

Drainage Maintenance and Management Schedule



Project	Land West of Hook Lane, Westergate
Project Number	23582

By Stuart Magowan

Date 9 March 2023

1 Introduction

- 1.1 The Surface Water system on site drains surface water from roofs via catchpits and distributions tanks into a lined permeable stone located below the access road and parking areas.
- 1.2 The parking areas are permeable and drain directly to the permeable stone below permeable surfacing.
- 1.3 The permeable stone discharges surface water at a restricted rate of 2l/s to the existing drainage ditch approximately 85m west of the north west site boundary.
- 1.4 The permeable stone provides storage for all rainfall event up to the 100 year plus climate change event.
- 1.5 The permeable stone and permeable road and parking construction provides suitable water quality mitigation to meet the level of risk on the site.

2 Schedule of Maintenance

- 2.1 Once appointed the Contractor will prepare a site specific method statement for the control of silt and other pollutants during construction. CIRIA Report C532, Control of water pollution from construction sites, provides further guidance on this.
- 2.2 The Contractor will maintain the proposed drainage system during construction and until the handing over of the site.
- 2.3 Upon completion the Principal Contractor will collate the data sheets, operation and maintenance details of all materials used in the construction of the site drainage system.
- 2.4 These details will issued to the Management Company for their records.
- 2.5 Upon completion management of shared drainage facilities (where not adopted by a Statutory Undertaker) will be passed on to a Management Company appointed by the



Certificate No 87852002

Developer on behalf of the Residents. The Management Company will be funded by the residents.

- 2.6 In the event that the Management Company becomes unable to discharge its duties within two years of first appointment the Developer will endeavour to appoint an alternative on behalf of the Residents.
- 2.7 Maintenance of individual property drainage connections is the responsibility of the individual property owners.
- 2.8 The following maintenance schedule details the typical tasks to be undertaken at different intervals.

Maintenance Schedule	Required Action	Frequency
Regular Maintenance	Manage vegetation and remove nuisance plants – aesthetics	As required
	Litter and debris removal – catchpits	Monthly or as required
	Cleaning of gutters and any filters on downpipes	3 Monthly
	Remove sediment and debris from silt trap chambers, channel drains and inlet chambers	6 monthly
	Visual inspection of permeable paving for defects and settlement	Annually
	Sweeping / brushing of permeable paving	Every 2 years
	Surface and foul water pipework – jetting / rodding	Every 2 years or as required
Corrective Maintenance	Remove debris / blockages to silt traps / channel drains / headwalls	As required
	Repairs to access chambers / manhole covers	As required
	Replace any broken permeable blocks / surface, remedial works to any depressions or rutting	As required
	Inspect inlet, outlet from downpipes, channel drains, headwalls and gullies for blockages, standing water and clear	As required
	Reconstruct storage structures if performance deteriorates or failure occurs	As required
Monitoring	Inspect silt traps and note the rate sediment has accumulated	Monthly in the first year and then annually
	Inspect storage structures to ensure they are fully emptying	Annually

Indicative Schedule of Maintenance for the Proposed Drainage System

2.9 Any parts, materials or products that require replacement or come to the end of the manufacture's recommended design life will be replaced by the Management Company or individual property owner as required.

Component	Inspection Frequency					
	1 Month	3 Months	6 Months	1 Year	After leaf fall in Autumn	2 Years
Gullies, Channels and Gutters		✓			✓	
Catchpits	✓				✓	
Surface and Foul Water Pipework						✓
Permeable Paving				✓		
Flow Controls			✓			
Storage Facilities				✓		

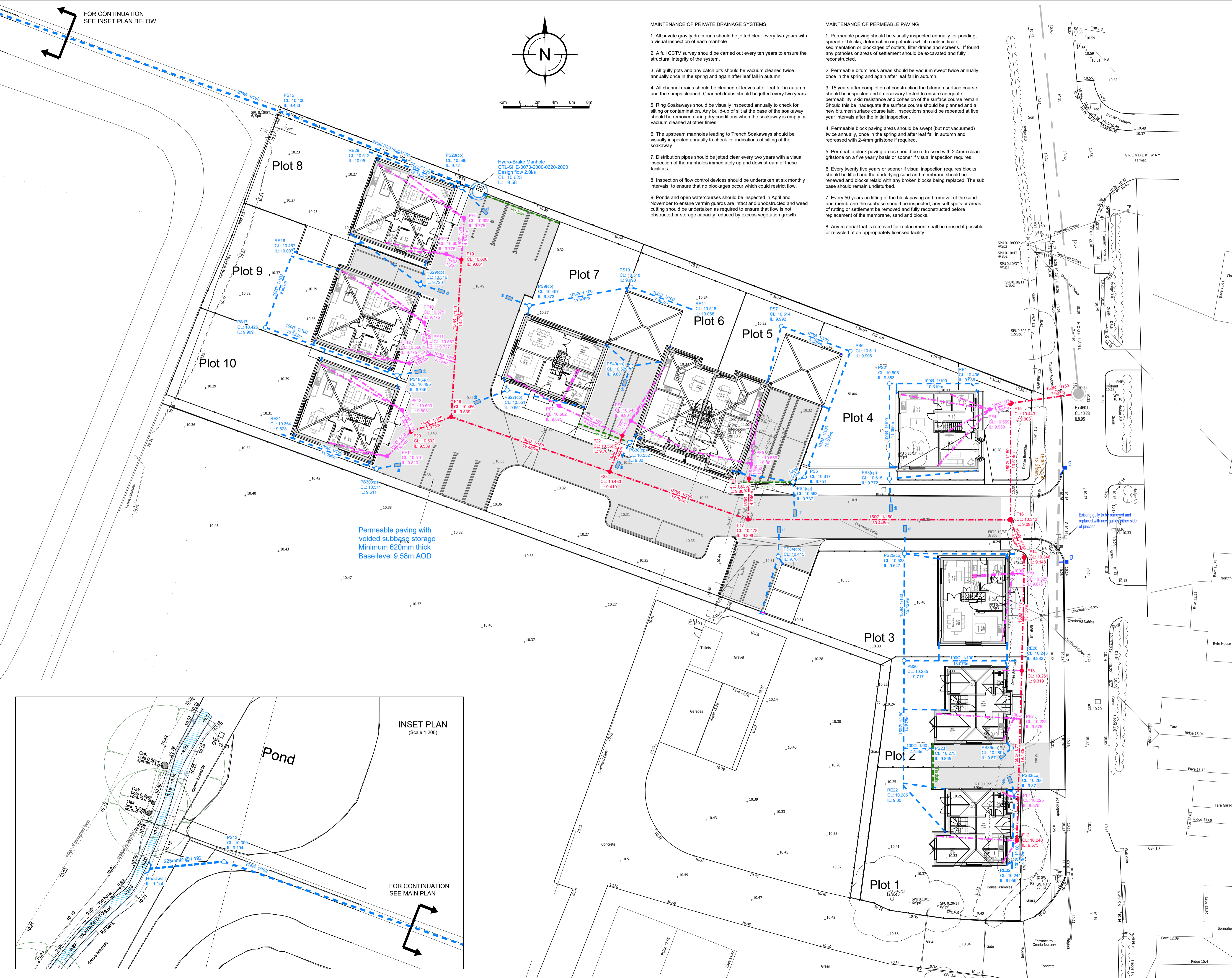
Inspection Frequency Summary

3 Design Life

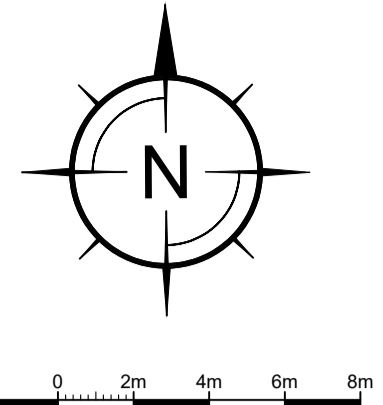
- 3.1 The design life of the development is likely to exceed the design life of the components within the SuDS network. During the routine drainage inspections it may be determined that some components have reached the end of their functional life cycle.
- 3.2 Where possible repairs should be the first option considered however if repairs are unviable it will be necessary for the property owner / Management Company to replace the faulty component.

4 Emergency Plan

- 4.1 Potential flood and maintenance indicators:
- Manholes or inspections chambers overflowing
 - Gullies overflowing or ponding
 - Channel drains overflowing or ponding
 - Other visual indicators of the drainage system not performing as it should
- 4.2 Should any of the items above occur then immediate action as outlined below should be undertaken:
- Inspect for blockages in the problem area
 - Should the problem not be identified via an initial inspection:
 - For unadopted onsite drainage the Management Company should appoint a suitable drainage engineer to inspect and survey the system and jet any blockages
 - For adopted onsite drainage the relevant statutory undertaker should be alerted
 - Where it is suspected that there is a problem with the downstream drainage network the Owner or relevant statutory undertaker of that system should be alerted



FOR CONTINUATION
SEE INSET PLAN BELOW



MAINTENANCE OF PRIVATE DRAINAGE SYSTEMS

1. All private gravity drain runs should be jetted clear every two years with a visual inspection of each manhole.
2. A full CCTV survey should be carried out every ten years to ensure the structural integrity of the system.
3. All gully pots and any catch pits should be vacuum cleaned twice annually once in the spring and again after leaf fall in autumn.
4. All channel drains should be cleaned of leaves after leaf fall in autumn and the sumps cleaned. Channel drains should be jetted every two years.
5. Ring Soakaways should be visually inspected annually to check for silting or contamination. Any build-up of silt at the base of the soakaway should be removed during dry conditions when the soakaway is empty or vacuum cleaned at other times.
6. The upstream manholes leading to Trench Soakaways should be visually inspected annually to check for indications of silting of the soakaway.
7. Distribution pipes should be jetted clear every two years with a visual inspection of the manholes immediately up and downstream of these facilities.
8. Inspection of flow control devices should be undertaken at six monthly intervals to ensure that no blockages occur which could restrict flow.
9. Ponds and open watercourses should be inspected in April and November to ensure vermin guards are intact and unobstructed and weed cutting should be undertaken to ensure that flow is not obstructed or storage capacity reduced by excess vegetation growth.

MAINTENANCE OF PERMEABLE PAVING

1. Permeable paving should be visually inspected annually for ponding, spread of blocks, deformation or potholes which could indicate sedimentation or blockages of outlets, filter drains and screens. If found any potholes or areas of settlement should be excavated and fully reconstructed.
2. Permeable bituminous areas should be vacuum swept twice annually, once in the spring and again after leaf fall in autumn.
3. 15 years after completion of construction the bitumen surface course should be inspected and if necessary tested to ensure adequate permeability, skid resistance and cohesion of the surface course remain. Should this be inadequate the surface course should be planned and a new bitumen surface course laid. Inspections should be repeated at five year intervals after the initial inspection.
4. Permeable block paving areas should be swept (but not vacuumed) twice annually, once in the spring and again after leaf fall in autumn and redressed with 2-4mm gritstone if required.
5. Permeable block paving areas should be redressed with 2-4mm clean gritstone on a five yearly basis or sooner if visual inspection requires.
6. Every twenty five years or sooner if visual inspection requires blocks should be lifted and the underlying sand and membrane should be renewed and blocks relaid with any broken blocks being replaced. The sub base should remain undisturbed.
7. Every 50 years on lifting of the block paving and removal of the sand and membrane the subbase should be inspected, any soft spots or areas of rutting or settlement be removed and fully reconstructed before replacement of the membrane, sand and blocks.
8. Any material that is removed for replacement shall be reused if possible or recycled at an appropriately licensed facility.

Permeable paving with voided subbase storage
Minimum 620mm thick
Base level 9.58m AOD

GENERAL NOTES:

1. All dimensions to be checked on site. All details and dimensions relating to sub-Contractors work must be checked and agreed between the sub-Contractor or supplier and the general Contractor.
2. This drawing is to be read in conjunction with all relevant Architect's and Engineer's drawings and specification.
3. The main Contractor is responsible for ensuring the stability of the structure whilst the works are in progress.
4. Any information given regarding existing underground services is given in good faith after consultation with the relevant authority. No liability is accepted by the Consultant and the main Contractor is responsible for obtaining and checking all information and taking due care and attention whilst undertaking the works.

DRAINAGE NOTES:

1. All adoptable pipes, bends and junctions shall be vitrified clay in accordance with the current version of BS EN 295-1, with flexible joints and kitemark certified.
2. All Adoptable sewers shall be in strict accordance with the SSG Appendix C - Design and Construction Guidance. Unless otherwise stated adoptable sewers shall be 150mm diameter and shall be laid in a class S bedding. Where the depth to soffit is less than 1.2m under a public highway or 0.9m elsewhere the pipe shall be laid with a class Z bedding.
3. Where a pipe is within 1m of a foundation the trench shall be filled with class GEN 3 concrete up to the lowest level of the foundation. Where the trench is further than 1m from the foundation, the trench shall be filled with class GEN 3 concrete to a level below the lowest level for the foundation equal to the distance from the foundation less 150mm. In both cases the pipe shall be bedded and surrounded in 150mm thick class GEN 3 concrete.
4. Where pipes, external to the structures, have a depth to soffit from ground level of less than 450mm they shall have a class GEN 3 concrete encasement (150mm thick). In all other cases the pipes shall be bedded and surrounded with 100mm thick granular material.
5. In any circumstances where pipes are bedded and surrounded in concrete flexible joints should be provided. Compressible boards (fibreglass or polystyrene) shall be provided at a maximum of 8m centres (coinciding with pipe joints). The boards shall be pre-cut to pipe diameter and to a height and width equal to the concrete cross section. A board thickness of 18mm for pipes up to 450mm nominal diameter and 36mm for pipes over 450mm nominal diameter.
6. Where existing pipes are to be abandoned they shall be dug out together with any abandoned manholes.
7. Any discrepancy between the drawing and site should be reported immediately to the Engineer.
8. All manhole and chamber sizes are given as a minimum to meet the SSG Appendix C-Design and Construction Guidance.

DRAINAGE KEY:	
Existing Sewers	Existing Storm Sewer
Existing Storm Sewer	Existing Foul Sewer
Private Storm Drainage	
PS (cp)	Inspection Chamber / Catchpit
RE	Rodding Eye
dt	Distribution Tank (Single)
Permeable Sub Base	Permeable Sub Base
Fin drain	Fin drain
Private Foul Drainage	
IC	Inspection Chambers
Adoptable Highway Drainage	
G	Road Gully
Adoptable Foul Drainage	
F	Inspection Chambers

E 09.03.23	AJD Minor amendments	SRM		
D 17.02.23	MJP Updated subbase depth and Hydrobrake	SRM		
C 02.11.22	MJP PS20, PS25, Hydrobrake, and permeable paving updated	SRM		
B 12.08.22	MJP Updated subbase depth. PS28 & PS29 IL	SRM		
A 28.07.22	AJD Updated to suit block plans	SRM		
REV	DATE	INT	DESCRIPTION	CHK

Issue Status **FOR APPROVAL**

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CLIENT		PROJECT NO.	
Oceanview Developments		23582	
PROJECT		REV.	
Land West Of Hook Lane, Hook Lane, Westergate		202 E	
TITLE		SCALE	
Site Layout - Drainage		1:200 @ A1, 1:400 @ A3	
DRAWN	DATE	PROJECT NO.	
A Davies	July 2022	23582	
ENGINEER	CHECKED	DRG. No.	REV.
S Magowan	SRM	202	E

ADOPTABLE FOUL								
MH No.	MANHOLE DIAMETER (mm)	MANHOLE TYPE	COVER LEVEL (m)	INVERT LEVEL (m)	DEPTH TO SOFFIT (m)	EASTING (m)	NORTHING (m)	COVER TYPE
F12	450	Type E	10.240	9.575	0.515	493454.036	104556.645	B125
F13	450	Type E	10.261	9.319	0.792	493454.608	104576.369	B125
F14	450	Type E	10.346	9.148	1.048	493454.903	104589.496	B125
F15	600	Type D	10.443	9.003	1.290	493453.303	104607.386	B125
F16	600	Type D	10.312	9.093	1.068	493453.303	104593.872	D400
F17	450	Type D	10.478	9.296	1.032	493422.857	104593.903	D400
F18	450	Type E	10.456	9.539	0.766	493388.385	104605.802	D400
F19	450	Type E	10.600	9.661	0.788	493389.572	104624.055	D400
F20	450	Type E	10.502	9.589	0.763	493383.715	104604.092	D400
F21	450	Type E	10.557	9.800	0.607	493422.946	104598.648	B125
F22	450	Type E	10.550	9.700	0.700	493408.265	104603.641	B125
F23	450	Type D	10.483	9.410	0.924	493406.784	104599.451	D400

PRIVATE								
MH No.	MANHOLE DIAMETER (mm)	MANHOLE TYPE	COVER LEVEL (m)	INVERT LEVEL (m)	DEPTH TO SOFFIT (m)	EASTING (m)	NORTHING (m)	COVER TYPE
PS2	180	Type E	10.505	9.883	0.522	493439.253	104609.674	A15
PS3	500	Catchpit	10.615	9.772	0.742	493439.253	104598.619	A15
PS4	500	Catchpit	10.563	9.737	0.726	493428.073	104598.257	B125
PS5	450	Type E	10.617	9.751	0.766	493429.407	104598.853	A15
PS6	450	Type E	10.511	9.906	0.505	493434.709	104613.419	A15
PS7	450	Type E	10.514	9.992	0.422	493426.688	104616.337	A15
PS9	500	Catchpit	10.497	9.873	0.524	493397.455	104618.714	A15
PS10	450	Type E	10.518	9.993	0.425	493409.255	104620.874	A15
PS12	450	Type E	10.542	9.767	0.675	493391.787	104632.449	A15
PS13	450	Type E	10.300	9.194	0.881	493322.086	104657.401	A15
PS15	450	Type E	10.400	9.453	0.722	493369.060	104641.069	A15
PS17	450	Type E	10.425	9.909	0.416	493366.817	104616.178	A15
PS18	500	Catchpit	10.495	9.746	0.649	493382.092	104610.624	A15
PS20	450	Type E	10.285	9.701	0.434	493440.934	104577.529	A15
PS23	450	Type E	10.273	9.865	0.309	493443.645	104567.349	A15
PS25	500	Catchpit	10.528	9.618	0.760	493440.986	104589.957	A15
PS27	500	Catchpit	10.551	9.651	0.800	493394.887	104608.915	A15
PS28	500	Catchpit	10.586	9.720	0.766	493390.220	104632.168	A15
PS29	500	Catchpit	10.516	9.720	0.696	493384.808	104621.163	B125
PS30	500	Catchpit	10.511	9.511	0.900	493379.698	104599.791	D400
PS33	500	Catchpit	10.266	9.870	0.296	493453.749	104562.300	A15
PS34	500	Catchpit	10.415	9.700	0.615	493426.348	104589.603	B125
PS36	500	Catchpit	10.280	9.870	0.310	493453.415	104567.384	B125
PS38	500	Catchpit	10.552	9.800	0.652	493409.114	104602.848	B125
PS39	180	Type E	10.495	9.495	0.900	493407.559	104589.898	A15
PS40	500	Catchpit	10.529	9.800	0.629	493409.240	104611.701	B125
RE1	110	Rodding Eye	10.436	9.986	0.350	493449.563	104609.674	A15
RE11	110	Rodding Eye	10.518	10.068	0.350	493416.304	104618.293	A15
RE16	110	Rodding Eye	10.457	10.007	0.350	493370.194	104625.443	A15
RE22	110	Rodding Eye	10.285	9.800	0.335	493440.872	104562.653	A15
RE26	110	Rodding Eye	10.245	9.882	0.264	493454.006	104577.529	A15
RE29	110	Rodding Eye	10.512	10.050	0.362	493378.901	104636.543	A15
RE31	110	Rodding Eye	10.364	9.628	0.635	493368.716	104603.785	A15
RE32	110	Rodding Eye	10.244	9.959	0.185	493453.491	104553.401	A15
PF1	300	Type E	10.225	9.575	0.550	493453.237	104561.811	A15
PF2	300	Type E	10.225	9.575	0.550	493453.695	104570.819	A15
PF3	450	Type E	10.325	9.675	0.550	493453.561	104587.644	A15
PF4	450	Type E	10.559	9.959	0.500	493450.904	104606.670	A15
PF5	450	Type E	10.556	9.900	0.556	493423.220	104600.354	B125
PF6	450	Type E	10.545	9.900	0.545	493409.218	104605.367	B125
PF7	450	Type E	10.562	9.875	0.587	493400.937	104607.055	A15
PF8	450	Type E	10.605	9.775	0.730	493389.885	104628.859	B125
PF9	450	Type E	10.604	9.775	0.729	493387.817	104624.694	B125
PF10	450	Type E	10.575	9.775	0.700	493385.269	104616.747	B125
PF11	450	Type E	10.567	9.727	0.740	493385.815	104613.101	D400
PF12	450	Type E	10.558	9.775	0.683	493383.535	104612.015	B125
PF13	450	Type E	10.503	9.803	0.600	493382.695	104606.580	B125
PF14	450	Type E	10.510	9.810	0.600	493380.952	104601.235	D400

Cover Types

Carriageway: D400

Drives: B125 or C250

Non vehicular areas: A15

See Drawing 23582/203 - Surface Finishes Layout Plan

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- Any discrepancy between the drawing and site should be reported immediately to the Engineer.
- All manhole and chamber sizes are given as a minimum to meet the SSG Appendix C-Design and Construction Guidance

D	09.03.23	AJD	Drawing updated	SRM
C	17.02.23	MJP	Table regarding cover types added	SRM
B	16.02.23	MJP	Updated suit updated plan	SRM
A	12.08.22	MJP	Updated subbase depth. PS28 & PS29 IL	SRM
REV	DATE	BY	DESCRIPTION	CHK

Issue Status **PRELIMINARY**

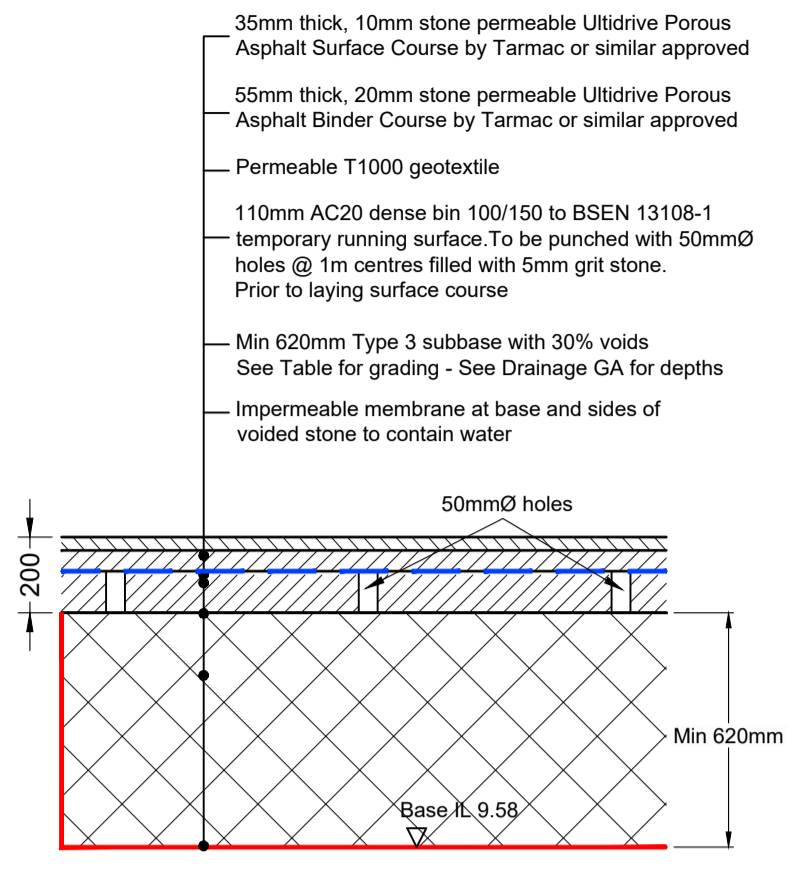
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CLIENT Oceanview Developments

PROJECT Land West Of Hook Lane, Hook Lane, Westergate

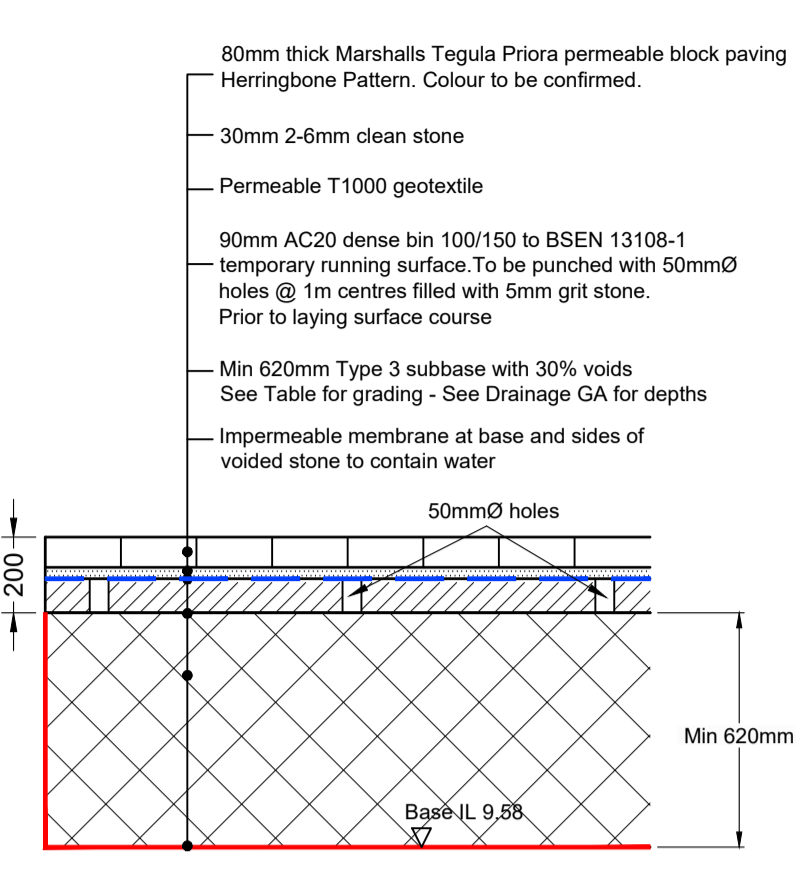
TITLE Manhole Schedule

DRAWN	DATE	PROJECT NO.
A Davies	July 2022	23582
DRAWN	CHECKED	REV.
S Magowan	SRM	
SCALE	DWG. No.	REV.
NTS	250	D

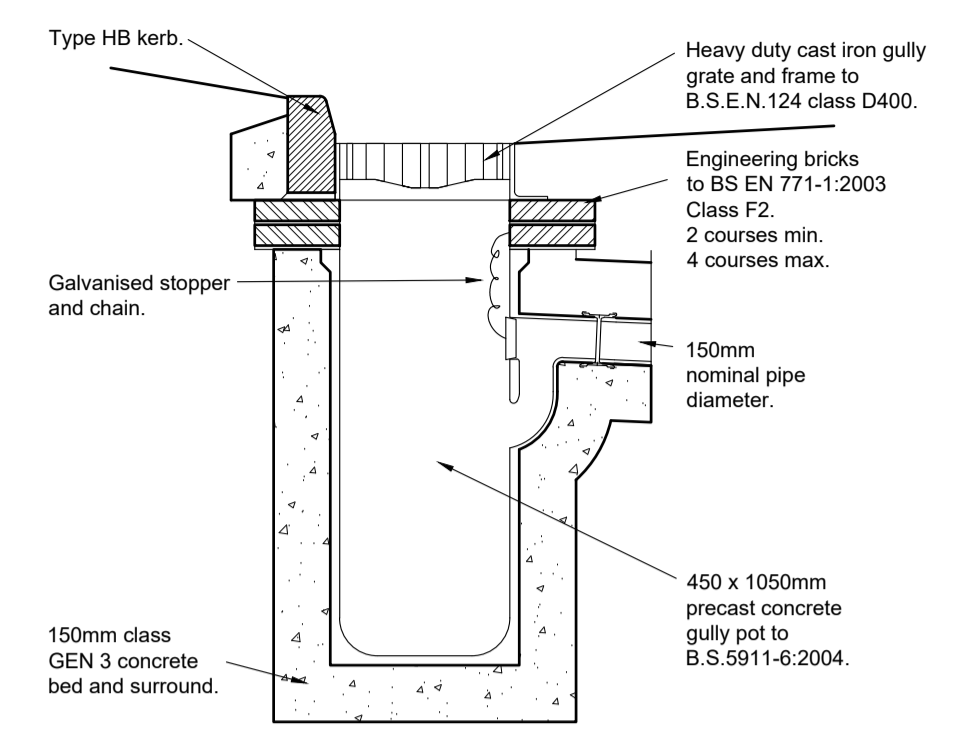


TYPE A - PERMEABLE TARMAC ACCESS ROAD CONSTRUCTION WITH TEMPORARY RUNNING COURSE
(Scale 1:20)

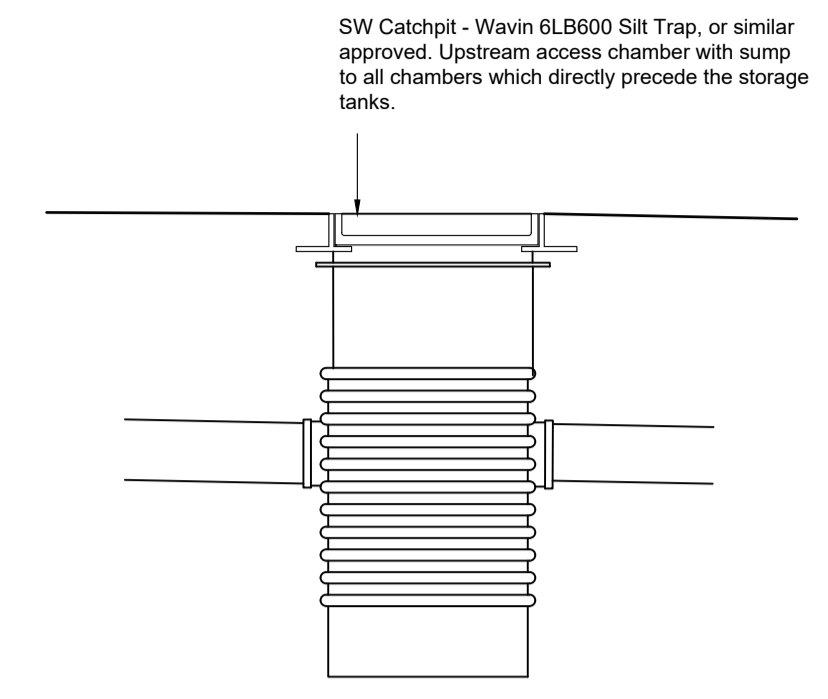
SEIVE SIZE	% PASSING
10mm	100
63mm	90-100
37.5mm	80-90
20mm	15-30
10mm	5-8



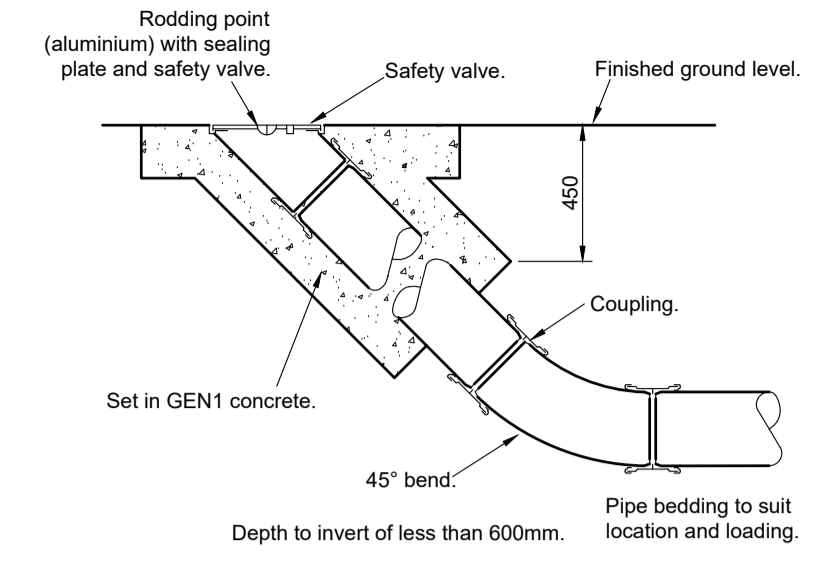
TYPE B - PERMEABLE BLOCK PAVING CAR PARK BAY CONSTRUCTION WITH TEMPORARY RUNNING COURSE
(Scale 1:20)



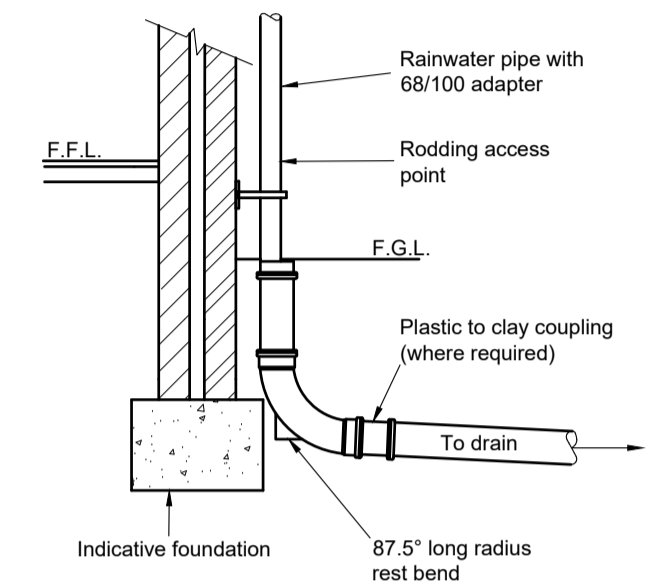
TYPICAL ROAD GULLY
(Scale 1:20)



CATCHPIT CHAMBER
(Scale 1:20)

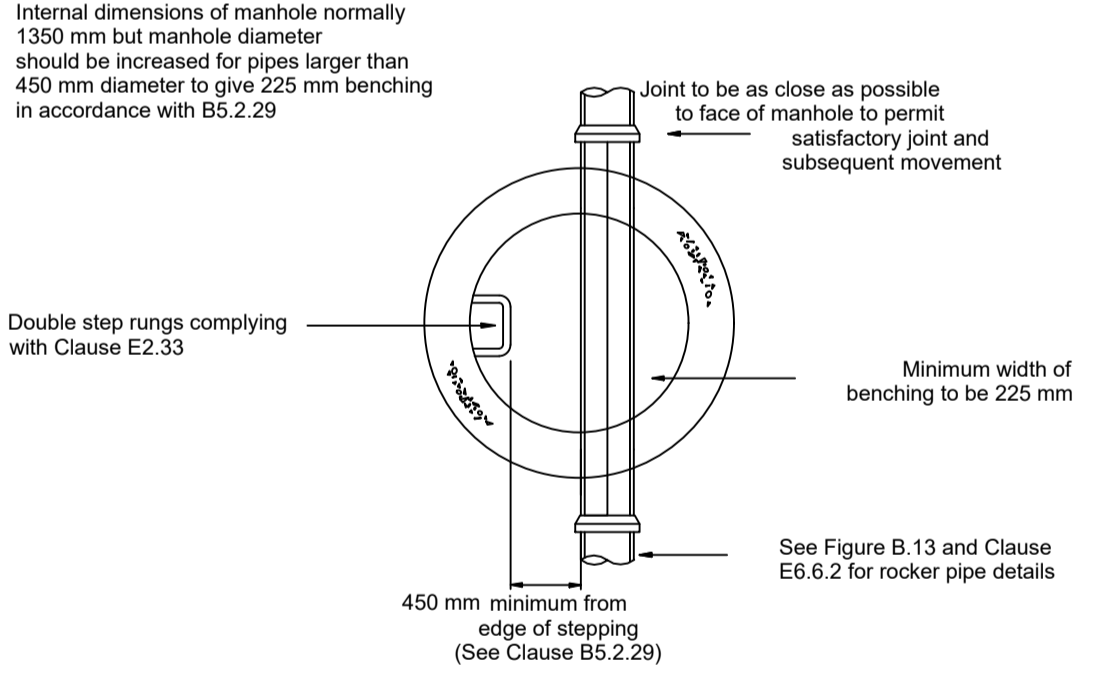
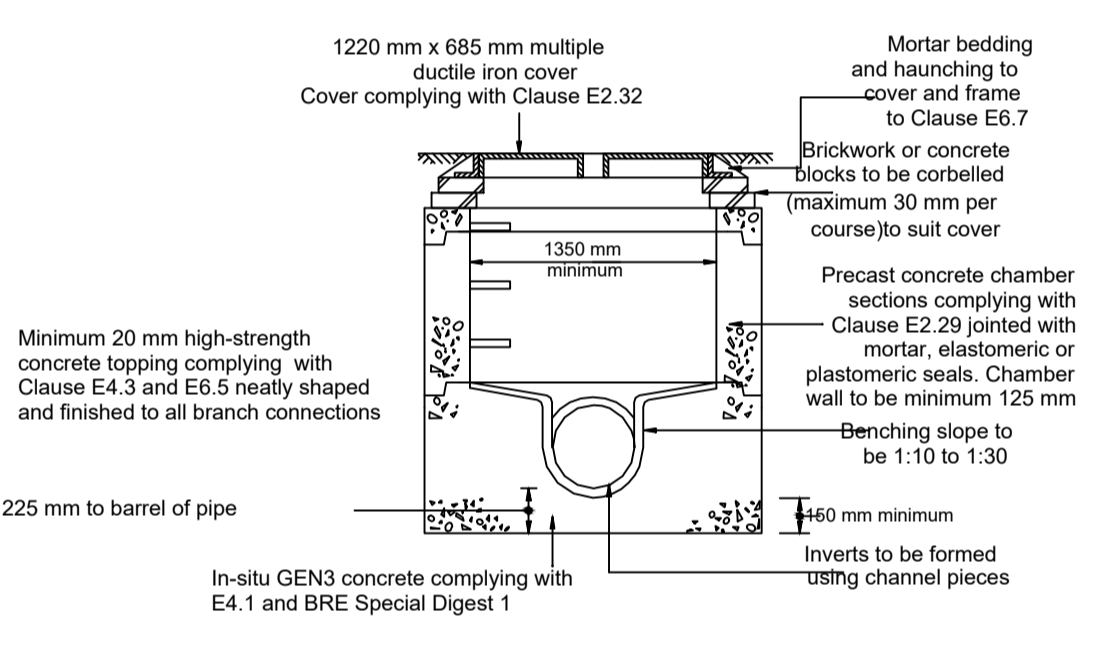


DETAIL OF RODDING EYE
(Scale 1:25)



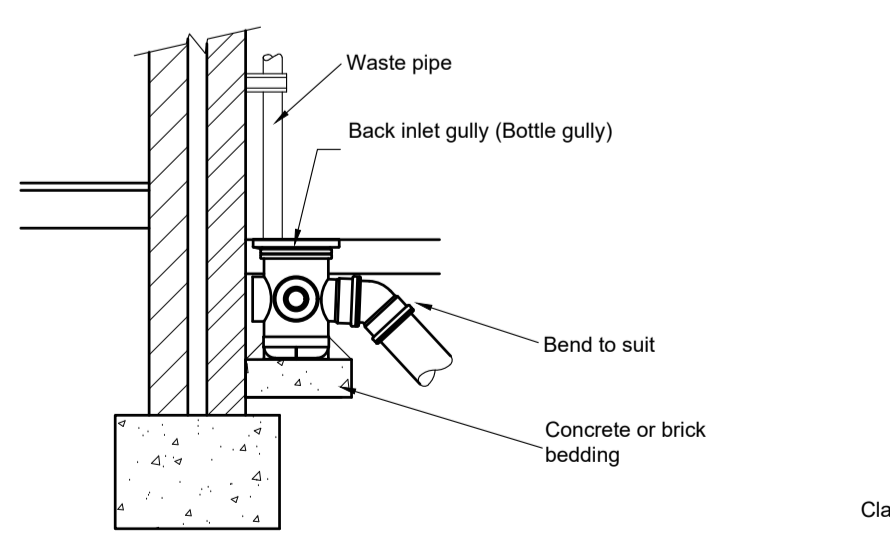
TYPICAL RAINWATER PIPE DETAIL
(Scale 1:25)

FIGURE B.15 TYPICAL MANHOLE DETAIL - TYPE C
Depth from cover level to soffit of pipe less than 1.5 m
Maximum pipe size 450 mm diameter
Rigid material construction

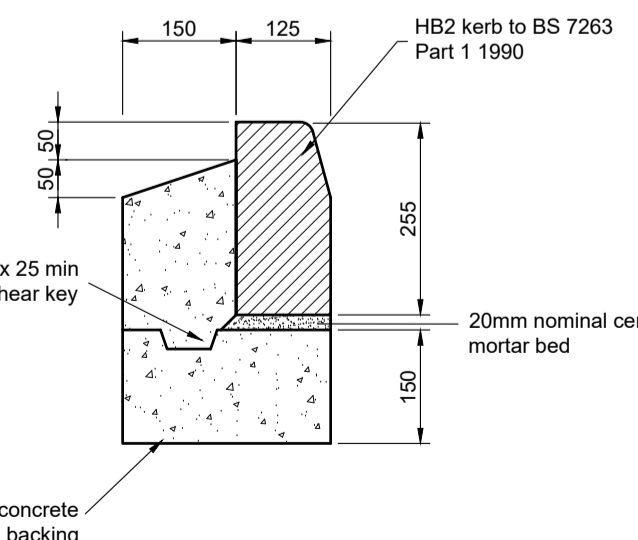


Note: The use of precast concrete chamber units to E2.29 with 150 mm GEN3 in-situ concrete complying with E4.1 and BRE Special Digest 1 in place of brickwork construction is permitted.

Not to scale

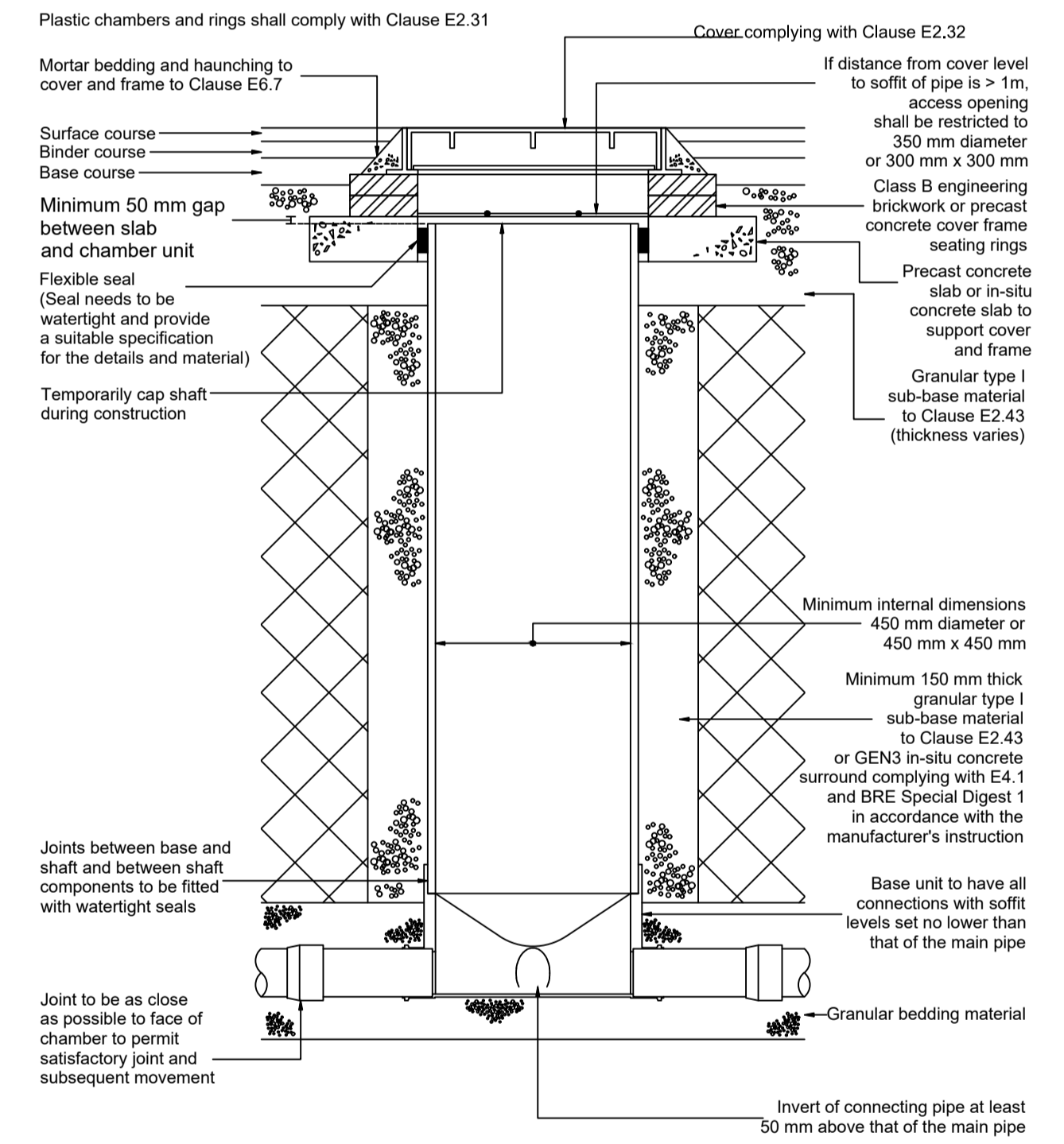


TYPICAL BACK INLET GULLY (UPVC)
(Scale 1:20)



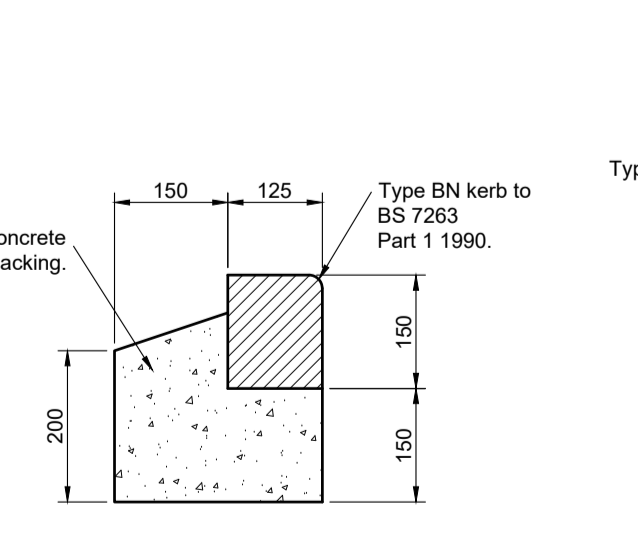
HB2 KERB DETAIL
(Scale 1:10)

FIGURE B.18 TYPICAL INSPECTION CHAMBER DETAIL - TYPE D
Depth from cover level to soffit of pipe up to 2 m
Flexible material construction for use in areas subject to vehicle loading

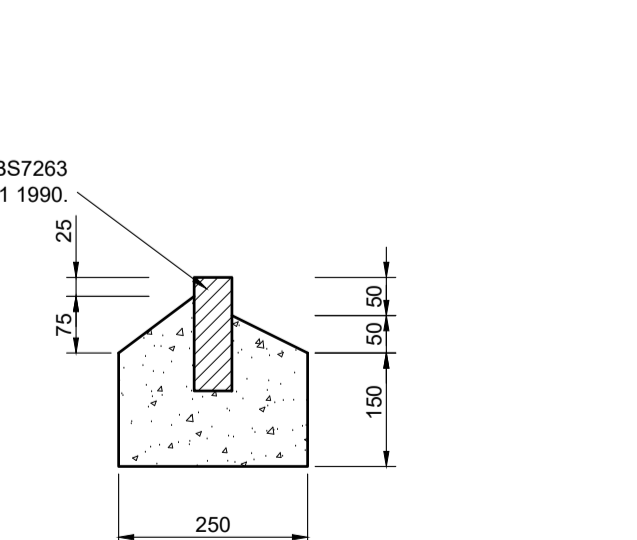


Note: Where the access chamber is in the highway (including any footway), the highway authority can have specific requirements.

Not to scale

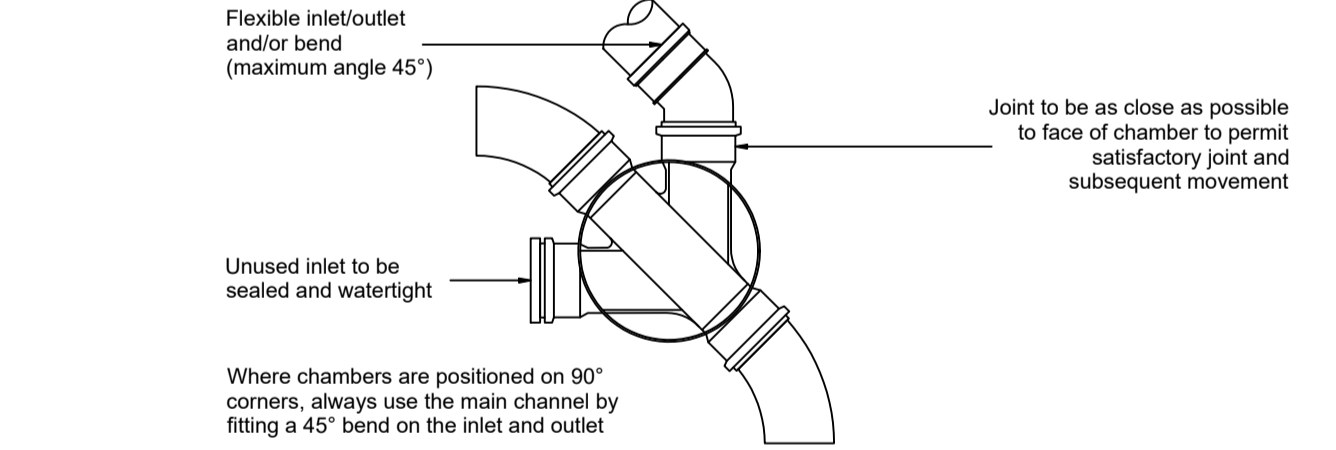
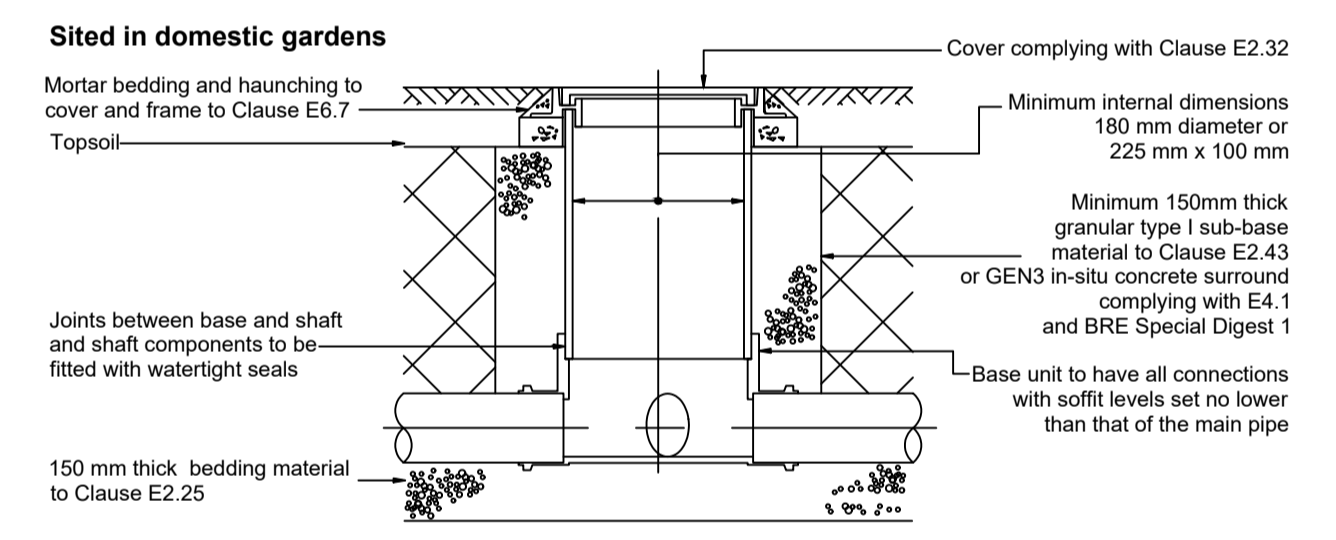
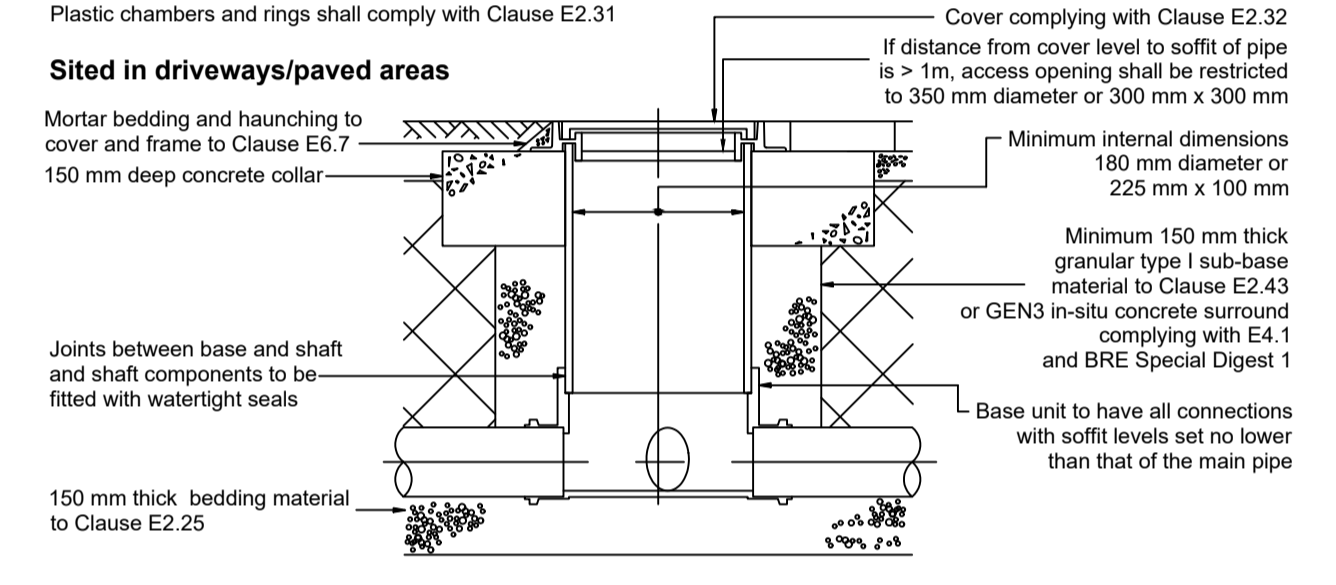


BN KERB DETAIL
(Scale 1:10)



EF EDGING DETAIL
(Scale 1:10)

FIGURE B.23 TYPICAL INSPECTION CHAMBER DETAIL - TYPE E
Depth from cover level to soffit of pipe up to 2 m
Flexible material construction



Note: Where the access chamber is in the highway (including any footway), the highway authority can have specific requirements.

Not to scale

- GENERAL NOTES:**
- All dimensions to be checked on site. All details and dimensions relating to sub-Contractors work must be checked and agreed between the sub-Contractor or supplier and the general Contractor.
 - This drawing is to be read in conjunction with all relevant Architect's and Engineer's drawings and specification.
 - The main Contractor is responsible for ensuring the stability of the structure whilst the works are in progress.
 - Any information given regarding existing underground services is given in good faith after consultation with the relevant authority. No liability is accepted by the Consultant and the main Contractor is responsible for obtaining and checking all information and taking due care and attention whilst undertaking the works.
- DRAINAGE NOTES:**
- All adoptable pipes, bends and junctions shall be vitrified clay in accordance with the current version of BS EN 295-1, with flexible joints and kilnmark certified.
 - All adoptable sewers shall be in strict accordance with the SSG Appendix C - Design and Construction Guidance. Unless otherwise stated adoptable sewers shall be 150mm diameter and shall be laid in a class S bedding. Where the depth to soffit is less than 1.2m under a public highway or 0.5m elsewhere the pipe shall be laid with a class Z bedding.
 - All private building drainage shall be constructed in strict accordance with the current version of BS EN 752:2017. Unless otherwise specified building drainage shall be 100mm diameter and shall be laid at a minimum gradient of 1 in 40 for foul drains and 1 in 80 for surface water drains. All building drains shall be laid in class B bedding unless otherwise specified.
 - Where a pipe is within 1m of a foundation the trench shall be filled with class GEN 3 concrete up to the lowest level of the foundation. Where the trench is further than 1m from the foundation, the trench shall be filled with class GEN 3 concrete to a level below the lowest level for the foundation equal to the distance from the foundation less 150mm. In both cases the pipe shall be bedded and surrounded in 150mm thick class GEN 3 concrete.
 - Where pipes, external to the structures, have a depth to soffit from ground level of less than 450mm they shall have a class GEN 3 concrete encasement (150mm thick). In all other cases the pipes shall be bedded and surrounded with 100mm thick granular material.
 - In any circumstances where pipes are bedded and surrounded in concrete flexible joints should be provided. Compressible boards (fibroboard or polystyrene) shall be provided at a maximum of 8m centres (coinciding with pipe joints). The boards shall be pre-cut to pipe diameter and to a height and width equal to the concrete cross section. A board thickness of 18mm for pipes up to 450mm nominal diameter and 36mm for pipes over 450mm nominal diameter.
 - All svsps shall have rodding access plates fitted at their bases (ground floor level).
 - Where existing pipes are to be abandoned they shall be dug out together with any abandoned manholes.
 - Any discrepancy between the drawing and site should be reported immediately to the Engineer.
 - All manhole and chamber sizes are given as a minimum to meet the SSG Appendix C-Design and Construction Guidance.

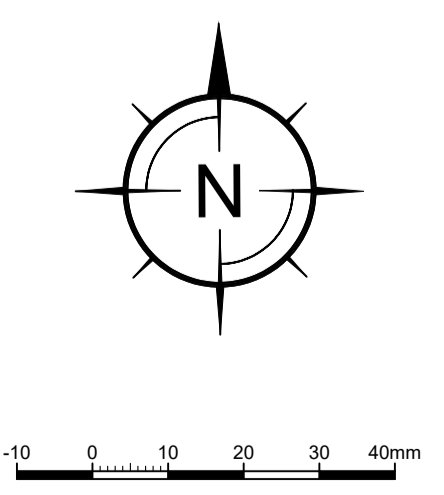
REV	DATE	BY	DESCRIPTION	CHK
B	17.02.23	MJP	Updated subbase depth & diffuser tank	SRM
A	12.08.22	MJP	Updated subbase depth	SRM

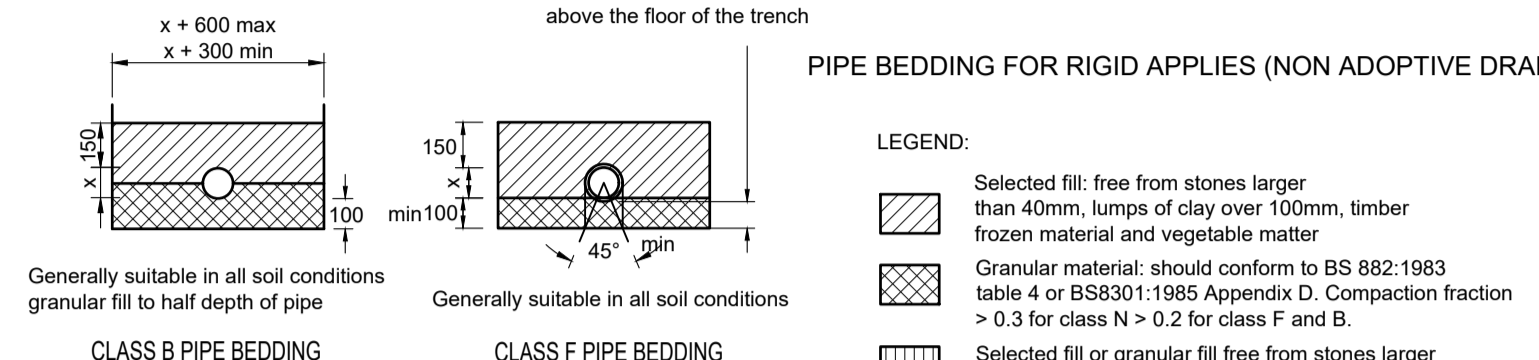
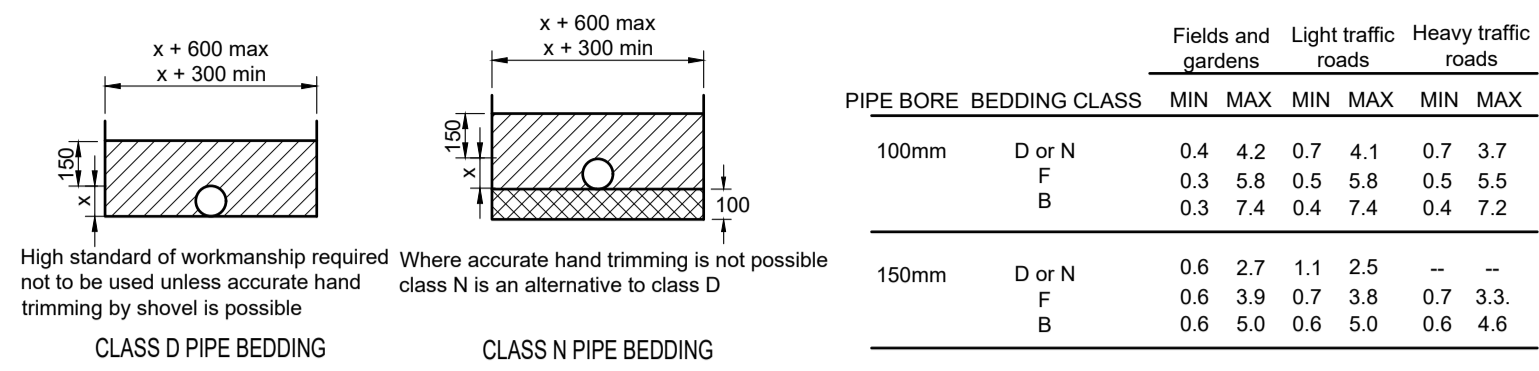
Issue Status: **PRELIMINARY**

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01273 424424
reception@civil.co.uk
www.civil.co.uk

CLIENT: Oceanview Developments
PROJECT: Land West Of Hook Lane, Hook Lane, Westergate
TITLE: Construction Details

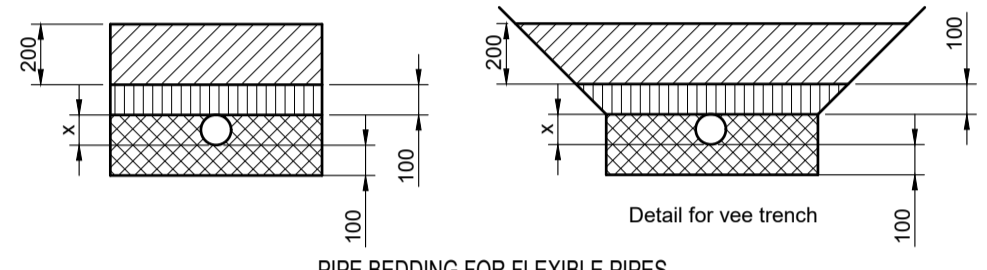
DRAWN: A Davies DATE: July 2022 PROJECT NO: 23582
DRAWN BY: S Magowan CHECKED: SRM
SCALE: As Shown @ A1



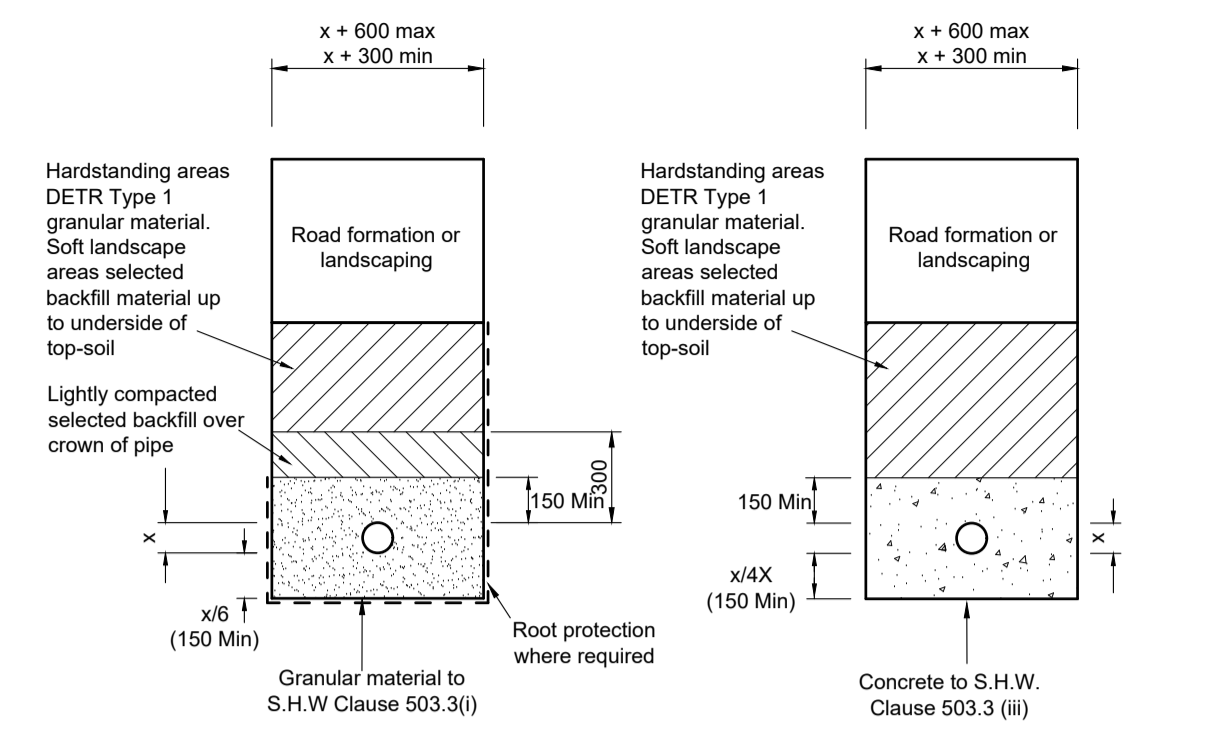


PIPE BEDDING FOR RIGID APPLIES (NON ADOPTIVE DRAINS)

PIPE BORE	BEDDING CLASS	Fields and gardens		Light traffic roads		Heavy traffic roads	
		MIN	MAX	MIN	MAX	MIN	MAX
100mm	D or N	0.4	4.2	0.7	4.1	0.7	3.7
	F	0.3	5.8	0.5	5.8	0.5	5.5
	B	0.3	7.4	0.4	7.4	0.4	7.2
150mm	D or N	0.6	2.7	1.1	2.5	--	--
	F	0.6	3.9	0.7	3.8	0.7	3.3
	B	0.6	5.0	0.6	5.0	0.6	4.6
225mm	D or N	--	--	--	--	--	--
	F	0.6	2.5	1.2	2.1	--	--
	B	0.6	3.3	0.8	3.2	0.9	2.4



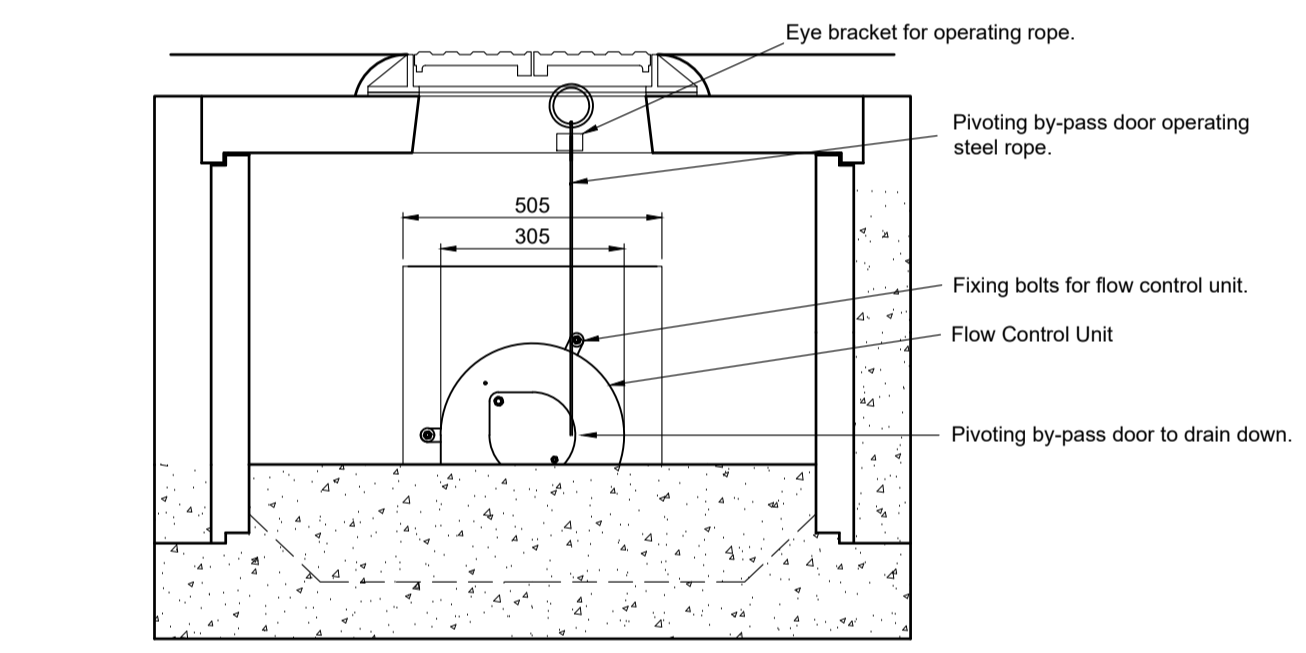
PIPE BEDDING DETAILS (NON ADOPTABLE DRAINS)
(Not To Scale)



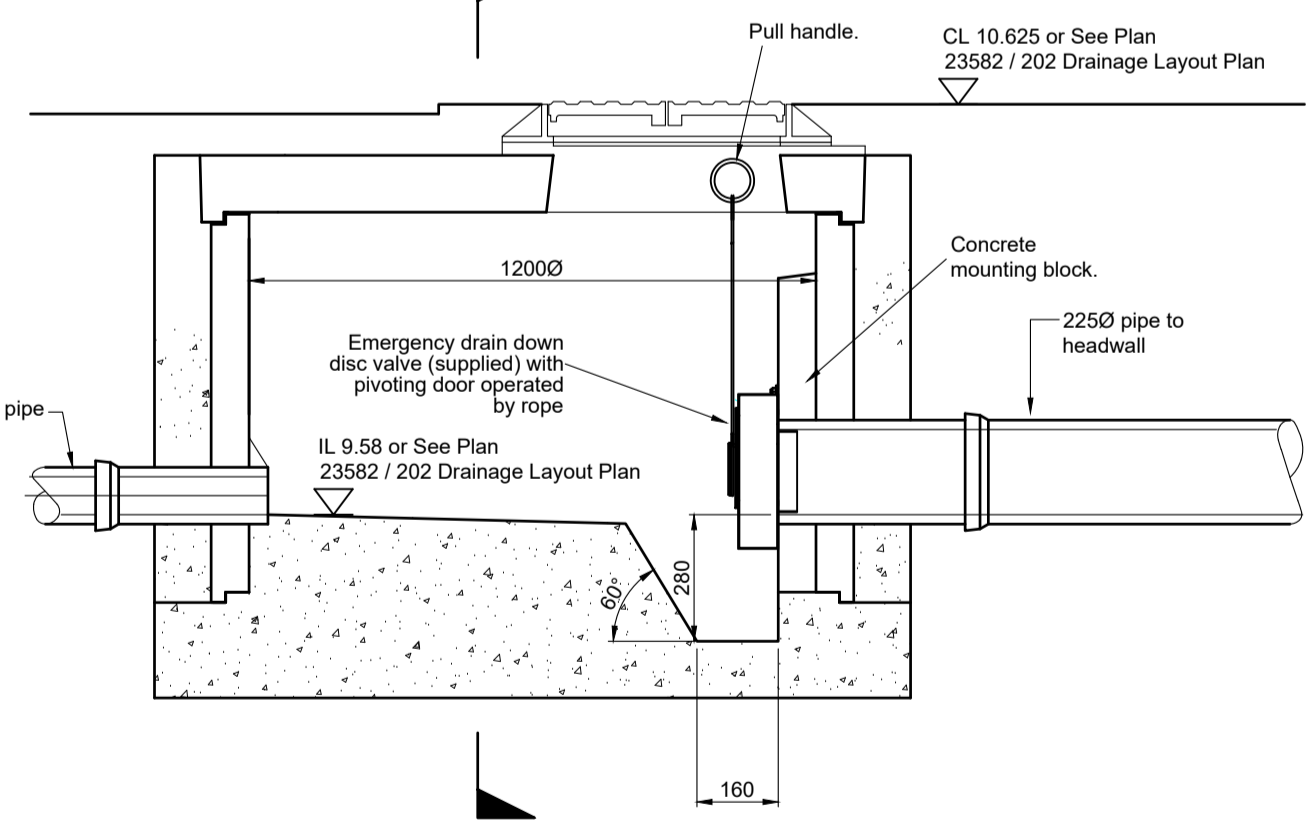
ADOPTABLE PIPE BEDDING DETAILS

ADOPTABLE PIPE BEDDING DETAILS

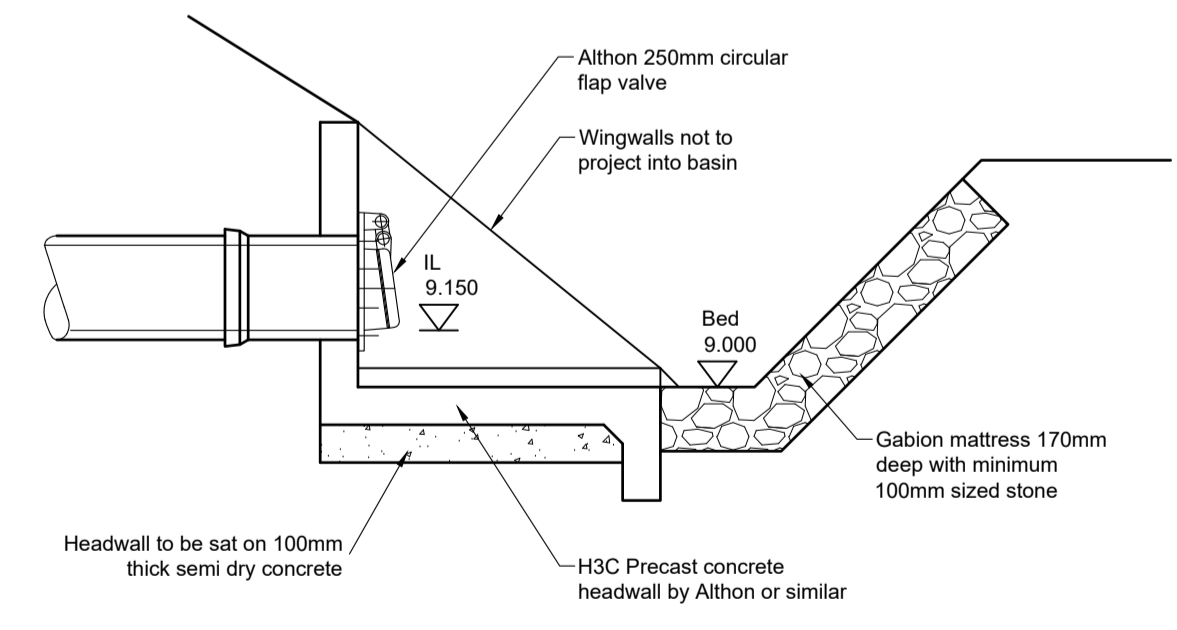
LEGEND:
 Selected fill: free from stones larger than 40mm, lumps of clay over 100mm, timber frozen material and vegetable matter
 Granular material: should conform to BS 882:1983 table 4 or BS3301:1985 Appendix D. Compaction fraction > 0.3 for class N > 0.2 for class F and B.
 Selected fill or granular fill free from stones larger than 40mm
 'x' - Denotes external pipe diameter.



SECTION
(Scale 1:20)



FLOW CONTROL VORTEX MANHOLE
(Scale 1:20)



HEADWALL DETAIL
(Scale 1:20)

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- All manhole and chamber sizes are given as a minimum to meet the SSG Appendix C-Design and Construction Guidance

REV	DATE	BY	DESCRIPTION	CHK
C	09.03.23	AJD	Hydrobrake detail updated	SRM
B	17.02.23	MJP	Outfall pipe changed to 225mm	SRM
A	12.08.22	MJP	Minor note update	SRM

Issue Status **PRELIMINARY**

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CLIENT
Oceanview Developments

PROJECT
Land West Of Hook Lane, Hook Lane, Westergate

TITLE
Construction Details

DRAWN	DATE	PROJECT NO.
A Davies	July 2022	23582
ENGINEER	CHECKED	REV.
S Magowan	SRM	
SCALE	PRG. No.	REV.
As Shown @ A1	252	C

