATED TO SUIT ADC COMMENTS	LH	CS	CS
P4	30.03.20	UPDATED TO SUIT ADC COMMENTS	LH	CS	CS
P3	04.03.20	FULL DRAINAGE LAYOUT	LH	CS	CS
P2	17.02.20	UPDATED TO SUIT NEW SITEPLAN	LH	CS	CS
P1	07.01.20	CONNECTION TO HWD INDICATED	LH	CS	CS
P.	16.01.20	PRELIMINARY ISSUE	LH	CS	CS
REV	DATE	DESCRIPTION	BY	CHK	APP

 Consulting Civil Engineers				
CLIENT GD GROUP				
ARCHITECT CITYZEN				
JOB TITLE ARUNDEL ROAD ANGMERING				
DRAWING TITLE DRAINAGE LAYOUT				
DRAWN LH	ENGINEER CS	CHECKED CS	APPROVED	
DATE NOVEMBER 2019	SCALE @ A1 1:200			
JOB No. C1276	STATUS P	DRAWING No. 100	REV. P6	

PRELIMINARY

COMPULSORY
All block paved porous driveways to have a permeable Inhibex Geotextile between the laying course and sub-base as per detail. Sub-base to be wrapped in an impermeable SC geomembrane as per detail.

AT1
Attenuation Tank using Polypipe
Polytorm units wrapped in
impermeable geomembrane
36x60x6m CP
Total 177.84m³ storage for 1 in 100
year + 40% peak storm event
CL 20.350
TOTT 18.567
INC IL 18.797
BOT/OUT IL 18.797
Min 500mm cover to units
Vent pipe to connect to S25 as per
detail

Existing manhole cover and frame
to be replaced with class D400
Ductile iron of the same size, and
bedded appropriately to the
satisfaction of Arun DC Engineer

SW Runoff
Drained site area = 0.2209ha
Qbar = 0.7ls
Proposed discharge restricted to
rate of 1ls/s

WARNING
All works within Root Protection
Zone to be carried out by hand
and under supervision of
Arboricultural Consultant

Existing manhole cover and frame
to be replaced with class D400
Ductile iron of the same size, and
bedded appropriately to the
satisfaction of Arun DC Engineer

New 1500 connection into WSCC
Highway Drain at Soft level -
Subject to approval from WSCC
and Arun DC

Pipes within RPZ to have
150mm concrete bedding and
surround in order to protect
against root intrusion. Trens
T32 and T33 to be felled

FW Drainage layout downstream
of F11 subject to Southern Water
Design under SWS Agreement

Existing manhole cover and frame
to be replaced with class D400
Ductile iron of the same size, and
bedded appropriately to the
satisfaction of Arun DC Engineer

Appendix B

Surface Water Drainage Calculations

Design Settings

Rainfall Methodology	FSR	Maximum Time of Concentration (mins)	30.00
Return Period (years)	2	Maximum Rainfall (mm/hr)	50.0
Additional Flow (%)	0	Minimum Velocity (m/s)	1.00
FSR Region	England and Wales	Connection Type	Level Soffits
M5-60 (mm)	20.000	Minimum Backdrop Height (m)	0.600
Ratio-R	0.350	Preferred Cover Depth (m)	1.200
CV	0.750	Include Intermediate Ground	✓
Time of Entry (mins)	5.00	Enforce best practice design rules	✓

Nodes

Name	Area (ha)	T of E (mins)	Cover Level (m)	Diameter (mm)	Easting (m)	Northing (m)	Depth (m)
S1	0.000	5.00	22.350	450	506703.234	105430.121	0.600
S2	0.038	5.00	22.400	450	506713.315	105429.313	0.755
S3	0.034	5.00	21.850	450	506715.215	105392.830	1.259
S4	0.049	5.00	21.000	600	506713.332	105364.271	0.878
S5	0.051	5.00	20.450	1200	506717.901	105332.597	1.067
AT1	0.000	5.00	20.350	1	506737.402	105336.897	1.553
S6	0.036	5.00	19.920	1200	506740.994	105319.027	1.249
S7	0.000	5.00	20.300	450	506706.861	105321.009	0.556
S8	0.015	5.00	20.300	450	506717.726	105322.850	0.701
S9	0.000	5.00	19.700	450	506723.985	105311.520	0.707
S10	0.000		19.920	450	506746.620	105314.833	1.600
S23	0.000		21.150	450	506714.294	105378.860	0.751
S24	0.000		20.300	600	506716.186	105344.453	0.735
S25	0.000		20.600	1200	506727.291	105334.667	1.788
S26			20.350	1000	506738.060	105331.582	1.577

Links (Input)

Name	US Node	DS Node	Length (m)	ks (mm) / n	US IL (m)	DS IL (m)	Fall (m)	Slope (1:X)	Dia (mm)	T of C (mins)	Rain (mm/hr)
1.000	S1	S2	10.113	0.600	21.750	21.645	0.105	96.3	100	5.22	50.0
1.001	S2	S3	36.532	0.600	21.645	20.591	1.054	34.7	150	5.57	50.0
1.002	S3	S23	14.000	0.600	20.591	20.399	0.192	72.9	150	5.77	50.0
1.003	S23	S4	14.621	0.600	20.399	20.197	0.202	72.4	150	5.97	50.0
2.000	S4	S24	20.022	0.600	20.122	19.565	0.557	35.9	225	6.13	50.0
2.001	S24	S5	11.979	0.600	19.565	19.383	0.182	65.8	225	6.25	50.0
2.002	S5	S25	9.615	0.600	19.383	18.812	0.571	16.8	225	6.30	50.0
2.003	S25	AT1	10.354	0.600	18.812	18.797	0.015	690.3	225	6.65	50.0
2.004	AT1	S26	5.356	0.600	18.797	18.773	0.024	225.0	225	6.76	50.0
2.005	S26	S6	12.893	0.600	18.773	18.671	0.102	126.4	225	6.94	50.0
3.000	S9	S6	18.592	0.600	18.993	18.796	0.197	94.4	100	5.39	50.0
4.000	S7	S8	11.020	0.600	19.744	19.599	0.145	76.0	100	5.21	50.0
4.001	S8	S5	9.749	0.600	19.599	19.508	0.091	107.1	100	5.43	50.0

Links (Input)

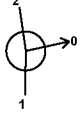
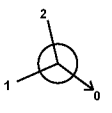
Name	US Node	DS Node	Length (m)	ks (mm) / n	US IL (m)	DS IL (m)	Fall (m)	Slope (1:X)	Dia (mm)	T of C (mins)	Rain (mm/hr)
5.000	S6	S10	7.017	0.600	18.671	18.320	0.351	20.0	150	6.99	50.0

Pipeline Schedule

Link	Length (m)	Slope (1:X)	Dia (mm)	Link Type	US CL (m)	US IL (m)	US Depth (m)	DS CL (m)	DS IL (m)	DS Depth (m)
1.000	10.113	96.3	100	Circular	22.350	21.750	0.500	22.400	21.645	0.655
1.001	36.532	34.7	150	Circular	22.400	21.645	0.605	21.850	20.591	1.109
1.002	14.000	72.9	150	Circular	21.850	20.591	1.109	21.150	20.399	0.601
1.003	14.621	72.4	150	Circular	21.150	20.399	0.601	21.000	20.197	0.653
2.000	20.022	35.9	225	Circular	21.000	20.122	0.653	20.300	19.565	0.510
2.001	11.979	65.8	225	Circular	20.300	19.565	0.510	20.450	19.383	0.842
2.002	9.615	16.8	225	Circular	20.450	19.383	0.842	20.600	18.812	1.563
2.003	10.354	690.3	225	Circular	20.600	18.812	1.563	20.350	18.797	1.328
2.004	5.356	225.0	225	Circular	20.350	18.797	1.328	20.350	18.773	1.352
2.005	12.893	126.4	225	Circular	20.350	18.773	1.352	19.920	18.671	1.024
3.000	18.592	94.4	100	Circular	19.700	18.993	0.607	19.920	18.796	1.024
4.000	11.020	76.0	100	Circular	20.300	19.744	0.456	20.300	19.599	0.601
4.001	9.749	107.1	100	Circular	20.300	19.599	0.601	20.450	19.508	0.842
5.000	7.017	20.0	150	Circular	19.920	18.671	1.099	19.920	18.320	1.450

Link	US Node	Dia (mm)	Node Type	MH Type	DS Node	Dia (mm)	Node Type	MH Type
1.000	S1	450	Manhole	Adoptable	S2	450	Manhole	Adoptable
1.001	S2	450	Manhole	Adoptable	S3	450	Manhole	Adoptable
1.002	S3	450	Manhole	Adoptable	S23	450	Manhole	Adoptable
1.003	S23	450	Manhole	Adoptable	S4	600	Manhole	Adoptable
2.000	S4	600	Manhole	Adoptable	S24	600	Manhole	Adoptable
2.001	S24	600	Manhole	Adoptable	S5	1200	Manhole	Adoptable
2.002	S5	1200	Manhole	Adoptable	S25	1200	Manhole	Adoptable
2.003	S25	1200	Manhole	Adoptable	AT1	1	Manhole	Adoptable
2.004	AT1	1	Manhole	Adoptable	S26	1000	Manhole	Adoptable
2.005	S26	1000	Manhole	Adoptable	S6	1200	Manhole	Adoptable
3.000	S9	450	Manhole	Adoptable	S6	1200	Manhole	Adoptable
4.000	S7	450	Manhole	Adoptable	S8	450	Manhole	Adoptable
4.001	S8	450	Manhole	Adoptable	S5	1200	Manhole	Adoptable
5.000	S6	1200	Manhole	Adoptable	S10	450	Manhole	Adoptable

Manhole Schedule

Node	Easting (m)	Northing (m)	CL (m)	Depth (m)	Dia (mm)	Connections	Link	IL (m)	Dia (mm)
S1	506703.234	105430.121	22.350	0.600	450				
						0	1.000	21.750	100
S2	506713.315	105429.313	22.400	0.755	450		1	1.000	21.645
						0	1.001	21.645	150
S3	506715.215	105392.830	21.850	1.259	450		1	1.001	20.591
						0	1.002	20.591	150
S4	506713.332	105364.271	21.000	0.878	600		1	1.003	20.197
						0	2.000	20.122	225
S5	506717.901	105332.597	20.450	1.067	1200		1	4.001	19.508
						2	2.001	19.383	225
						0	2.002	19.383	225
AT1	506737.402	105336.897	20.350	1.553	1		1	2.003	18.797
						0	2.004	18.797	225
S6	506740.994	105319.027	19.920	1.249	1200		1	3.000	18.796
						2	2.005	18.671	225
						0	5.000	18.671	150
S7	506706.861	105321.009	20.300	0.556	450				
						0	4.000	19.744	100
S8	506717.726	105322.850	20.300	0.701	450		1	4.000	19.599
						0	4.001	19.599	100
S9	506723.985	105311.520	19.700	0.707	450				
						0	3.000	18.993	100
S10	506746.620	105314.833	19.920	1.600	450		1	5.000	18.320
						0	1.002	20.399	150
S23	506714.294	105378.860	21.150	0.751	450		1	1.002	20.399
						0	1.003	20.399	150
S24	506716.186	105344.453	20.300	0.735	600		1	2.000	19.565
						0	2.001	19.565	225

Manhole Schedule

Node	Easting (m)	Northing (m)	CL (m)	Depth (m)	Dia (mm)	Connections	Link	IL (m)	Dia (mm)
S25	506727.291	105334.667	20.600	1.788	1200	1	2.002	18.812	225
							0	2.003	18.812
S26	506738.060	105331.582	20.350	1.577	1000	1	2.004	18.773	225
							0	2.005	18.773

Simulation Settings

Rainfall Methodology	FSR	Analysis Speed	Normal
FSR Region	England and Wales	Skip Steady State	x
M5-60 (mm)	20.000	Drain Down Time (mins)	240
Ratio-R	0.350	Additional Storage (m³/ha)	20.0
Summer CV	0.750	Check Discharge Rate(s)	x
Winter CV	0.840	Check Discharge Volume	x

Storm Durations

15 | 30 | 60 | 120 | 180 | 240 | 360 | 480 | 600 | 720 | 960 | 1440

Return Period (years)	Climate Change (CC %)	Additional Area (A %)	Additional Flow (Q %)
30	0	0	0
100	40	0	0

Node S6 Online Hydro-Brake® Control

Flap Valve	x	Objective	(HE) Minimise upstream storage
Replaces Downstream Link	✓	Sump Available	✓
Invert Level (m)	18.671	Product Number	CTL-SHE-0044-1000-1249-1000
Design Depth (m)	1.249	Min Outlet Diameter (m)	0.075
Design Flow (l/s)	1.0	Min Node Diameter (mm)	1200

Node AT1 Depth/Area Storage Structure

Base Inf Coefficient (m/hr)	0.00000	Safety Factor	2.0	Invert Level (m)	18.797
Side Inf Coefficient (m/hr)	0.00000	Porosity	0.95	Time to half empty (mins)	

Depth (m)	Area (m²)	Inf Area (m²)	Depth (m)	Area (m²)	Inf Area (m²)	Depth (m)	Area (m²)	Inf Area (m²)
0.000	216.0	0.0	0.800	216.0	0.0	0.801	0.0	0.0

Node S1 Carpark Storage Structure

Base Inf Coefficient (m/hr)	0.00000	Invert Level (m)	21.750	Slope (1:X)	31.0
Side Inf Coefficient (m/hr)	0.00000	Time to half empty (mins)	0	Depth (m)	0.360
Safety Factor	2.0	Width (m)	8.600	Inf Depth (m)	
Porosity	0.30	Length (m)	6.000		

Node S3 Carpark Storage Structure

Base Inf Coefficient (m/hr)	0.00000	Invert Level (m)	20.591	Slope (1:X)	50.0
Side Inf Coefficient (m/hr)	0.00000	Time to half empty (mins)	5	Depth (m)	0.360
Safety Factor	2.0	Width (m)	5.000	Inf Depth (m)	
Porosity	0.30	Length (m)	10.000		

Node S4 Carpark Storage Structure

Base Inf Coefficient (m/hr)	0.00000	Invert Level (m)	20.122	Slope (1:X)	30.0
Side Inf Coefficient (m/hr)	0.00000	Time to half empty (mins)	0	Depth (m)	0.360
Safety Factor	2.0	Width (m)	5.000	Inf Depth (m)	
Porosity	0.30	Length (m)	5.000		

Node S5 Carpark Storage Structure

Base Inf Coefficient (m/hr)	0.00000	Invert Level (m)	19.383	Slope (1:X)	30.0
Side Inf Coefficient (m/hr)	0.00000	Time to half empty (mins)	2	Depth (m)	0.360
Safety Factor	2.0	Width (m)	6.000	Inf Depth (m)	
Porosity	0.30	Length (m)	5.300		

Node S5 Carpark Storage Structure

Base Inf Coefficient (m/hr)	0.00000	Invert Level (m)	19.383	Slope (1:X)	30.0
Side Inf Coefficient (m/hr)	0.00000	Time to half empty (mins)	2	Depth (m)	0.360
Safety Factor	2.0	Width (m)	6.000	Inf Depth (m)	
Porosity	0.30	Length (m)	7.000		

Node S8 Carpark Storage Structure

Base Inf Coefficient (m/hr)	0.00000	Invert Level (m)	19.599	Slope (1:X)	30.0
Side Inf Coefficient (m/hr)	0.00000	Time to half empty (mins)	0	Depth (m)	0.360
Safety Factor	2.0	Width (m)	8.000	Inf Depth (m)	
Porosity	0.30	Length (m)	7.000		

Node S6 Carpark Storage Structure

Base Inf Coefficient (m/hr)	0.00000	Invert Level (m)	18.671	Slope (1:X)	30.0
Side Inf Coefficient (m/hr)	0.00000	Time to half empty (mins)		Depth (m)	0.360
Safety Factor	2.0	Width (m)	6.000	Inf Depth (m)	
Porosity	0.30	Length (m)	8.500		

Node S23 Carpark Storage Structure

Base Inf Coefficient (m/hr)	0.00000	Invert Level (m)	20.399	Slope (1:X)	40.0
Side Inf Coefficient (m/hr)	0.00000	Time to half empty (mins)	0	Depth (m)	0.360
Safety Factor	2.0	Width (m)	7.750	Inf Depth (m)	
Porosity	0.30	Length (m)	8.000		

Node S23 Carpark Storage Structure

Base Inf Coefficient (m/hr)	0.00000	Invert Level (m)	20.399	Slope (1:X)	40.0
Side Inf Coefficient (m/hr)	0.00000	Time to half empty (mins)	0	Depth (m)	0.360
Safety Factor	2.0	Width (m)	7.000	Inf Depth (m)	
Porosity	0.30	Length (m)	8.000		

Results for 30 year Critical Storm Duration. Lowest mass balance: 99.49%

Node Event	US Node	Peak (mins)	Level (m)	Depth (m)	Inflow (l/s)	Node Vol (m³)	Flood (m³)	Status
15 minute summer	S1	1	21.750	0.000	0.0	0.0000	0.0000	OK
15 minute winter	S2	10	21.712	0.067	12.6	0.0780	0.0000	OK
15 minute winter	S3	12	20.737	0.146	23.6	0.9098	0.0000	OK
15 minute winter	S4	11	20.219	0.097	33.1	0.3540	0.0000	OK
15 minute winter	S5	11	19.501	0.118	53.3	1.0084	0.0000	OK
720 minute winter	AT1	705	19.195	0.398	5.4	81.6614	0.0000	SURCHARGED
720 minute winter	S6	705	19.195	0.524	1.2	4.2560	0.0000	SURCHARGED
15 minute summer	S7	1	19.744	0.000	0.0	0.0000	0.0000	OK
15 minute winter	S8	11	19.670	0.071	5.0	0.2319	0.0000	OK
720 minute winter	S9	705	19.195	0.202	0.1	0.0322	0.0000	SURCHARGED
15 minute summer	S10	1	18.320	0.000	0.7	0.0000	0.0000	OK
15 minute winter	S23	13	20.525	0.126	20.9	1.4320	0.0000	OK
15 minute winter	S24	11	19.690	0.125	32.9	0.0355	0.0000	OK
15 minute winter	S25	7	19.439	0.627	53.6	0.7095	0.0000	SURCHARGED
720 minute winter	S26	705	19.195	0.422	0.9	0.3312	0.0000	SURCHARGED
Link Event (Upstream Depth)	US Node	Link	DS Node	Outflow (l/s)	Velocity (m/s)	Flow/Cap	Link Vol (m³)	Discharge Vol (m³)
15 minute summer	S1	1.000	S2	0.0	0.000	0.000	0.0273	
15 minute winter	S2	1.001	S3	12.4	1.016	0.410	0.4535	
15 minute winter	S3	1.002	S23	20.9	1.353	1.005	0.2298	
15 minute winter	S4	2.000	S24	32.9	1.683	0.378	0.3916	
15 minute winter	S5	2.002	S25	53.6	1.553	0.421	0.2921	
720 minute winter	AT1	2.004	S26	0.9	0.390	0.026	0.2130	
720 minute winter	S6	Hydro-Brake®	S10	0.7				35.8
15 minute summer	S7	4.000	S8	0.0	0.000	0.000	0.0316	
15 minute winter	S8	4.001	S5	4.6	0.799	0.793	0.0564	
720 minute winter	S9	3.000	S6	-0.1	-0.010	-0.008	0.1455	
15 minute winter	S23	1.003	S4	19.3	1.288	0.925	0.2197	
15 minute winter	S24	2.001	S5	32.8	1.629	0.510	0.2619	
15 minute winter	S25	2.003	AT1	54.5	1.901	2.798	0.2912	
720 minute winter	S26	2.005	S6	0.9	0.105	0.019	0.5128	

Results for 100 year +40% CC Critical Storm Duration. Lowest mass balance: 99.49%

Node Event	US Node	Peak (mins)	Level (m)	Depth (m)	Inflow (l/s)	Node Vol (m³)	Flood (m³)	Status
15 minute summer	S1	1	21.750	0.000	0.0	0.0000	0.0000	OK
15 minute winter	S2	10	21.741	0.096	22.8	0.1124	0.0000	OK
15 minute winter	S3	13	21.048	0.457	42.9	4.2368	0.0000	SURCHARGED
15 minute winter	S4	10	20.245	0.123	49.1	0.5195	0.0000	OK
15 minute winter	S5	12	19.637	0.254	84.0	3.8635	0.0000	SURCHARGED
1440 minute winter	AT1	1380	19.626	0.829	6.1	164.2626	0.0000	SURCHARGED
1440 minute winter	S6	1380	19.622	0.951	1.1	4.9852	0.0000	FLOOD RISK
15 minute winter	S7	13	19.745	0.001	0.0	0.0002	0.0000	OK
15 minute winter	S8	13	19.744	0.145	9.0	0.8540	0.0000	SURCHARGED
1440 minute winter	S9	1380	19.623	0.630	0.0	0.1001	0.0000	FLOOD RISK
15 minute summer	S10	1	18.320	0.000	0.7	0.0000	0.0000	OK
15 minute winter	S23	16	20.592	0.193	29.4	3.3385	0.0000	SURCHARGED
15 minute winter	S24	11	19.743	0.178	48.7	0.0504	0.0000	OK
1440 minute winter	S25	1380	19.625	0.813	5.9	0.9197	0.0000	SURCHARGED
1440 minute winter	S26	1380	19.622	0.849	1.6	0.6665	0.0000	SURCHARGED
Link Event (Upstream Depth)	US Node	Link	DS Node	Outflow (l/s)	Velocity (m/s)	Flow/Cap	Link Vol (m³)	Discharge Vol (m³)
15 minute summer	S1	1.000	S2	0.0	0.000	0.000	0.0384	
15 minute winter	S2	1.001	S3	22.6	1.385	0.744	0.5401	
15 minute winter	S3	1.002	S23	29.4	1.671	1.412	0.2465	
15 minute winter	S4	2.000	S24	48.7	1.753	0.560	0.5594	
15 minute winter	S5	2.002	S25	77.2	1.945	0.606	0.3824	
1440 minute winter	AT1	2.004	S26	1.6	0.387	0.046	0.2130	
1440 minute winter	S6	Hydro-Brake®	S10	0.9				74.3
15 minute winter	S7	4.000	S8	0.0	0.007	-0.001	0.0432	
15 minute winter	S8	4.001	S5	6.9	0.883	1.175	0.0763	
1440 minute winter	S9	3.000	S6	0.2	0.028	0.035	0.1455	
15 minute winter	S23	1.003	S4	22.0	1.299	1.054	0.2499	
15 minute winter	S24	2.001	S5	48.5	1.664	0.756	0.4403	
1440 minute winter	S25	2.003	AT1	5.9	0.491	0.300	0.4118	
1440 minute winter	S26	2.005	S6	0.9	0.103	0.020	0.5128	

Appendix C

Hydro-Brake maintenance schedule

Hydro International Warranty, Maintenance and COSHH Statement

This statement covers all Hydro International Hydro-Brake Optimum®, Hydro-Brake® and S-Type Vortex Flow Control units (hereafter referred to as "Flow Control")

Defects Liability Period Warranty

The standard Hydro International warranty covers products for one year from date of purchase and will only be considered for those defects or faults reported directly to Hydro International in writing. Warranties will not be considered for defects or faults caused by accident, acts of vandalism or unauthorised repair/rectification. Please contact Hydro International for product specific details.

Maintenance

Normally, little maintenance is required as there are no moving parts within the Flow Control. Experience has shown that if blockages occur they do so at the intake, and the cause on such occasions has been due to a lack of attention to engineering detail such as approach velocities being too low, inadequate benching, or the use of units below the minimum recommended size. The Flow Control (where applicable) is fitted with a pivoting by-pass door, which allows the manhole chamber to be drained down should blockages occur. The smaller type conical units, below the minimum recommended size, are also supplied with rodding facilities or vortex suppressor pipes as standard.

Following installation of the Flow Control it is vitally important that any extraneous material i.e. Building materials are removed from the unit and the chamber. After the system is made live, and assuming that the chamber design is satisfactory, it is recommended that each unit be inspected monthly for three months and thereafter at six monthly intervals with hose down if required. If problems are experienced please do not hesitate to contact the company so that an investigation may be made.

All Flow Controls are typically manufactured from grade 304 Stainless Steel which has an estimated life span in excess of the design life of drainage systems.

COSHH

Flow Controls are manufactured from Stainless Steel, which is not regarded as hazardous to health and exhibits no chemical hazard when used under normal circumstances for the stated applications.

Manual Handling

The handling of Flow Controls should be in accordance with current legislation and regulations:

- The Health and Safety at Work Act 1972.
- The Management of Health and Safety at Work Regulations 1999.
- The Manual Handling Operations Regulations 1992.

All published and printed by the Health and Safety Executive.



Appendix D

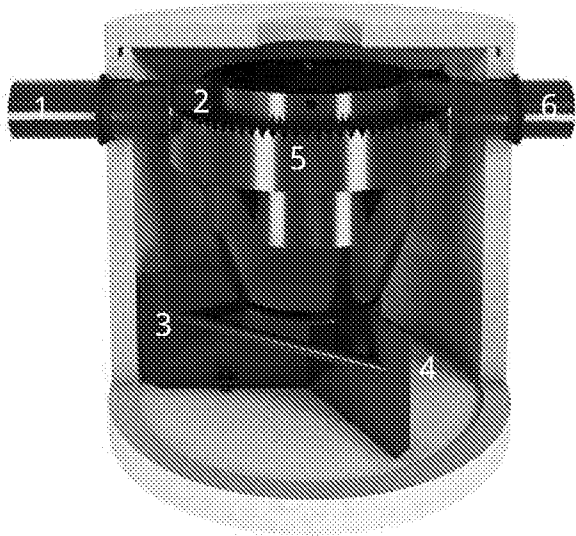
Storm Shark Maintenance Schedule

Product Description



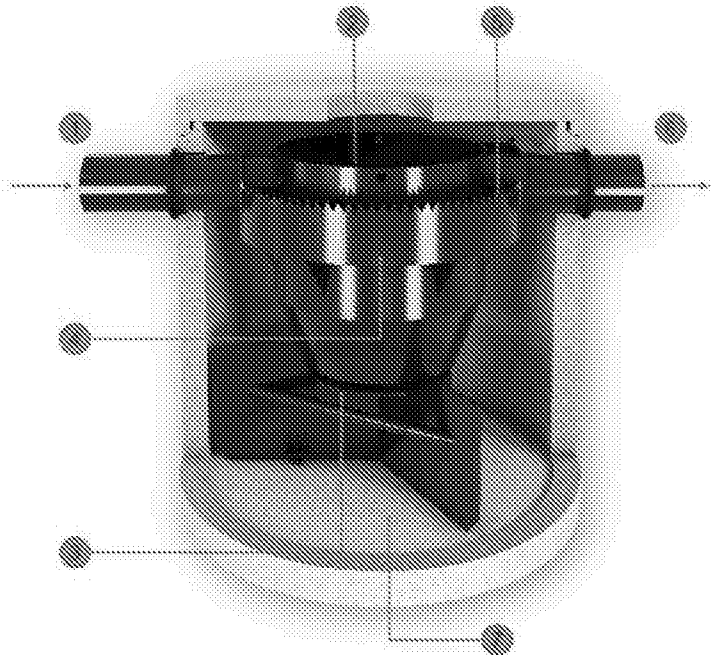
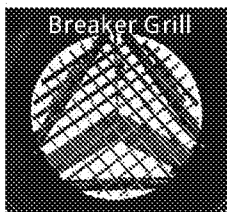
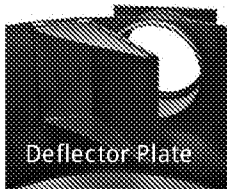
All 7 sizes in both Standard and Max ranges of the STORMSHARK have the same operational components.

1. Inlet
2. Deflector plate
3. Flow breakers
4. Sludge trap
5. Balancing weir
6. Outlet



- ⌋ The STORMSHARK must only be handled and lifted by competent persons.
- ⌋ Avoid any concentrated point loading. Unloading, storage and transport at site are the responsibility of the Purchaser/Site.
- ⌋ Backfill and excavation works are by others, in accordance with site designs, site ground conditions and legal requirements.
- ⌋ If you are in any doubt as to the function, applicability or safety of the device as supplied at site, you must seek Professional Advice from your supplier and/or a Professional Engineer.
- ⌋ The water quality flow rates and hydraulic peak capacity for the site would have been fully designed.

Working Principles



1. Incoming stormwater is deflected into a radial flow pattern.
2. Vortex flow ensures solids settle to the sludge chamber, floatables are retained at surface.
3. Solids pass down into the sludge chamber below the treatment chamber. Remobilisation of the retained solids is not possible; flow baffles and grill prevent this.
4. Cleaned water flows up the outer chamber, in an even flow distribution.
5. Water flows over the toothed balancing weir to the annular flow channel, surrounding the treatment chamber.
6. Clean stormwater then passes to the outlet to discharge to the downstream drainage system or directly into the watercourse.
7. Retained solids are easily removed by gully sucker or suction hose, once the flow breaker grill is lifted using the lifting chain supplied and fitted within the unit.

Operating and Maintenance Instructions



Due to the solids and pollutants in stormwater runoff, stormwater treatment systems must be checked and cleaned at regular intervals. The following notes provide guidance for the typical work necessary or associated with the effective operation of the STORMSHARK.

Annual maintenance:

- ⌘ The Unit should be checked and cleaned if and as required upon commissioning. This is vital to ensure construction phase debris has not accumulated. In the first year we recommend an initial check at 6 months, always.
- ⌘ Subsequently, at intervals between 6 months and 3 years, the sludge trap must be emptied as site conditions or local regulations or site specific maintenance require. The amount of sludge accumulating depends on the local stormwater conditions and will vary between sites. Those nearer the coast may need to have an increased level of management, due to the potential of increased amounts of silt and sand entering the drainage system and entering the STORMSHARK.
- ⌘ For stormwater runoff with untypically low or high solids loads, these interval periods will vary. This variation can be observed and determined to prepare a Programmed Maintenance Schedule, after the first few years of operation.
- ⌘ The STORMSHARK is classed as a Confined Space. However it is not necessary for man entry into the STORMSHARK for routine maintenance. The flow breaker floor can be lifted using the lifting wire supplied (which should be extended according to the overall chamber depth. Local regulations must be fully observed in the event of planned or unplanned man entry. If in doubt you must consult with a Professional Engineer or other Competent Person who can advise you.

Equipment and materials typically required



-
- ⌘ Suction and flushing vehicle (with tanker) or submersible sludge pump with hoses
 - ⌘ Generator when there is no power connection nearby
 - ⌘ High-pressure cleaner or flushing lance for connection to the pump
 - ⌘ Tanker to take away the sludge and waters, if required in addition to the suction plant.

Important Pollution Prevention Notes

- ⌘ The water pumped out of the chamber and the sludge trap may only be discharged into a foul sewer or a combined sewer where local regulations allow. In no case may the water be discharged into a surface water course, a stormwater sewer or a groundwater infiltration system.
- ⌘ If there is no possibility to discharge the water, a mobile water treatment system may be used, or else the waters must be taken to a Municipal Waste water Treatment Plant or Landfill site. A registered waste contractor should be employed, and the waters treated as a Controlled Waste as appropriate.
- ⌘ The treated water can ONLY be discharged into a foul water or storm sewer in accordance with local permits and Regulations.

Maintenance Instructions



1. First remove the layer of floatables and oils on the water surface. Then lower the water level down to the level of the grate by means of the suction hose.
2. Remove any coarse debris from the grate and then pull up the grate with help of the looped straining wire.
3. Suck the sludge and the solids from the sludge trap chamber. Make sure that you clear each section of the trap. If necessary, rinse with water.
4. Close the grate, check the lock and finally securely close the top most cover of the system.

If you are in any doubt you must consult a Professional Engineer or Competent Person to advise you on Maintenance. Or Contact us - Turtle Enviro on 0191 3036315

Commissioning and Audit Protocol for STORMSHARK
Please use this as an example template.
Modify to suit local conditions.



Building Project Owner represented by

Specialist Company represented by

No	Feature	Remarks
1.	Standard Model only: The STORMSHARK was fixed into the chamber housing by means of prescribed fixing anchors.	
2.	Standard Model only: The STORMSHARK insert and pipework are aligned.	
3.	Standard Model only : The uniform distance of the balancing weir to the chamber wall was checked.	
4.	Standard Model only: The bottom of the chamber was filled with minimum 10 cm of concrete, or is otherwise secured by welding etc	

The instruction for the operation of the plant have been handed over. The required operating documents and existing operating and maintenance instructions for this installation were handed over in full.

Signature of Contractor

Signature of Owner's representative

Maintenance Record Sheets

Please use this as an example template.

Modify to suit local conditions.



Maintenance Date	Remarks	Maintenance work performed	Name and Signature
Date:		- ó Inspection of the system for visible damage - ó Emptying and cleaning of the sludge trap - ó Grate is closed and locked	

Date:		- ó Inspection of the system for visible damage - ó Emptying and cleaning of the sludge trap - ó Grate is closed and locked	
-------	--	---	--

Date:		- ó Inspection of the system for visible damage - ó Emptying and cleaning of the sludge trap - ó Grate is closed and locked	
-------	--	---	--



Appendix E

Groundwater Monitoring Data



KEY:

- :recycle + refuse bins
- :pedestrian paving
- :1.8m screen fence
- :garden gates
- :secure cycle parking
- :non adoptable road
- :disabled parking bay
- :private driveway and parking bay
- :visitor parking bay
- :site boundary

note: refer to HUSKISSON
BROWN landscape layout for
material details: HBA/790/01

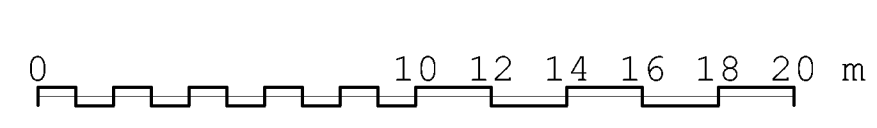
This drawing is protected under
The Copyright, Designs and Patents Act 1988

**THIS DRAWING IS TO BE READ IN CONJUNCTION
WITH ALL OTHER PROJECT, CONSULTANT +
SPECIALIST CONTRACTORS DOCUMENTATION
Do not scale, except for Planning purposes only.
Figured dimensions only are to be taken from this drawing. All
dimensions are to be checked on site before any work is put
in hand.**

notes:

key:

revisions:
FIRST ISSUE:



Arundel Road, Angmering
Groundwater monitoring

Date and depth of water (mbgl)												
Trial Pit	15-Jan	22-Jan	29-Jan	05-Feb	12-Feb	19-Feb	26-Feb	04-Mar	11-Mar	18-Mar	25-Mar	01-Apr
SAT3 1.6m dp	N/A	N/A	N/A	1.55	1.45	1.4	1.5	1.3	1.4	N/A	N/A	N/A